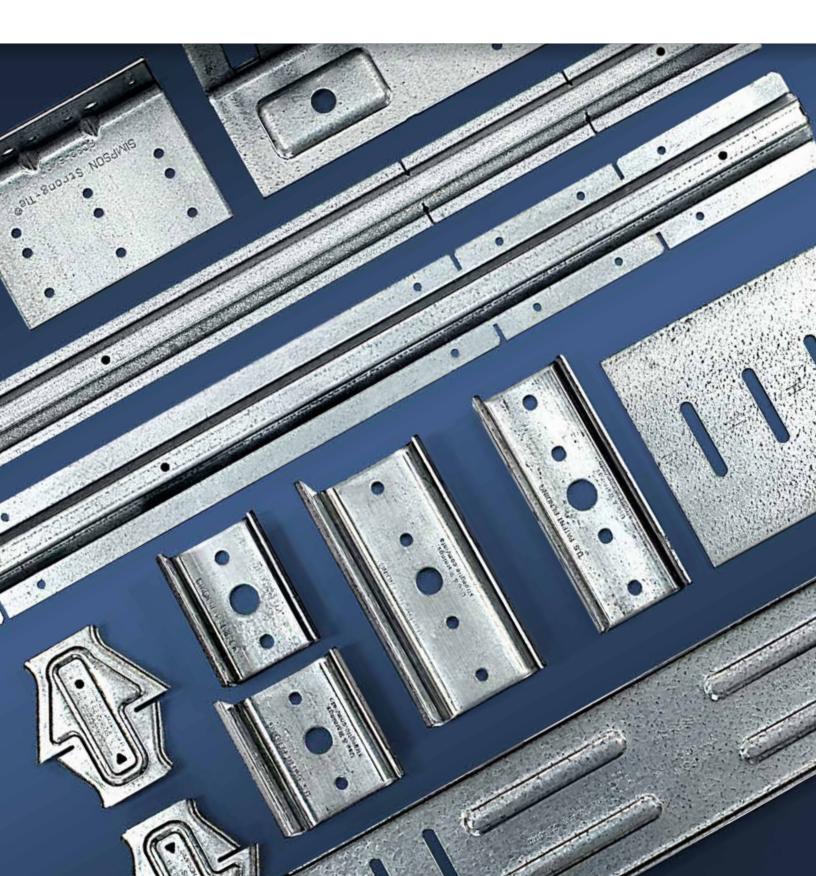
Connectors for Cold-Formed Steel Construction

C-CF-2017 | (800) 999-5099 | strongtie.com





Innovation to make CFS jobs faster and easier

At Simpson Strong-Tie, we are committed to being your valued partner and total solutions provider for cold-formed steel (CFS) commercial curtain-wall, mid-rise and residential construction. Our products are manufactured with quality that our customers can rely on — with precision engineering, thorough lab testing and design software that automates AISI design calculations. The result is, we deliver innovative and lower installed-cost solutions for CFS applications.

Complete, Tested Solutions to Meet Specifier Needs

We are committed to developing fully engineered products through extensive assembly testing capturing the interaction of the connector with the framing member and the interaction of the connector with the anchorage. Assembly testing mitigates design risk by eliminating member behavior assumptions near the connector, and eccentricity and prying action assumptions for fasteners and anchors.

We also provide our specifiers with state-of-the-art software, enabling powerful, efficient and accurate member/connector analysis and design in accordance with AISI specifications. Our Simpson Strong-Tie[®] CFS Designer[™] software is an affordable, easy-to-use solution — helping you expedite most of your day-to-day CFS design needs. Visit **strongtie.com/software** to learn more.

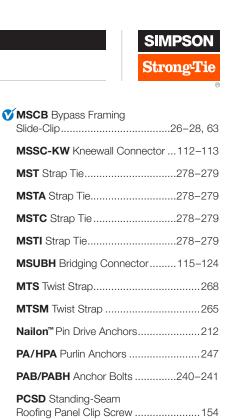
Lower Installed Costs to Meet Contractor Budgets

Our commitment extends to reducing your installed costs — designing products to minimize screw count, eliminate predrilling the connector and minimize tool clearance issues without compromising strength or durability. We field-test our products to ensure they can be installed accurately and efficiently following your unique specifications.

Alphabetical Index

	ABS Anchor Bolt Stabilizer
	AHEP Adjustable Hip-End Purlin274
	AnchorMate® Anchor Bolt Holder 173
	AT-XP® Anchoring Adhesive
	BP/LPB Bearing Plates175
	CBSDQ Sheathing-to-CFS Screw 150–151
	CM/CMST Coiled Strap
	CNW Coupler Nuts176
	Code Listing Key Chart11
	Conversion Charts17
	Corrosion Resistance18-21
	Crimp Drive® Anchors208-211
	CS Coiled Strap136, 259
	Custom Clips and Angles
V	DBC Drywall Bridging Connector 134–135
V	DBR/SBR Spacer Bracers
ø	DIA Drop-In Internally Threaded Anchor218–224
ø	DIAB Drop-In Internally Threaded Anchor215–217
	DSP Stud Plate Tie261
V	DSSCB Drift Strut Sliding Clip Bypass44–45
	DTC Head-of-Wall Slide-Clip Application38
	DTC Roof Truss Clips273
ø	DTT1Z Tension Tie244
	DWF Drywall Screw155
	DWFSD Drywall Screw155
ø	FC Bypass Framing Fixed-Clip Connector57–60
V	FCB Bypass Framing
	Fixed-Clip 35, 47–55, 65–67
~	FHA Strap Tie278–279
鲫	FSB Bypass Framing Fixed-Clip Strut Connector
	GCN-MEP Gas-Actuated Concrete Nailer225–226
ø	GCN-MEP Gas-Actuated Concrete Nailer Fuel Cell
凾	GCN-MEPMAG Gas-Actuated

ø	GCN-MEP Gas-Actuated Concrete Nailer Magnetic Ring
ø	GDP Concrete Pins225–227
ø	GDPS Concrete Pins225–227
ø	GDPSK Spiral Knurl Gas Pins225–228
	H Hybrid Connectors
	H Seismic and Hurricane Ties262
	H10S Seismic and Hurricane Tie
	HETA/HHETA Embedded Truss Anchors266–267
	HPA Purlin Anchor247
	HRS Strap Tie278–279
	HSCNW High-Strength Coupler Nut 176
	HTS Twist Strap268
	HTSM Twist Strap265
	HTT Tension Tie244
	HUC Hanger
	ICFVL Ledger Connector
ø	IDCB Drift-Clip Bypass Framing Connector43
	L Skewable Angles257
	LBP Bearing Plate
	LCB Column Base 177
	LCE Post Cap305
	LS Skewable Angles257
	LSTA Strap Tie278–279
V	LSTHD/STHD Strap Tie Holdowns248–249
	LSTI Strap Tie278–279
	LSUBH Bridging Connector 115–124
	LTA2 Masonry Connector270
	LTB/TB Bridging137, 259
	LTP5 Framing Anchor281
	LTS Twist Strap
	MASA Mudsill Anchor178–179
	MASAP Mudsill Anchor178–179
	META Embedded Truss Anchors266–267
	Metric Conversions



Powder-Actuated Fasteners	
	230–235

PS Strap Tie278-279
PSCA Panel Sheathing Clip
PSCL Panel Sheathing Clip
PSPN Protecting Shield Plate
PT-27 General Purpose Fastening Tool229

Quik Drive® Systems
and Applications159–171
RCA Rigid Connector Angles92–98
RFB Retrofit Bolt239
S/B Hanger254
S/BA Hanger254
S/DHUTF Drywall Hangers255
S/DSC Drag Strut Connector277
S/DTT Tension Tie244
S/H and H Seismic and Hurricane Ties262
S/H1A Seismic and Hurricane Tie264
S/HDB Holdown245–246
S/HDS Holdown245-246
S/HDU Holdown243
S/HGAM Masonry Connector



Concrete Nailer Lathing Washer......228

Alphabetical Index

V	S/HJCT Hanger252–253
	S/HTC Heavy Truss Clip272
V	S/JCT Hanger252–253
	S/LBV Hanger254
	S/LS Skewable Angle99, 257, 272
	S/LTT Tension Tie
	SB Anchor Bolts236
øv	SBR/DBR Spacer Bracers
ø	Slide-Clip Connector
V	SCB/MSCB Bypass Framing Slide-Clip26–28, 35, 63
⊚ V	SCHA Slide-Clip Connectors for Horizontal Anchorage
	Screws Fastening to Metal140
	SCW Head-of-Wall Slide-Clip36-37, 64
	Self-Drilling E Metal Screw142
	Self-Drilling X Metal Screw145–146
	SET-XP® Anchoring Adhesive 182–185
V	SFC Steel Framing Connector
V	SJC Steel-Joist Connector 61, 68–69, 82–87, 91, 251
	SP Stud Plate Tie
	Special Order Custom Clips and Angles56
	SSB Bypass Framing Slide-Clip Strut
V	SSC Steel-Stud Connector
	SSP Stud Plate Tie
V	SSTB® Anchor Bolt237–239
	SSWAB Anchor Bolts
	SSWHSR Extension Kit
	SSWT Anchor Bolt Template278–279
	ST Strap Tie
	STC Roof Truss Clip273
	STCT Roof Truss Clip273
	Steel Strong-Wall [®] Anchorage Details – Solutions – Templates

Steel Strong-Wall® Cold-Formed Steel 1st-Story Floor Systems290–291

Steel Strong-Wall [®] Cold-Formed Steel on Concrete Foundations	
Steel Strong-Wall [®] Cold-Formed Steel Two-Story Stacked on Concrete Foundation292–295	
Steel Strong-Wall [®] Shearwall285-295	
Steel Strong-Wall [®] Uplift Equations 289	
STHD/LSTHD Strap Tie	
Holdowns	
StrapMate® Strap Holder174	
Strong Frame® Special Moment Frame283–284	
Strong-Bolt® 2 Wedge Anchor 198-203	
Strong-Drive® FPHSD FRAMING-TO-CFS Screw143–144	
Strong-Drive® PPSD SHEATHING-TO-CFS Screw152–153	
Strong-Drive® SELF-DRILLING X METAL Screw	
Strong-Drive® TB WOOD-TO-STEEL Screw	
Strong-Drive® XE EXTERIOR STRUCTURAL METAL Screw141	
Strong-Drive [®] XL LARGE-HEAD METAL Screw43, 140, 148–149, 165	
Strong-Drive® XM MEDIUM-HEAD METAL Screw9, 140, 147, 165	
VSUBH Bridging Connector115–124	
TB and LTB Bridging259	
TBD22 Truss Brace Diagonal275	
Tie-Wire Wedge Anchor213–214	
Titen HD [®] Heavy-Duty Screw Anchor189–197	
Titen [®] 2 Concrete and Masonry Screws204–207	
Titen [®] Hex-Head Screws 	
TJC Jack Truss and Rafter Connector271	
TSBR Truss Spacer Restraint277	
TSP Stud Plate Tie261	
Utility Clip Dimensions90-91	
VGT and SVGT2.5 Variable-Pitch Girder Tiedown269	
W/WP Hangers256	
愛 Zinc Nailon™ Pin Drive Anchors	



General Information	10-22 ►
Deflection Connectors	24-41 🕨
Drift Clips	42-45 ►
Rigid Connectors	46-113 ►
Bridging and Bracing Connectors	114–137 ►
Fasteners and Quik Drive®	138–171 ►
Concrete Connectors	172-179 🕨
Anchors	180-241 🕨
Holdowns and Tension Ties	242-249 🕨
Joist Framing Connectors	250-259 ►
Roof, Truss and Rafter Connectors, Ties and Straps	260-275 ►
Lateral Connectors, Ties and Straps	276-281 ►
Lateral Systems	282-303 ►
Miscellaneous	304-307 ►

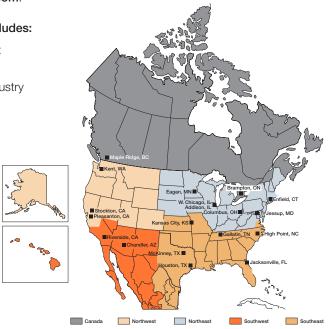
Introduction

For more than 60 years, Simpson Strong-Tie has focused on creating structural products that help people build safer and stronger homes and buildings. A leader in structural systems research and technology, Simpson Strong-Tie is one of the largest suppliers of structural building products in the world. The Simpson Strong-Tie commitment to product development, engineering, testing and training is evident in the consistent quality and delivery of its products and services.

For more information, visit the company's website at **strongtie.com**.

The Simpson Strong-Tie Company Inc. No-Equal pledge includes:

- Quality products value-engineered for the lowest installed cost at the highest-rated performance levels
- The most thoroughly tested and evaluated products in the industry
- Strategically located manufacturing and warehouse facilities
- National code agency listings
- The largest number of patented connectors in the industry
- · Global locations with an international sales team
- In-house R&D and tool and die professionals
- In-house product testing and quality control engineers
- Support of industry groups including AISI, AITC, ASTM, ASCE, AWC, AWPA, ACI, AISC, CSI, CFSEI, ICFA, NBMDA, NLBMDA, SDI, SETMA, SFA, SFIA, STAFDA, SREA, NFBA, TPI, WDSC, WIJMA, WTCA and local engineering groups



SIMPSON

Strong-Tie

The Simpson Strong-Tie Quality Policy

We help people build safer structures economically. We do this by designing, engineering and manufacturing No-Equal structural connectors and other related products that meet or exceed our customers' needs and expectations. Everyone is responsible for product quality and is committed to ensuring the effectiveness of the Quality Management System.



Karen Colonias Chief Executive Officer

Getting Fast Technical Support

When you call for engineering technical support, having the following information on hand will help us to serve you promptly and efficiently:

- Which Simpson Strong-Tie[®] catalog are you using? (See the front cover for the catalog number.)
- Which Simpson Strong-Tie product are you using?
- What are your application and load requirement?
- What are the carried and/or supporting members' size, gauge and strength?



We Are ISO 9001-2008 Registered

Simpson Strong-Tie is an ISO 9001-2008 registered company. ISO 9001-2008 is an internationally-recognized quality assurance system that lets our domestic and international customers know that they can count on the consistent quality of Simpson Strong-Tie[®] products and services.

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Connectors for Cold-Formed Steel Construction

New Products for 2017



Version V2.0

Simpson Strong-Tie[®] CFS Designer[™] Software

Simpson Strong-Tie CFS Designer gives cold-formed steel (CFS) Designers the ability to design CFS beam-column members according to AISI specifications as well as analyze complex beam loading and span conditions. Intuitive design tools automate common CFS systems such as wall openings, compression posts for shearwalls and floor joists with unbalanced live-load combinations.

Version V2.0 is now equipped with a powerful tool to automate the design of load-bearing wall systems up to eight stories in height.

See p. 23 for more information.



RCKW Kneewall Connectors

The Simpson Strong-Tie® RCKW rigid connectors have been developed to resist overturning moment at the base of exterior kneewalls and parapets as well as interior partial-height walls. These connectors offer a unique anchor-hole pattern that permits anchorage to both concrete and structural steel.

See pp. 100–111 for more information.



SCHA Slide-Clip Connectors for Horizontal Anchorage

The SCHA slide-clip connector is an ideal solution for panelized or stick-frame construction where cold-formed steel bypass framing anchors to the top of a floor slab or the bottom flange of a steel beam. The connector features a wide anchorage leg that minimizes connection eccentricity and accommodates several different anchorage methods to concrete and steel. The included SCVS slider provides superior rotational support to the vertical leg of the SCHA connector, helping to improve the buckling performance of the anchored leg.

See pp. 39-41 for more information.



SBR and DBR Spacer Bracers

Specify the only bridging connectors on the market with load ratings based on assembly testing. The new Simpson Strong-Tie® SBR and DBR spacer bracers come with load data based on assembly testing so you can mitigate risk and maximize design confidence. The tabulated design values and precision-engineered slots make it easier to provide a value-engineered solution to your customers.

See pp. 125–132 for more information.

Connectors for Cold-Formed Steel Construction

New Products for 2017





FC Bypass Framing Fixed-Clip Connectors

Ideal for high-seismic areas, Simpson Strong-Tie® FC connectors are the optimal solution for fixed-clip bypass framing. FC clips are often welded to the structure in high-seismic zones, but they also feature anchorage holes so that concrete screws or powder-actuated fasteners can be used to attach the clip to the structure. In addition to its anchorage versatility, the FC clip features prepunched screw holes for the framing attachment, eliminating the need for predrilling holes or worrying that fastener placement doesn't match the Designer specifications. FC connectors are manufactured using heavy-duty 10- and 12-gauge steel to provide exceptional resistance to in-plane seismic load.

See pp. 57-60 for more information.



SC Bypass Framing Slide-Clip Connectors

Ideal for high-seismic areas, Simpson Strong-Tie® SC connectors are the optimal solution for slide-clip bypass framing. SC clips are often welded to the structure in high-seismic zones, but they also feature anchorage holes so that concrete screws or powder-actuated fasteners can be used to attach the clip to the structure. SC connectors are manufactured using heavy-duty 10- and 12-gauge steel to provide exceptional resistance to in-plane seismic load.

See pp. 29-32 for more information.

FSB Bypass Framing Fixed-Clip Strut Connector

The FSB connector is the fixed-clip version of our popular SSB slide-clip strut connector. The FSB is commonly used at the bottom flange of a steel beam to accommodate large stand-off distances for bypass curtain-wall studs. The connector features anchor holes throughout the length of the part to enable use with a wide array of stand-offs. The FSB can also be field-trimmed for smaller stand-offs.

See pp. 61-62 for more information.



IDCB Drift-Clip Bypass Framing Connector

The IDCB drift-clip connector is used to secure bypass stud framing to the edge of a slab. The connector will accommodate 1" of lateral drift in each direction and 1" of upward and downward vertical deflection. Horizontal embossments and corner gussets optimize performance for resisting out-of-plane loads.

See p. 43 for more information.



RCA Rigid Connector Angles

The Simpson Strong-Tie® rigid connector angle is a general-purpose clip angle designed for a wide range of cold-formed steel construction applications. With prepunched holes for fastener attachment, these L-shaped clips save time and labor on the job.

See pp. 92-98 for more information.

Connectors for Cold-Formed Steel Construction

New Products for 2017





S/DHUTF Drywall Hangers

The S/DHUTF top-mount hangers are designed to carry floor joist loads to a CFS stud wall through two layers of %" gypsum board. These hangers install after the drywall is in place. The hangers come in sizes that accommodate 8", 10" and 12" joist depths.

See p. 255 for more information.



DTT1Z Tension Tie

DTT1Z tension tie is suitable for lighter-duty hold-down applications on single or back-to-back studs, and installs easily with #10 self-drilling screws.

See p. 244 for more information.

Titen[®] 2 Concrete and Masonry Screw

With patented undercutting threads that make installation easier and increase load capacity, the Titen 2 concrete and masonry screw is ideal for attaching all types of components to concrete and masonry. The improved thread design undercuts the base material more efficiently, reducing installation torque and making it easier to drive without binding, snapping or stripping, even during installation into hard base material.

See pp. 204-207 for more information.





Lipped Drop-In

DIAB Drop-In Internally Threaded Anchor

Simpson Strong-Tie introduces a new, redesigned Drop-In Anchor (DIAB) that provides easier installation into base materials. Improved geometry in the preassembled expansion plug allows the anchor to install with 40% fewer hammer strikes than previous versions. These deformation-controlled expansion anchors are easily set by driving the plug toward the bottom of the anchor using either the hand- or power-setting tools.

See pp. 215-217 for more information.



Strong-Drive® XM MEDIUM-HEAD METAL Screw

Strong-Drive XM Medium-Head Metal screws have been engineered as a 1-for-1 replacement option for power-actuated pins in steel decking to structural members involving wide or narrow valley, nestable or interlocking steel decking. This means you can keep the same spacing and easily substitute screws for pins. This screw is also an ideal solution for CFS connectors that demand high pullover values.

See p. 147 for more information.

How To Use This Catalog



New Products

New products are shown with the symbol. There are also many new sizes within existing model series.



Value Engineered

This icon indicates a product that is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

Extra Corrosion Protection

The teal arrow icon identifies products that are available with additional corrosion protection (ZMAX[®], hot-dip galvanized or double-barrier coating). The SS teal arrow icon identifies products also available in stainless steel. Other products may also be available with additional protection; contact Simpson Strong-Tie for options. The end of the product name will indicate what type of extra corrosion protection is provided (Z = ZMAX, HDG = hot-dip galvanized or SS = stainless steel). Stainless products may need to be manufactured upon ordering. See pp. 18–21 for information on corrosion, and visit our website **strongtie.com/info** for more technical information on this topic.

How We Determine Allowable Loads

Allowable loads in this catalog are determined using calculations and/or one or more of the following methods:

- a minimum of 3 static load tests in CFS assemblies;
- a minimum of 3 static load tests in structural steel jigs;
- a minimum of 3 static load tests of products embedded in concrete or masonry.

Where available, testing is performed to test criteria established by industry (ASTM, AISI or ICC-ES Acceptance Criteria). Where a test standard is unavailable, testing is conducted per sound engineering principles. Some tests include only portions of a product such as purlin anchor tests — only the embedded hook is tested, not the screwed or

bolted section of the strap, which is calculated. Testing to determine allowable loads in this catalog is not done on connection systems in buildings. Testing is conducted in an IAS accredited laboratory. Typically the allowable load is limited to an average test load at ¼" deflection, an average or lowest test value (nominal load) divided by a safety factor or the calculated value. The safety factor is prescribed by Section F of AISI-S100. For LRFD, the nominal connector strength is multiplied by a resistance factor, also prescribed by Section F of AISI-S100.

For detailed information regarding how Simpson Strong-Tie tests specific products, contact Simpson Strong-Tie.

Load Table Explanation

Dimensions: This shows the product dimensions (material thickness, length and width in this case.) The product drawing includes these callouts as a cross-reference. Allowable Design Load: The maximum load imposed on a connection during the life of a structure. There may be multiple design loads acting in different directions (up, down, lateral, perpendicular, etc.) imposed on a connection. When connectors are attached to two CFS members of different thicknesses, the Designer shall use the thinner of the two members for selecting allowable loads.

This Sim	del No.: is the pson Strc duct name	<u> </u>	\bigwedge	the and	s teners fastene I type re lieve the	r quant equired	the (hows to w ity attac to base	CFS sup hich the ched. A	oporting e produ Ilowable nis CFS	e load is supportin		for th Key whic	e Ref: See the Code Re Chart, to de h code rep de this prod	eference etermine orts
		Connector	Dimensi	ons (in.)		- <u> </u>		Fasteners (Total) Rafter/Stud/Joist Thickness		Allowabl	e ASD Tens	ion Load		
	Model No.	Material Thickness	w							33 mil 4	43 mil 54 mil	Code Ref.		
		mil. (ga.)	vv	L	33 mil ((20 ga.)	43 mil (18 ga.)	54 mil	(16 ga.)	(20 ga.)	(18 ga.)	(16 ga.)		
	LSTA9		11⁄4	9	(8) :	#10	(8) #10	(8)	#10	705	1,120	1,190		
	LSTA12	33	11⁄4	12	(10)	#10	(10) #10	(8)	#10	885	1,190	1,190	IP1,	
	LSTA15	(20 ga.)	1 1⁄4	15	(12)	#10	(12) #10	(10)	#10	1,060	1,190	1,190	L2, FL	
	LSTA18		1 1⁄4	18	(14)	#10	(12) #10	(10)	#10	1,190	1,190	1,190		

Other Catalog Definitions:

Deflection: The distance a point moves when a load is applied.

Nominal Tension Load (Strength): The capacity of a structure or component to resist the effects of loads, as determined in accordance with AISI-S100 using specified material strengths and dimensions. Typically taken as the average value of at least three tests.

The Nominal Tension Load should not be compared against design loads (ASD, LRFD), but used only where the AISI Lateral Design Standard requires the holdown to have nominal tension load (strength) to resist the lesser of the amplified seismic load or the maximum force the system can deliver.

Codes

Code Reference Column in Load Tables

The alpha-numeric "Code Reference numbers" that appear in the "Code Reference" column in load tables throughout this catalog are intended to identify products listed in evaluation agency reports, typically called "code reports," and the specific reports that cover them. The letter designates the evaluation agency from which the report was obtained. The Code Reference column, used in conjunction with the chart at right, indicates which code listing applies to a product. The reference numbers also clearly identify:

- Products submitted for evaluation report listing (160)
- Products with no evaluation report listing (170)
- Products not submitted because they have no load rating and an evaluation report listing is not necessary (180)
- Products that meet prescriptive or conventional construction code requirements (190)
- Product is calculated per code; testing is not required (200)

Where a model has been submitted for listing (160) or does not have an evaluation report listing (170), Simpson Strong-Tie can supply complete test data to support our published loads. Please contact us for a copy of our product test documentation at (800) 999-5099. Product acceptance may be obtained through the Alternate Methods and Materials section of the applicable building code.

Some loads and applications may not be covered in the code report and specific reductions and restrictions may be required by other product evaluation agencies. Visit **strongtie.com/codes** or visit the product evaluation agencies' web sites for the current evaluation reports.

Simpson Strong-Tie[®] products are listed by several product evaluation agencies. Agencies that list our products include ICC-ES; IAPMO UES; UL; FM; the City of Los Angeles, California; and the State of Florida.

Simpson Strong-Tie currently maintains more than 60 ICC-ES ESR and IAPMO UES ER reports evaluated to the 2006/2009/2012/2015 IBC and IRC. We continue to submit product information to ICC-ES in order to update reports or receive additional reports for products in compliance with the latest codes.

To quickly determine which of our stamped and welded connector products are listed in ESR reports, we have obtained the ICC-ES ESR-2523 index report. This report is a reference document to other ESR reports held by Simpson Strong-Tie and will be updated frequently by ICC-ES as new stamped and welded connector evaluation services reports are issued. Please visit **strongtie.com** for the latest information or contact ICC Evaluation Service at **icc-es.org**.

IAPMO Uniform Evaluation Service has been evaluating products for more than 75 years and has the same ANSI accreditation as ICC Evaluation Service for evaluating structural building products to the building codes. IAPMO UES began evaluating structural building products in 2004, utilizing licensed structural engineers to perform quality reviews. To quickly determine which of our stamped and welded connector products are listed in ER reports, we have obtained IAPMO UES ER-102 index report, which will be updated frequently as products are added to ERs. Please visit **strongtie.com** for the latest information or contact IAPMO Uniform Evaluation Service at **iapmoes.org**.

In November 2010, the California Division of the State Architect, issued a revised IR 23-1. The revised Interpretation of Regulation (IR) addresses and clarifies issues relating to Pre-fabricated Wood Construction Connectors. IR 23-1 defines the Purpose and Scope and clarifies Listing Requirements, Acceptable Load Capacities, Design Requirements, Installation Requirements Connector Fabrication (which addresses corrosion-resistant material and/or coatings) and testing requirements. Also IRA-5, updated in October 2012, addresses product and evaluation report acceptance.

On October 1, 2003, the State of Florida's Statewide Product Approval System became effective. The purpose of this system is to provide a single product evaluation and approval system that applies statewide to operate in coordination with the Florida Building Code. This Florida product evaluation and approval system is governed by Florida Statutes, Chapter 553, Section 553.842. Since this law specifies that the product approval system is to apply statewide, Notice of Acceptance is no longer necessary where a product has a statewide approval that is applicable in the High Velocity Hurricane Zone (HVHZ) and is installed in accordance with its conditions of use.

To access pertinent code reports related to Simpson Strong-Tie[®] products, you can access our Code Report Finder Software at **strongtie.com/codes.**

Code Report Reference Key Chart

Agency	Code Listing	Code Reference
	ESR-1161	1
	ESR-2138	12
	ESR-2508	13
	ESR-2555	14
ICC-ES	ESR-2611	15
ESR	ESR-2713	16
	ESR-2811	17
	ESR-2920	18
	ESR-1679	19
	ESR-3006	110
	ER-124	IP1
	ER-238	IP2
IAPMO UES ER	ER-242	IP3
	ER-326	IP4
	ER-263	IP5
	RR 25469	L1
	RR 25489	L2
	RR 25741	L3
	RR 25744	L4
	RR 25827	L5
City of Los Angeles,	RR 25837	L6
California	RR 25851	L7
	RR 25943	L8
	RR 25960	L9
	RR 25625	L10
	RR25670	L11
	RR25917	L12
State of Florida	Florida Product Approval Visit strongtie.com/codes or floridabuilding.org for accurate and up-to-date product approval and code evaluation reports.	FL
Submitted for Listing	Call us for Status and Test Data	160
No Code Listing	Call us for Test Data	170
No Load Rating	_	180
Prescriptive Code	_	190
Calculated per Code	_	200

* Because code reports can be issued throughout the year, we encourage the user to visit strongtie.com, icc-es.org, iapmoes.org, ladbs.org, and floridabuilding.org. For the most current information, call Simpson Strong-Tie at (800) 999-5099, or contact the code agency directly.

SIMPSOI

Strong-T



Warning

Simpson Strong-Tie Company Inc. structural connectors, anchors, and other products are designed and tested to provide specified design loads. To obtain optimal performance from Simpson Strong-Tie Company Inc. products and achieve maximal allowable design load, the products must be properly installed and used in accordance with the installation instructions and design limits provided by Simpson Strong-Tie Company Inc. To ensure proper installation and use, Designers and installers must carefully read the following General Notes, General Instructions for the Installer and General Instructions for the Designer, as well as consult the applicable catalog pages for specific product installation instructions and notes.

Proper product installation requires careful attention to all notes and instructions, including these basic rules:

- 1. Be familiar with the application and correct use of the connector.
- Follow all installation instructions provided in the applicable catalog, website, *Installer's Pocket Guide* or any other Simpson Strong-Tie publications.
- Install all required fasteners per installation instructions provided by Simpson Strong-Tie Company Inc.: (a) use proper fastener type; (b) use proper fastener quantity; (c) fill all fastener holes; (d) do not overdrive or underdrive nails, including when using gun nailers; and (e) ensure screws are completely driven.
- 4. Only bend products that are specifically designed to be bent. For those products that require bending, do not bend more than once.
- 5. Cut joists to the correct length, do not "short-cut." The gap between the end of the joist and the header material should be no greater than 1%" unless otherwise noted.

Failure to follow fully all of the notes and instructions provided by Simpson Strong-Tie Company Inc. may result in improper installation of products. Improperly installed products may not perform to the specifications set forth in this catalog and may reduce a structure's ability to resist the movement, stress and loading that occurs from gravity loads as well as impact events such as earthquakes and high-velocity winds.

Simpson Strong-Tie Company Inc. does not guarantee the performance or safety of products that are modified, improperly installed or not used in accordance with the design and load limits set forth in this catalog.

Important Information

In addition to following the basic rules provided above as well as all notes, warnings and instructions provided in the catalog, installers, Designers, engineers and consumers should consult the Simpson Strong-Tie Company Inc. website at **strongtie.com** to obtain additional design and installation information, including:

- Instructional builder/contractor training kits containing an instructional video, an instructor guide and a student guide in both English and Spanish;
- Installer's Pocket Guide (form S-C-INSTALL), which is designed specifically for installers and uses detailed graphics and minimal text in both English and Spanish to explain visually how to install many key products;
- Information on workshops Simpson Strong-Tie conducts at various training centers throughout the country;
- Product-specific installation videos;
- Specialty catalogs;
- Code reports Simpson Strong-Tie[®] Code Report Finder software;
- Technical fliers and bulletins;
- Engineering letters;
- Master format specifications;
- Safety data sheets;
- Corrosion information;
- Connector selection guides for engineered wood products (by manufacturer);
- Simpson Strong-Tie® Connector Selector® software;
- Simpson Strong-Tie® Joist Hanger Selector;
- Simpson Strong-Tie® AutoCAD® menu;
- Simpson Strong-Tie[®] CFS Designer™ software;
- Simpson Strong-Tie[®] Anchor Designer software;
- Simpson Strong-Tie[®] Strong-Wall[®] Selector software;
- Simpson Strong-Tie[®] Strong Frame[®] Selector;
- Simpson Strong-Tie® Fastener Finder;
- Simpson Strong-Tie[®] YouTube Channel; and
- Answers to frequently asked questions and technical topics.
- For all our web and mobile apps, visit strongtie.com/software.



General Notes

These notes are provided to ensure proper installation of Simpson Strong-Tie® products and must be followed fully.

- a. Simpson Strong-Tie Company Inc. reserves the right to change specifications, designs and models without notice or liability for such changes.
- b. Steel used for each Simpson Strong-Tie product is individually selected based on the product's steel specifications, including strength, thickness, formability, finish, and weldability. Contact Simpson Strong-Tie for steel information on specific products.
- c. Unless otherwise noted, dimensions are in inches and loads are in pounds.
- d. Unless otherwise noted, welds, bolts, screws and nails may not be combined to achieve highest load value.
- e. Unless otherwise noted, catalog loads are based on cold-formed steel members having a minimum yield strength, Fy, of 33 ksi and tensile strength, Fu, of 45 ksi for 43 mil (18 ga.) and thinner, and a minimum yield strength, Fy, of 50 ksi and tensile strength, Fu, of 65 ksi for 54 mil (16 ga.) and thicker.
- f. Simpson Strong-Tie Company Inc. will manufacture non-catalog products provided prior approval is obtained and an engineering drawing is included with the order. Steel specified on the drawings as 1/s", 3/16", and 1/4" will be 11 gauge (0.120"), 7 gauge (0.179"), and 3 gauge (0.239"), respectively. The minimum yield and tensile strengths are 33 ksi and 52 ksi, respectively.
- g. SSTB (p. 237) is ASTM A36, RFB (p. 239) is ASTM A307.
- Unless otherwise noted, bending steel in the field may cause fractures at the bend line. Fractured steel will not carry load and must be replaced.
- Top flange hangers may cause unevenness. Possible remedies should be evaluated by a professional and include using a face mount hanger or cutting the subfloor to accommodate the top flange thickness.
- j. Built-up members (multiple members) must be fastened together to act as one unit to resist the applied load (excluding the connector fasteners). This must be determined by the Designer/Engineer of Record.
- k. Do not overload. Do not exceed catalog allowable loads, which would jeopardize the connection.
- I. Some model configurations may differ from those shown in this catalog. Contact Simpson Strong-Tie for details.

- m. Some combinations of hanger options are not available. In some cases, combinations of these options may not be installable. Horizontal loads induced by sloped joists must be resisted by other members in the structural system. A qualified designer must always evaluate each connection, including carried and carrying member limitations, before specifying the product. Fill all fastener holes with fastener types specified in the tables, unless otherwise noted. Hanger configurations, height and fastener schedules may vary from the tables depending on joist size, skew and slope. See the allowable table load for the non-modified hanger, and adjust as indicated. Material thickness may vary from that specified depending on the manufacturing process used. W hangers normally have single stirrups; occasionally, the seat may be welded. S/B, S/LBV, W and WP hangers for sloped seat installations are assumed backed.
- n. Simpson Strong-Tie will calculate the net height for a sloped seat. The customer must provide the H1 joist height before slope.
- o. Do not weld products listed in this catalog unless this publication specifically identifies a product as acceptable for welding, or unless specific approval for welding is provided in writing by Simpson Strong-Tie. Some steels have poor weldability and a tendency to crack when welded. Cracked steel will not carry load and must be replaced.
- p. Steel for the framing members must comply with ASTM A1003 Grade 33 minimum. Reference General Note "e" for additional requirements.
- q. Quik Drive[®] screws have been tested per AISI Standard Test Method S904.
- r. Consideration should be given to the screw head specified as this may affect the attached materials.
- s. Do not add fastener holes or otherwise modify Simpson Strong-Tie products. The performance of modified products may be substantially weakened. Simpson Strong-Tie will not warrant or guarantee the performance of such modified products.
- t. All references to bolts or machine bolts (MBs) are for structural quality through bolts (not lag screws or carriage bolts) equal to or better than ASTM Standard A307, Grade A.

General Instructions to the Installer

These general instructions for the installer are provided to ensure proper selection and installation of Simpson Strong-Tie products and must be followed carefully. These general instructions are in addition to the specific installation instructions and notes provided for each particular product, all of which should be consulted prior to and during installation of Simpson Strong-Tie products.

- a. All specified fasteners must be installed according to the instructions in this catalog. Incorrect fastener quantity, size, type, material, or finish may cause the connection to fail.
- b. Holes for $\frac{1}{2}$ " diameter or greater bolts shall be no more than a maximum of $\frac{1}{16}$ " larger than the bolt diameter per AISI S100 Table E3a, Appendix A.
- c. Install all specified fasteners before loading the connection.
- d. Some hardened fasteners may have premature failure if exposed to moisture. The fasteners are recommended to be used in dry interior applications.
- e. Use proper safety equipment.
- f. When installing a joist into a connector with a seat, the joist shall bear completely on the seat. The gap between the end of the joist and the connector or header shall not exceed 1/8" per ICC-ES AC 261 and ASTM D1761 test standards, unless otherwise noted.
- g. For holdowns, anchor bolt nuts should be finger-tight plus ½ to ½ turn with a hand wrench. Care should be taken to not over-torque the nut and impact wrenches should not be used. This may preload the holdown.



General Instructions to the Installer (cont.)

- h. Holdowns and tension ties may be raised off the track as dictated by field conditions to accommodate an anchor mislocated no more than 1½". The holdown shall be raised off the bottom track at least 3" for every ¼" that the anchor is offset from the model's centerline. Anchor bolt slope shall be no greater than 1:12 (or 5 degrees). Contact the Designer if the holdown anchor is offset more than 1½" or raised more than 18". Raised holdown height is measured from the top of the concrete to the top of the holdown bearing plate.
- i. All screws shall be installed in accordance with the screw manufacturer's recommendations. All screws shall penetrate

and protrude through the attached materials a minimum of three full exposed threads per AISI S200 General Provisions Section D1.3.

- j. Welding galvanized steel may produce harmful fumes; follow proper welding procedures and safety precautions. Welding should be in accordance with American Welding Society (AWS) standards. Unless otherwise noted, Simpson Strong-Tie connectors cannot be welded.
- k. Temporary lateral support for members may be required during installation.

General Instructions to the Designer

These general instructions for the Designer are provided to ensure proper selection and installation of Simpson Strong-Tie Company Inc. products and must be followed carefully. These general instructions are in addition to the specific design and installation instructions and notes provided for each particular product, all of which should be consulted prior to and during the design process.

- a. Allowable loads are determined per the AISI S100 unless otherwise specified. Other code agencies may use different methodologies.
- b. The allowable load is typically limited to an average test load at ¼" deflection, or an average or lowest test value (nominal load) divided by a safety factor or the calculation value. The safety factor is prescribed by Section F1 of the AISI S100.
- c. To achieve the loads shown in this catalog, the Designer must verify that the self-drilling screws used for connector installation have P_{SS}/Ω and P_{tS}/Ω values greater than or equal to the values given in the table, Minimum ASD Loads for Screws (lb.), per p. 22 of this catalog.
- d. Allowable simultaneous loads in more than one direction on a single connector must be evaluated as follows:

Design Uplift/Allowable Uplift + Design Lateral Parallel to Track/ Allowable Lateral Parallel to Track + Design Lateral Perpendicular to Track/Allowable Lateral Perpendicular to Track \leq 1.0.

The three terms in the unity equation are due to the three possible directions that exist to generate force on a connector. The number of terms that must be considered for simultaneous loading is at the sole discretion of the Designer and is dependent on their method of calculating wind forces and the utilization of the connector within the structural system.

- e. The term "Designer" used throughout this catalog is intended to mean a licensed/certified building design professional, a licensed professional engineer, or a licensed architect.
- f. All connected members and related elements shall be designed by the Designer.
- g. Unless otherwise noted, member strength is not considered in the loads given and, therefore, one should reduce allowable loads when member strength is limiting.
- h. The average ultimate breaking strength for some models is listed under "nominal tension load".
- i. Simpson Strong-Tie strongly recommends the following addition to construction drawings and specifications: "Simpson Strong-Tie

connectors and fasteners are specifically required to meet the structural calculations of plan. Before substituting another brand, confirm load capacity based on reliable published testing data or calculations. The Engineer/Designer of Record should evaluate and give written approval for substitution prior to installation."

- j. Verify that the dimensions of the supporting member are sufficient to receive the specified fasteners, and develop the top flange bearing length.
- k. Simpson Strong-Tie will provide, upon request, code testing data on all products that have been code tested.
- Most of the allowable loads published in this catalog are for use when utilizing the traditional Allowable Stress Design (ASD) methodology. A method for using Load and Resistance Factor Design (LRFD) for cold-formed steel is also included in AISI S100. When designing with LRFD, the nominal connector strength multiplied by the resistance factor must be used. If not listed or noted in a table footnote, contact Simpson Strong-Tie for the LRFD values of products listed in this catalog.
- m. All steel-to-steel connector screws must comply with ASTM C1513.
- n. Screw strength shall be calculated in accordance to AISI S100 Section E4 or shall be based upon the manufacturer's design capacity determined from testing.
- Simpson Strong-Tie recommends that hanger height be at least 60% of joist height for stability against rotation while under construction prior to sheathing install.
- p. Local and/or regional building codes may require meeting special conditions. Building codes often require special inspection of anchors installed in concrete and masonry. For compliance with these requirements, it is necessary to contact the local and/or regional building authority. Except where mandated by code, Simpson Strong-Tie products do not require special inspection.
- q. When connectors are attached to two CFS members of different thicknesses, the Designer shall use the thinner of the two members for selecting allowable loads.

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Additional Important Information and General Notes for Hybrid (Steel-to-Wood) Connections

These notes are in addition to the previous notes for steel-to-steel connections and are provided to ensure proper installation of Simpson Strong-Tie[®] products and must be followed fully.

a. Unless otherwise noted, allowable loads are for Douglas Fir-Larch under continuously dry conditions. Allowable loads for other species or conditions must be adjusted according to the code. The section from the AC13 criteria indicating the range of specific gravity reads as follows: 3.2.3 The species of lumber used shall have a specific gravity not greater than 0.55 as determined in accordance with the NDS. This chart shows specific gravity and perpendicular to grain compression capacities for the different wood species:

Species	Fc⊥	Specific Gravity
Douglas Fir-Larch (DFL)	625 psi	0.50
Southern Pine (SP)	565 psi	0.55
Spruce-Pine-Fir (SPF)	425 psi	0.42
Hem Fir (HF)	405 psi	0.43
Glulam	650 psi	0.50
LVL (DF/SP)	750 psi	0.50
TimberStrand [®] LSL (E=1.3x106)	680 psi	0.50
TimberStrand [®] LSL (E>1.5x106)	880 psi	0.50
Parallam [®] PSL	750 psi	0.50

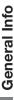
- b. For face-mount hangers and straight straps, use 0.86 of Douglas-Fir table loads for Spruce-Pine-Fir.
- c. A fastener that splits the wood will not take the design load. Evaluate splits to determine if the connection will perform as

required. Dry wood may split more easily and should be evaluated as required. If wood tends to split, consider pre-boring holes with diameters not exceeding 0.75 of the nail diameter (2015 NDS 12.1.5.3).

- d. Wood shrinks and expands as it loses and gains moisture, particularly perpendicular to its grain. Take wood shrinkage into account when designing and installing connections. Simpson Strong-Tie manufactures products to fit common dry lumber dimensions. If you need a connector with dimensions other than those listed in this catalog, Simpson Strong-Tie may be able to vary connector dimensions; contact Simpson Strong-Tie. The effects of wood shrinkage are increased in multiple lumber connections, such as floor-to-floor installations. This may result in the vertical rod nuts becoming loose, requiring post-installation tightening.
- e. Top flange hangers may cause unevenness. Possible remedies should be evaluated by a professional and include using a face mount hanger, and notching the beam or cutting the subfloor to accommodate the top flange thickness.
- f. Built-up lumber (multiple members) must be fastened together to act as one unit to resist the applied load (excluding the connector fasteners). This must be determined by the Designer/Engineer of Record.

Additional Instructions for the Installer for Hybrid (Steel-to-Wood) Connections

- a. Bolt holes into wood members shall be at least a minimum of ¹/₂₂" and no more than a maximum of ¹/₁₆" larger than the bolt diameter (per the 2015 NDS 12.1.3.2 and AISI S100 Table E3a, if applicable).
- b. Joist shall bear completely on the connector seat, and the gap between the joist end and the header shall not exceed ½" per ICC-ES AC261, ASTM D1761 and ASTM D7147 test standards (unless specifically noted otherwise).
- c. For holdowns, anchor bolt nuts should be finger-tight plus ½ to ½ turn with a hand wrench, with consideration given to possible future wood shrinkage. Care should be taken to not over-torque the nut and impact wrenches should not be used. This may preload the holdown.



SIMPSON Strong-Tie

Additional Instructions for the Designer for Hybrid (Steel-to-Wood) Connections

- a. Loads are based on the AISI S100 and the 2015 AF National Design Specifications (NDS), unless otherwise specified. Other code agencies may use different methodologies.
- Duration of load adjustments for fasteners into wood as specified by the code are as follows:

Do not alter installation procedures from those set forth in this catalog.

"FLOOR" and "DOWN" (100) — no increase for duration of load.
"SNOW" (115) — 115% of design load for 2-month duration of load.
"ROOF LOAD" (125) — 125% of design load for 7-day

duration of load. "EARTHQUAKE/WIND" (160) — 160% of design load for earthquake/wind loading.

- b. Some catalog illustrations show connections that could cause cross-grain tension or bending of the wood during loading if not sufficiently reinforced. In this case, mechanical reinforcement should be considered.
- c. Most of the allowable loads published in this catalog are for use when utilizing the traditional Allowable Stress Design (ASD)

methodology. A method for using Load and Resistance Factor Design (LRFD) for cold-formed steel is also included in AISI S100. When designing with LRFD, the nominal connector strength multiplied by the resistance factor must be used. If not listed or noted in a table footnote, contact Simpson Strong-Tie for the LRFD values of products listed in this catalog. A method for using Load and Resistance Factor Design (LRFD) for wood has been published in ASTM D5457. For more information, refer to the 2015 NDS Appendix N, which contains a conversion procedure that can be used to derive LRFD capacities. When designing with LRFD, reference lateral resistances must be used.

d. Pneumatic or powder-actuated fasteners may deflect and injure the operator or others. Unless otherwise noted, powder-actuated fasteners should not be used to install connectors. Pneumatic nail tools may be used to install connectors, provided the correct quantity and type of fasteners are properly installed in the fastener holes. Tools with fastener hole-locating mechanisms should be used. Follow the manufacturer's instructions and use the appropriate safety equipment. Over driving fasteners may reduce allowable loads. Contact Simpson Strong-Tie as needed.

General Information

Additional Important Information and General Notes for Allowable Anchorage Load Tables for SCB / MSCB, SC, SCW, SSB, FCB, FC and FSB Connectors

- 1. Allowable loads for #12–24 self-drilling screws and PDPAT powder-actuated fasteners are based on installation in minimum $\frac{3}{16}$ " thick structural steel with Fy = 36 ksi. It is the responsibility of the Designer to select the proper length fasteners based on the installation.
- 2. Allowable loads for Simpson Strong-Tie® PDPAT-62KP powderactuated "tophat" fasteners also apply to alternate fasteners with a minimum shank 0.157", a minimum head diameter of 0.300", a minimum allowable shear of 410 lb. and tension strength of 260 lb. for A36 steel, and a minimum allowable shear of 420 lb. and tension strength of 305 lb. for A572 or A992 steel per ESR-2138. "Tophat" fasteners are recommended to ensure adequate clamping force and consistent installations.
- 3. Allowable loads for Simpson Strong-Tie Titen[®] screws are based on installation in concrete with a minimum f'_c = 2,500 psi and a maximum f'_c = 4,000 psi. Reference the current Anchoring and Fastening Systems for Concrete and Masonry catalog for more information about Titen screws.
- 4. Allowable loads for welded connections require E70XX electrodes with a minimum throat size equal to the clip thickness. Welding shall be in compliance with AWS D1.3. Welding galvanized steel may produce harmful fumes; follow proper welding procedures and precautions.
- Allowable loads are for anchorage only. It is the responsibility of the Designer to verify the strength and stability of the structure for the loads imposed by the cold-formed steel framing connections.

General Info

Simpson Strong-Tie Company Inc. warrants catalog products to be free from defects in material or manufacturing. Simpson Strong-Tie Company Inc. products are further warranted for adequacy of design when used in accordance with design limits in this catalog and when properly specified, installed and maintained. This warranty does not apply to uses not in compliance with specific applications and installations set forth in this catalog, or to non-catalog or modified products, or to deterioration due to environmental conditions.

Simpson Strong-Tie® connectors are designed to enable structures to resist the movement, stress and loading that results from impact events such as earthquakes and high-velocity winds. Other Simpson Strong-Tie products are designed to the load capacities and uses listed in this catalog. Properly-installed Simpson Strong-Tie products will perform in accordance with the specifications set forth in the applicable Simpson Strong-Tie catalog. Additional performance limitations for specific products may be listed on the applicable catalog pages.

Due to the particular characteristics of potential impact events, the

specific design and location of the structure, the building materials used, the quality of construction, and the condition of the soils involved, damage may nonetheless result to a structure and its contents even if the loads resulting from the impact event do not exceed Simpson Strong-Tie catalog specifications and Simpson Strong-Tie connectors are properly installed in accordance with applicable building codes.

All warranty obligations of Simpson Strong-Tie Company Inc. shall be limited, at the discretion of Simpson Strong-Tie Company Inc., to repair or replacement of the defective part. These remedies shall constitute Simpson Strong-Tie Company Inc.'s sole obligation and sole remedy of purchaser under this warranty. In no event will Simpson Strong-Tie Company Inc. be responsible for incidental, consequential, or special loss or damage, however caused.

This warranty is expressly in lieu of all other warranties, expressed or implied, including warranties of merchantability or fitness for a particular purpose, all such other warranties being hereby expressly excluded. This warranty may change periodically · consult our website strongtie.com for current information.

Terms and Conditions of Sale

Product Use

Products in this catalog are designed and manufactured for the specific purposes shown, and should not be used with other connectors not approved by a qualified Designer. Modifications to products or changes in installations should only be made by a qualified Designer. The performance of such modified products or altered installations is the sole responsibility of the Designer.

Indemnity

Customers or Designers modifying products or installations, or designing non-catalog products for fabrication by Simpson Strong-Tie Company Inc. shall, regardless of specific instructions to the user, indemnify, defend and hold harmless Simpson Strong-Tie Company Inc. for any and all claimed loss or damage occasioned in whole or in part by non-catalog or modified products.

Bolt Diameter

Non-Catalog And Modified Products

Consult Simpson Strong-Tie Company Inc. for applications for which there is no catalog product, or for connectors for use in hostile environments, with excessive wood shrinkage, or with abnormal loading or erection requirements.

Non-catalog products must be designed by the customer and will be fabricated by Simpson Strong-Tie in accordance with customer specifications.

Simpson Strong-Tie cannot and does not make any representations regarding the suitability of use or load-carrying capacities of non-catalog products. Simpson Strong-Tie provides no warranty, express or implied, on non-catalog products. F.O.B. Shipping Point unless otherwise specified.

Conversion Charts

Metric Conversion					
Imperial	Metric				
1 in.	25.40 mm				

1 in.	25.40 mm	
1 ft.	0.3048 m	
1 lb.	4.448N	
1 Kip	4.448 kN	
1 psi	6,895 Pa	
		-

If Common Rafter Roof Pitch is...

mperial	Metric		in.	mm		Rise/Run	Slope					
1 in.	25.40 mm		3⁄8	9.5		1/12	5°					
1 ft.	0.3048 m		1⁄2	12.7		2/12	10°					
1 lb.	4.448N		5⁄8	15.9		3/12	14°					
1 10.	4.44014		3⁄4	19.1		4/12	18°					
1 Kip	4.448 kN		7⁄8	22.2		5/12	23°					
1 psi	6,895 Pa		1	25.4		6/12	27°					
						7/12	30°					
					→	8/12	34°					
Use th	ese Roof Pit	ch to	b Hip/V	alley Raf	ter	9/12	37°					
	itch convers			,		10/12	40°					
,	valley rafters that are skewed 45° right or left. All other skews will cause the slope											
ieit. F				that liste		12/12	45°					

Then Hip/Valley Rafter Roof Pitch becomes...

Rise/Run	Slope
1/17	3°
2/17	7°
3/17	10°
4/17	13°
5/17	16°
6/17	19°
7/17	22°
8/17	25°
9/17	28°
10/17	30°
11/17	33°
12/17	35°

US Standard Steel Gauge Equivalents in Nominal Dimensions

Min. Thick.	Design Thick.	Ref.	Thickness of Steel Sheets (in.)					
mil	in.	Ga. ²	Uncoated Steel	Galvanized Steel (G90)	ZMAX® (G185)			
229	0.2405	3	0.239	_				
171	0.1795	7	0.179	0.186	_			
118	0.1240	10	0.134	0.138	0.140			
111	0.1163	11	0.120	0.123	0.125			
97	0.1017	12	0.105	0.108	0.110			
68	0.0713	14	0.075	0.078	0.080			
54	0.0566	16	0.060	0.063	0.065			
43	0.0451	18	0.048	0.052	0.054			
33	0.0346	20	0.036	0.040	0.042			
27	0.0283	22	0.030	0.033	0.035			

1. Steel thickness may vary according to industry mill standards. 2. Gauge numbers shown are for reference only.



Understanding the Corrosion Issue

Many environments and materials can cause corrosion, including ocean salt air, fire retardants, fumes, fertilizers, preservative-treated wood, de-icing salts, dissimilar metals and more. Metal connectors, fasteners and anchors could corrode and lose load-carrying capacity when installed in corrosive environments or when installed in contact with corrosive materials.

The many variables present in a building environment make it impossible to accurately predict if, or when, corrosion will begin or reach a critical level. This relative uncertainty makes it crucial that specifiers and users are knowledgeable of the potential risks and select a product suitable for the intended use. It is also prudent that regular maintenance and periodic inspections are performed, especially for outdoor applications.

It is common to see some corrosion in outdoor applications. Even stainless steel can corrode. The presence of some corrosion does not mean that load capacity has been affected or that failure is imminent. If significant corrosion is apparent or suspected, then the framing members, fasteners and connectors should be inspected by a qualified engineer or qualified inspector. Replacement of affected components may be appropriate.

Some wood-preservative chemicals and fire-retardant chemicals and retentions pose increased corrosion potential and are more corrosive to steel connectors and fasteners than others. Testing by Simpson Strong-Tie has shown that ACQ-Type D is more corrosive than Copper Azole Type C, Micronized Copper Azole and CCA-C. At the same time, others have shown that the inorganic boron treatment chemicals, specifically SBX-DOT, are less corrosive than CCA-C.

Due to the many different chemical treatment formulations, chemical retention levels, moisture conditions and regional formulation variants, selection of fasteners has become a complex task. We have attempted to provide basic knowledge on the subject here, but it is important to fully educate yourself by reviewing our technical bulletins on the topic (**strongtie.com/info**) and also by reviewing information, literature and evaluation reports published by others.

Galvanic Corrosion

Galvanic corrosion occurs when two electrochemically dissimilar metals contact each other in the presence of an electrolyte (such as water) that acts as a conductive path for metal ions to move from the more anodic to the more cathodic metal. In the galvanic couple, the more anodic metal will corrode preferentially. The Galvanic Series of Metals table provides a qualitative guide to the potential for two metals to interact galvanically. Metals in the same group (see table) have similar electrochemical potentials. The farther the metals are apart on the table, the greater the difference in electrochemical potential, and the more rapidly galvanic corrosion will occur. Corrosion also increases with increasing conductivity of the electrolyte.

Good detailing practice, including the following, can help reduce the possibility of galvanic corrosion of fasteners:

- Use fasteners and metals with similar electrochemical properties
- Separate dissimilar metals with insulating materials
- · Ensure that the fastener is the cathode when dissimilar metals are present
- · Prevent exposure to and pooling of electrolytes

Galvanic Series of Metals

Corroded End (Anode)

Magnesium, Magnesium alloys, Zinc

Aluminum 1100, Cadmium,

Aluminum 2024-T4, Iron and Steel

Lead, Tin, Nickel (active), Inconel Ni-Cr alloy (active), Hastelloy alloy C (active)

Brasses, Copper, Cu-Ni alloys, Monel

Nickel (passive)

304 stainless steel (passive), 316 stainless steel (passive), Hasteloy alloy C (passive)

Silver, Titanium, Graphite, Gold, Platinum

Protected End (Cathode)

Treatment Use Categories and Exposure Conditions

Hydrogen-Assisted Stress-Corrosion Cracking

Some hardened fasteners may experience premature failure if exposed to moisture as a result of hydrogen-assisted

stress-corrosion cracking. These fasteners are recommended specifically for use in dry, interior locations.

The American Wood Protection Association (AWPA) identifies 12 Use Category designations (UC) for wood treatment chemicals that are based on protection of the wood material; the Use Categories are based on service conditions and environments and agents of deterioration. At the same time, the building codes require specific corrosion resistance for connectors and fasteners that are in contact with chemically-treated wood, and the corrosion resistance is independent of the service environments and treatments that are the basis of the AWPA Use Categories. From the building code perspective, fastener corrosion resistance is provided by hot-dip galvanization applied following ASTM A153, Class D, or by a corrosion-resistant base metal, such as stainless steel, silicon bronze or copper, regardless of exposure. Connectors in contact with preservative-treated wood require a minimum of ASTM A653, Type G185 zinc-coated galvanized steel, or equivalent. Some exceptions are provided in the International Code Council's (ICC) International Residential Code (IRC) for mechanical galvanization applied to screws. The International Building Code (IBC) has exceptions for plain carbon steel fasteners, nuts and washers in SBX/DOT and zinc borate preservative-treated wood in interior, dry environments.

The International Code Council – Evaluation Service (ICC-ES) implemented AC257 as a method to evaluate alternate corrosion resistance mechanisms for fasteners used in wood construction where hot-dip galvanization (ASTM A153, Class D) is used as the benchmark performance. Under AC257, fastener corrosion resistance is qualified for one or more of four exposure conditions with no salt exposure: (1) treated wood in dry service; (2) clean wood in a salt air dry-service environment; (3) treated wood in a wet-service condition with no salt exposure; and (4) general use with no limitations.

SIMPSON Strong-Tie

Coatings Available

Not all products are available in all finishes.

Contact Simpson Strong-Tie for product availability, ordering information and lead times.

Finish/Material	Description	Level of Corrosion Resistance
	Connectors	
Gray Paint	Water-based paint intended to protect the product while it is warehoused and in transit to the jobsite.	Low
Powder Coating	Baked-on paint finish that is more durable than our standard paint and produces a better-looking finished product.	Low
Galvanized	Standard (G90) zinc-galvanized coating containing 0.90 oz. of zinc per square foot of surface area (total both sides).	Low
G185	Galvanized (G185) 1.85 oz. of zinc per square foot of surface area (hot-dip galvanized per ASTM A653 total both sides). These products require hot-dip galvanized fasteners (fasteners which meet the specifications of ASTM A153). Products with a powder-coat finish over a ZMAX base have the same level of corrosion resistance.	Medium
HOTDIPDG GALVANIZED®	Products are hot-dip galvanized after fabrication (14 ga. and thicker). The coating weight increases with material thickness. The minimum average coating weight is 2.0 oz./ft. ² (per ASTM A123 total both sides). These products require hot-dip galvanized fasteners (fasteners which meet the specifications of ASTM A153). Anchor bolts are hot-dip galvanized per ASTM F2329.	Medium
Stainless Steel Type 316L Stainless Steel	Type 316L stainless steel is a nickel-chromium austenitic grade of stainless steel with 2-3% Molybdenum. Type 316L stainless steel is not hardened by heat treatment and is inherently nonmagnetic. It provides a level of corrosion protection suitable for severe environments, especially environments with chlorides. Type 316L stainless-steel fasteners are compliant with the 2012 and 2015 IBC and IRC.	High/Severe
	Fasteners	1
Electrocoating (E-Coat™)	Electrocoating utilizes electrical current to deposit the coating material on the fastener. After application, the coating is cured in an oven. Electrocoating provides a minimum amount of corrosion protection and is recommended for dry, non-corrosive applications only.	Low
Type 410 Stainless Steel with Protective Top Coat	Carbon martensitic grade of stainless steel which is inherently magnetic, with an added protective top coat. This material can be used in mild atmospheres and many mild chemical environments.	Medium
Mechanically Galvanized Coating, Class 55	Simpson Strong-Tie [®] Strong-Drive [®] SD Connector screws are manufactured with a mechanically-applied zinc coating in accordance with ASTM B695, Class 55 with a supplemental overcoat. These fasteners are compatible with painted and zinc-coated (G90 and ZMAX) connectors.	Medium
Double-Barrier Coating	Simpson Strong-Tie Strong-Drive SDS Heavy-Duty Connector screws are manufactured with two different finishes that together provide a level of corrosion protection that equals that provided by the previous HDG coating.	Medium
HOT DE CONTRACTOR CONT	Simpson Strong-Tie Strong-Drive Timber-Hex screws are hot-dip galvanized in accordance with ASTM A153, Class C. Hot-dip galvanized fasteners have a minimum average of 1.25 oz./ft. ² of zinc coating. Hot-dip galvanized fasteners are compliant with the 2012 and 2015 IRC (R317.3) and IBC.	High/Severe

See Corrosion Information for more specific performance and application information on these finishes.

Simpson Strong-Tie General Recommendations

Simpson Strong-Tie has evaluated the AWPA (American Wood Protection Association) Use Categories (AWPA U1-16) and the ICC-ES, AC257 Exposure Conditions and developed from that evaluation a set of Corrosion Resistance Recommendations. These recommendations address the coating systems and materials used by Simpson Strong-Tie for connector and fastener products.

Dry-service (or damp-service) environments lead to wood moisture contents less than or equal to 19%. The corrosion potential, even in chemically-treated wood, is reduced in these conditions. These conditions are typical of AWPA UC1 and UC2 for wood treatment and AC257 Exposure Condition 1. See the Corrosion Resistance Classification Table for the Simpson Strong-Tie assessment of corrosion needs in these conditions. The AC257 Exposure Condition 2 reflects the presence of air-borne salt in a dry-service environment and corrosion hazard to exposed metal surfaces; it does not include effects of treatment chemicals.

Outdoor environments are generally more corrosive to steel either because the moisture exposure is elevated (greater than 19%) and/or the treatment chemical-retention level is higher than for interior service. The AWPA classifies exterior above-ground treatments as Use Categories UC3 (A and B) depending on moisture run-off; and for ground-contact levels of protection, it has Use Categories UC4 (A-C). ICC-ES considers the exterior exposure to be limited by the type of chemicals and retention level of the chemicals in the qualification testing and whether the exposure includes salt exposure. In general, The AC257 Exposure Condition 3 includes AWPA Use Categories UC1 (interior dry) to UC4A (exterior ground contact, general use).

Types 316/305/304 stainless steel, copper, silicon bronze and hot-dip galvanized (Class-C) are the most effective protection against corrosion risk, where Type 316 is the best choice for salt marine and chloride-

containing environments regardless of treatment chemicals or wood species. If you choose to use hot-dip galvanized (Class-D), mechanically-galvanized (C3, N2000, or Class 55), double-barrier or Quik Guard[®] coated fasteners on outdoor projects (e.g., a deck), you should periodically inspect the fasteners or have a professional inspection performed, and regular maintenance is a good practice. See the Corrosion Resistance Classifications Table for the Simpson Strong-Tie assessment of the corrosion resistance associated with materials and coatings and an appropriate level of corrosion resistance for various environments.

Due to the many variables involved, Simpson Strong-Tie cannot provide estimates of service life of connectors and fasteners. We suggest that all users and specifiers obtain recommendations on corrosion from the treated wood supplier or for the type of wood used. As long as Simpson Strong-Tie recommendations are followed, Simpson Strong-Tie stands behind its product performance and our standard warranty applies (p. 17).

Simpson Strong-Tie does not recommend painting stainless-steel fasteners or hardware. The reason behind this recommendation is that sometimes painting can facilitate corrosion. Stainless steel is "stainless" because it forms a protective chromium oxide film on the surface by passive oxidation with air. The paint film on the stainless steel surface may be imperfect or it can be injured during service, and in either case the metal may be exposed. Microscopic-sized film imperfections and scratches facilitate collection of dirt and water that can be stagnant and degrade or block the passive formation of the protective chromium oxide film. When this happens, crevice corrosion can initiate. Crevice corrosion eventually becomes visible as a brown stain or as red rust. This is the reason that painting usually does not improve corrosion resistance of stainless steel.

Guidelines for Selecting Corrosion-Resistant Connectors and Fasteners

Evaluate the Application

Consider the importance of the connection.

Evaluate the Exposure

Consider these moisture and treatment chemical exposure conditions:

- **Dry Service:** Generally interior applications and includes wall and ceiling cavities, raised floor applications in enclosed buildings that have been designed to prevent condensation and exposure to other sources of moisture. Prolonged exposure during construction should also be considered, as this may constitute a Wet Service or Elevated Service Condition.
- Wet Service: Generally exterior construction in conditions other than Elevated Service. These include Exterior Protected and Exposed and General Use Ground Contact as described by the AWPA UC4A.
- Elevated Service: Includes fumes, fertilizers, soil, some preservative-treated wood (AWPA UC4B and UC4C), industrial zones, acid rain and other corrosive elements.
- Uncertain: Unknown exposure, materials or treatment chemicals.
- Ocean/Water Front: Marine environments that include airborne chlorides and some splash. Environments with de-icing salts are included.
- Treatment Chemicals: See AWPA Use Category Designations. The preservative-treated wood supplier should provide all of the pertinent information about the wood being used. The information should include Use Category Designation, wood

species group, wood treatment chemical and chemical retention. See appropriate evaluation reports for corrosion effects of treatment chemicals and fastener corrosion resistance recommendations.

Fire-Retardant-Treated (FRT) Wood: Metal connectors in contact with FRT wood in dry service applications may generally be uncoated, painted or galvanized G90 zinc-coated steel. Refer to the FRT wood manufacturer's recommendations for fastener and connector protection requirements. The 2015 IBC Section 2304.10.5.4 and 2015 IRC Section R317.3.4 refer to the manufacturer's recommendations for fastener corrosion requirements. In the absence of recommendations from the manufacturer, the code requires fasteners to be hot-dip galvanized, stainless steel, silicon bronze or copper. Fastener shear and withdrawal allowable loads may be reduced in FRT lumber. Refer to the FRT manufacturer's evaluation report for reduction factors.

Use the Simpson Strong-Tie[®] Corrosion Classification Table

If the treatment chemical information is incomplete, Simpson Strong-Tie recommends the use of a 300-series stainless-steel product. If the treatment chemical is not shown in the Corrosion Classification Table, then Simpson Strong-Tie has not evaluated it and cannot make any recommendations other than the use of coatings and materials in the Severe category. Manufacturers may independently provide test results of other product information; Simpson Strong-Tie expresses no opinion regarding such information.

Corrosion Resistance Recommendations

Low	Medium	High	Severe
Phosphate (gray, black), Clear (bright) zinc (ASTM F1941), Heavy electro-galvanized (ASTM A641 – Class 1), Yellow zinc (ASTM F1941), Electrocoat (E-Coat [™]), Type 410 stainless steel	Mechanically galvanized (AS 3566.2-C3, N2000, ASTM B695 – Class 55), Quik Guard® coating, Hot-dip galvanized (ASTM A153 – Class D), Double-barrier coating, Type 410 stainless steel with protective top coat	Type 304 stainless steel, Type 305 stainless steel	Type 316 stainless steel, Hot-dip galvanized (ASTM A153 – Class C), Silicon bronze, Copper
	Conne	ctors	
Simpson Strong-Tie [®] gray paint, Powder coating, Standard G90 zinc coating	Powder coating, Hot-dip galvanized		Type 316L stainless steel

Corrosion Resistance Classifications

		Material to Be Fastened											
	Untreated	Preservative-Treated Wood											
Environment	Wood or Other Material	SBX-DOT Zinc Borate	Chemical Retention ≤ AWPA, UC4A	Chemical Retention > AWPA, UC4A	ACZA	Other or Uncertain	FRT Wood ⁹						
Dry Service	Low	Low	Low	High	Med	High	Med						
Wet Service	Med	N/A	Med	High	High	High	High						
Elevated Service	High	N/A	Severe	Severe	High	Severe	N/A						
Uncertain	High	High	High	Severe	High	Severe	Severe						
Ocean/Water Front	Severe	N/A	Severe	Severe	Severe	Severe	N/A						

1. These are general guidelines that may not consider all application criteria. Refer to product-specific information for additional guidance.

2. Type 316/305/304 stainless-steel products are recommended where preservativetreated wood used in ground contact has a chemical retention level greater than those for AWPA UC4A; CA-C, 0.15 pcf; CA-B, 0.21 pcf; micronized CA-C, 0.14 pcf; micronized CA-B, 0.15 pcf; ACQ-Type D (or C), 0.40 pcf.

3. Testing by Simpson Strong-Tie following ICC-ES AC257 showed that mechanical galvanization (ASTM B695, Class 55), Quik Guard® coating and double-barrier coating will provide corrosion resistance equivalent to hot-dip galvanization (ASTM A153, Class D) in contact with chemically-treated wood in dry-service and wet-service exposures (AWPA UC1-UC4A, ICC-ES AC257 Exposure Conditions 1 and 3) and will perform adequately subject to regular maintenance and periodic inspection.

- 4. Mechanical galvanizations C3 and N2000 should not be used in conditions that would be more corrosive than AWPA UC3A (exterior, above ground, rapid water run off).
- 5. If uncertain about Use Category, treatment chemical or environment, use Types 316/305/304 stainless steel, silicon bronze or copper.

6. Some treated wood may have excess surface chemicals making it potentially more corrosive than wood with lower retentions. If this condition is suspected, use Type 316/305/304 stainless steel, silicon bronze or copper fasteners.

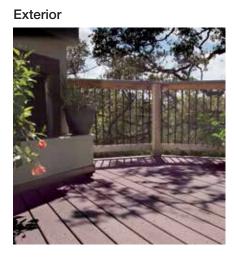
7. Type 316 stainless-steel, silicon bronze and copper fasteners are the best recommendation for ocean-salt air and other chloride-containing environments. Hot-dip galvanized fasteners with at least ASTM A153, Class C protection can also be an alternative for some applications in environments with ocean air and/or elevated wood moisture content.

8. Some woods, such as cedars, redwood and oak, contain water-soluble tannins and are more susceptible to staining when in contact with metal connectors and fasteners. According to the California Redwood Association (calredwood.org), applying a quality finish to all surfaces of the wood prior to installation can help reduce the amount of staining, which in redwood, for example, is caused by surface tannins leaching out during rains.

9. Fasteners in contact with FRT lumber shall be hot-dip galvanized, stainless steel, silicon bronze or copper unless recommended otherwise by the FRT manufacturer. Some FRT manufacturers permit low-resistant finishes for interior dry conditions. Fastener shear and withdrawal capacities may be reduced in FRT lumber. Refer to the FRT manufacturer's code report for reduction factors.

Interior Dry



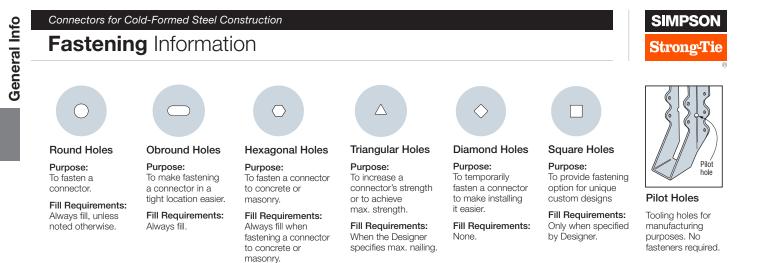


Severe



SIMPSON

Strong-T



To achieve the loads shown in this catalog, the Designer must verify that the self-drilling screws used for connector installation have P_{ss}/Ω and P_{ts}/Ω values greater than or equal to the values tabulated in this table.

Hex head screws shown are required for connectors in this catalog. Where sheathing or finishes will be applied over the screws and low-profile heads are needed (such as with bracing connectors, hurricane ties and stud-plate ties), the Designer is to ensure that the minimum screw head diameter complies with ASME B18.6.4.

Minimum ASD Loads for Screws (lb.)

Screw No. Designation	Nominal Diameter d	Washer Diameter d _w	Allowable Screw Shear	AII	Allowable Shear Connection Strength (Pns/Ω, Pss/Ω) Allowable Allowable Steel Thickness mil (ga.) Screw Tension Chroneth Screw mil (ga.)						e, P _{ts} /Ω) nickness			
	(in.)	(in.)	Strength (P _{SS} /Ω)	33–33	43-43	54–54	68–68	97–97	Strength (P _{ts} /Ω)	33	43	54	68	97
				(20–20)	(18–18)	(16–16)	(14–14)	(12–12)		(20)	(18)	(16)	(14)	(12)
#8	0.164	0.318	470	150	235	335	335	335	605	70	95	170	215	305
#10	0.190	0.375	540	165	255	510	540	540	820	75	110	200	250	355
#12	0.216	0.375	840	175	260	545	730	755	845	75	110	215	285	405
#14	0.242	0.500	1,045	180	180 285 555 795 825			1,220	85	120	215	295	455	

1. Allowable loads are per AISI S-100 and are for use when utilizing the traditional Allowable Stress Design methodology. The tabulated loads may be multiplied by a Factor of Safety (Ω) of 3 to determine the screw nominal strength. The LRFD load may be determined by multiplying the nominal screw load by a Resistance Factor (\$\phi\$) of 0.50.

2. Allowable loads may not be increased for wind or seismic load unless otherwise noted.

3. Allowable loads are based on cold-formed steel members with a minimum yield strength, Fy, of 33 ksi and tensile strength, Fu, of 45 ksi for 43 mil (18 ga.) and thinner, and a minimum yield strength of 50 ksi and tensile strength of 65 ksi for 54 mil (16 ga.) and thicker.

4. Allowable loads are based on design steel thickness for 33 mil = 0.0346", 43 mil = 0.0451" 54 mil = 0.0566", 68 mil = 0.0713", and 97 mil = 0.1017" per AISI S201 Product Data, Table B2-1.

5. Self-drilling tapping screw fasteners for steel-to-steel connections used for connectors in this catalog shall be in compliance with ASTM C1513.

6. Minimum required screw length is the greater of 3/4" and the minimum length required for the screw to extend through the steel connection a minimum of (3) exposed threads per AISI S200-12 General Provisions Standard, Sect. D1.3.

7. Screw diameters per AISI S200-12 Commentary Table C-E4-1.

8. Size 1/4"-diameter self-tapping screws may be substituted for #14 screws.

Screw Suitability

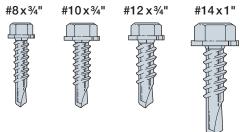
22

Screw Point	Screw	Maximum Material Thickness								
Туре	Size	(in.)	(mm)							
	#6	0.100	2.54							
#2	#8	0.100	2.54							
	#10	0.100	2.54							
	#7	0.125	3.18							
	#8	0.140	3.56							
#3	#10	0.175	4.45							
	#12	0.210	5.33							
	#14	0.220	5.59							
#4	#12	0.250	6.35							
#4	#14	0.250	6.35							
#5	#12	0.500	12.70							
#3	#14	0.500	12.70							

1. Total thickness of all steel, including any spacing between layers.

2. Drill and tap capacities may vary.

3. Table is guideline only; see individual product for specific maximum material thickness.



Shown Actual Size

See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

CFS Framing Member

Mil	Gauge	Design T	hickness	Minimum Thickness		
IVIII	Gauge	(in.)	(mm)	(in.)	(mm)	
18	25	0.0188	0.48	0.0179	0.45	
27	22	0.0283	0.72	0.0269	0.68	
30	20 (drywall)	0.0312	0.79	0.0296	0.75	
33	20 (structural)	0.0346	0.88	0.0329	0.84	
43	18	0.0451	1.14	0.0428	1.09	
54	16	0.0566	1.44	0.0538	1.37	
68	14	0.0713	1.81	0.0677	1.72	
97	12	0.1017	2.58	0.0966	2.45	

1. One "mil" is 1/1000 (0.001) of an inch. Mil thickness measures the uncoated base material.

CFS

GET THERE QUICKER!

With Simpson Strong-Tie[®] CFS Designer[™] Software

New for 2017 – CFS Designer Version V2.0!

CFS Designer V2.0 now includes a module that empowers users to design multi-story load-bearing wall systems. With this new time-saving feature engineers can design up to 8-stories of wall framing in a matter of minutes. The design tool automates the bookkeeping for transferring loads from one level to another, and it also checks up to ten different load combinations for each stud.



Stacked walls design module automates load-bearing framing for platform and ledger framing

Additional Key Features:

- Enables input of a complicated array of axial loads, distributed loads, sloped loads, and point loads.
- Supports 2012, 2010, 2007 and 2004 AISI Specifications
- Member design includes single stud and track sections, z-sections, box-sections, I-Sections, built-up stud and track sections, and HSS sections per AISC 13th Edition.
- Automates the selection of Simpson Strong-Tie SUBH bridging connectors, and SCB, SCW, SSB, and FCB curtain-wall connectors.



Please visit

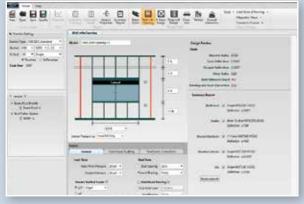
strongtie.com/cfsdesigner to:

- View complete system features
- Compare to LGBEAMER[™] software
- Test drive the demo version
- View release notes
- Purchase and download CFS Designer[™] software

Efficient, accurate, AISI-compliant design of cold-formed steel (CFS) structures is made possible by Simpson Strong-Tie[®] CFS Designer[™] software. Powerful design tools automate common CFS systems, complicated AISI design provisions, complex loading scenarios and more. A modern development platform and intuitive user interface enable fast input and simplify file management, as multiple systems can be saved within a single job file. Output is generated in PDF files that can be saved separately, if needed.



Beam-column design tool models up to 3-span beams with overhangs at each end



Design automation tools for framed wall openings, x-braced and sheathed shearwalls, floor joists, rafters and stacked walls

System Requirements

- 1. Microsoft Windows® 2003 or newer
- 2. Microsoft .NET Framework 4.0 Client Profile
- 3. Adobe Reader 9.0
- 4. Display Resolution 1280 x 768

Deflection Connectors

Innovative Solutions for Curtain-Wall Framing

SUBH

SIMPSON Strong-Tie **Deflection Connectors**

Simpson Strong-Tie has developed a line of connectors for use with curtain-wall steel stud framing. Curtain-wall projects require a variety of connectors that provide a load path from the curtain wall to the primary structure for wind loads, seismic loads and dead loads. Slide-clip connectors enable the structural building frame to deflect independently of the curtain-wall system. Fixed-clip connectors support the dead load of a curtain wall from the structural frame. Fixed clips have the added benefit of providing connector solutions for load-bearing walls and for roof systems utilizing steel trusses and rafters.

Our connectors for curtain-wall construction accommodate many different bypass framing applications in a variety of stand-off conditions. We also offer connectors for head-ofwall and strut applications.

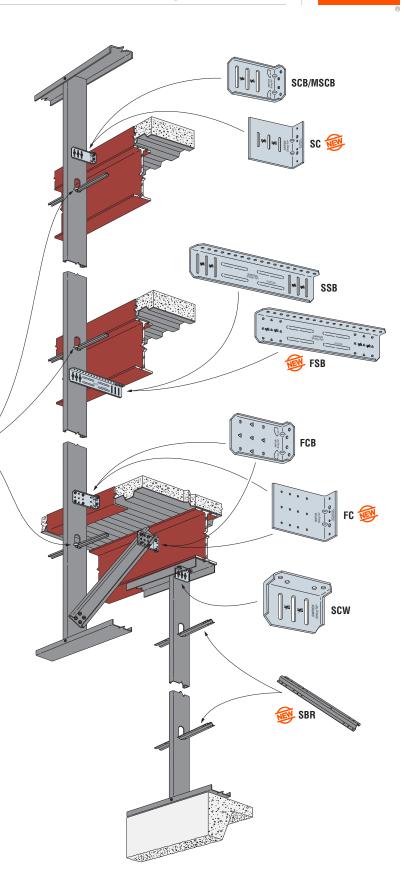
Tailored to Your Design

Our standard slide clips accommodate either ¾" or 1" of both upward and downward movement, equivalent to an L/360 live-load deflection for a 30' span. Our standard clips also accommodate stand-offs as large as 12¼". For deflections greater than 1", or stand-offs greater than 12¼", Simpson Strong-Tie can provide custom clips to suit most framing needs (see p. 56).

Complete, Tested Solutions

Designers of curtain walls will often know the capacity of a connector, but since the capacity does not take into account the way in which the connector is anchored to the supporting structure, the Designer must then manually calculate this important aspect of the connection design. These calculations are complicated by considerations of eccentric and prying forces that often exist but are difficult to predict. Through comprehensive testing Simpson Strong-Tie provides total, code-listed connector solutions. Our testing extends from the capacity of the connector and its attachment to the framing, to the anchorage of the connector to the primary structure. By providing complete data on the entire connection system, we save the Designer time and ensure that all forces, including eccentric and prying forces, are adequately considered.

As with all Simpson Strong-Tie[®] products, our slide-clip and fixed-clip connectors for curtain-wall steel stud framing carry our promise of quality and performance, and are backed by prompt, knowledgeable service.





Deflection Connectors

This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The SCB/MSCB slide-clip connectors are high-performance connectors for bypass framing applications designed to reduce

design time and overall installed cost. Various anchorage methods have been tested, and the resulting allowable loads eliminate the need to design connector anchorage. The SCB/MSCB can accommodate applications that typically require two parts with a single connector, reducing material and labor cost. These connectors are manufactured in five different lengths to accommodate a variety of stand-off conditions and steel-stud sizes.

Features:

- Provides a full 1" of both upward and downward movement
- · The precision-manufactured shouldered screws provided with the SCB/MSCB connector are designed to prevent overdriving and to ensure the clip functions properly
- · Strategically placed stiffeners, embossments and anchor holes maximize connector performance
- Simpson Strong-Tie® No-Equal stamps mark the center of the slots to help ensure correct shouldered-screw placement

Material: SCB - 54 mil (16 ga.); MSCB - 68 mil (14 ga.)

Finish: Galvanized (G90)

Installation:

- Use the specified type and number of anchors.
- Use the specified number of XLSH34B1414 #14 shouldered screws (included). Install shouldered screws in the slots adjacent to the No-Equal stamp.
- Use a maximum of one screw per slot.

Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

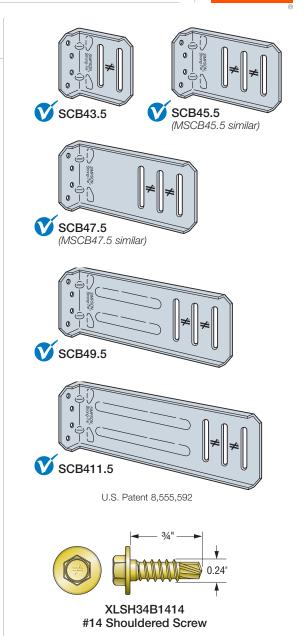
SCB43.5-KT contains:

- 25 connectors
- (55) XLSH34B1414 #14 shouldered screws

SCB45.5-KT, MSCB45.5-KT, SCB47.5-KT, MSCB47.5-KT, SCB49.5-KT. and SCB411.5-KT contain:

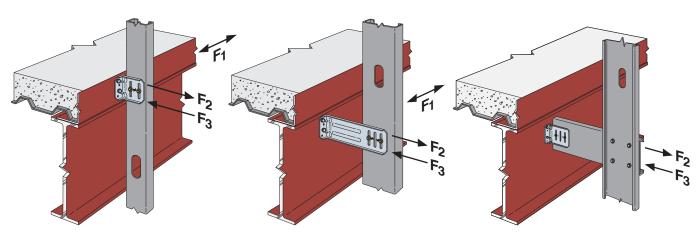
- 25 connectors
- (83) XLSH34B1414 #14 shouldered screws

Note: Replacement #14 shouldered screws for SCB/MSCB connectors are XLSH34B1414-RP83



SIMPSON

Strong



Typical SCB/MSCB Installation

SCB/MSCB Installation at Fascia Beam

Typical SCB/MSCB Installation with Stud Strut

SCB/MSCB Allowable Connector Loads

	Connector		No. of #14						Stud Th	ickness											
Model No.	Material Thickness	L (in.)	Shouldered	33	mil (20 g	ia.)	43	mil (18 g	a.)	54	mil (16 g	a.)	68	mil (14 g	a.)	Code Ref.					
	mil (ga.)		Screws	F1 ^{3,4}	F2 ²	F3 ²	F1 ^{3,4}	F ₂ ²	F3 ²	F1 ^{3,4}	F ₂ ²	F3 ²	F1 ^{3,4}	F ₂ ²	F3 ²	F3 ²					
SCB43.5	54 (16)	3½	2	100	520	520	155	610	690	185	760	975	185	760	975						
SCB45.5	E4 (1C)	5½	2 ¹	75	490	520	85	610	690	85	760	975	85	760	975	IP2, L8, FL					
30040.0	54 (16)	J/2	3	120	675	675	140	895	1,000	140	990	1,260	140	990	1,260						
MSCB45.5	45.5 68 (14) 5½		2 ¹	75	490	520	120	780	690	135	1,055	1,200	135	1,195	1,475	IP2, L8					
10136640.0		00 (14) 5/2	00 (14) 572	10.0 (14)	00 (14) 07	00 (14) 5/2	00 (14)	J/2	3	120	675	675	185	1,070	1,000	225	1,220	1,930	225	1,365	1,930
SCB47.5	54 (16) 7½	71/	2 ¹	55	490	520	55	610	690	55	760	945	55	760	945	IP2,					
30047.0		54 (16) 73	34 (10)	1 /2	3	70	675	675	70	895	1,000	70	990	1,260	70	990	1,260	L8, FL			
MSCB47.5	69 (14)	7½	2 ¹	70	490	520	75	780	690	85	1,055	1,200	85	1,195	1,475	IP2, L8					
WI3CD47.3	68 (14)	1 /2	3	90	675	675	90	1,070	1,000	110	1,220	1,930	110	1,365	1,930	IFZ, LO					
000405	E4 (10)	01/	2 ¹	40	490	520	40	690	690	40	760	945	40	760	945						
SCB49.5	54 (16)	9½	3	45	675	675	45	895	1,000	45	990	1,260	45	990	1,260	IP2,					
0004115	E4 (10)	4 4 1/	2 ¹	30	490	520	30	690	690	30	990	920	30	990	920	L8, FL					
SCB411.5	54 (16)	11½	3	35	675	675	35	860	1,000	35	990	1,260	35	990	1,260						

1. When the SCB or MSCB connector is used with two shouldered screws, the screws may be installed in any two slots.

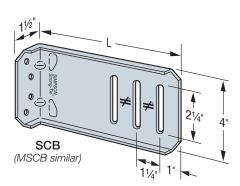
2. Allowable loads are based on clips installed with (4) #12–14 screws in the anchor leg. For other anchorage installations, the capacity of the connection system will be the minimum of the tabulated value and the allowable load

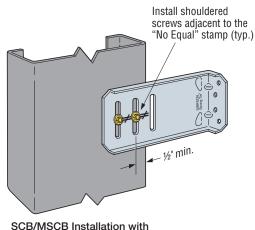
from the SCB/MSCB Allowable Anchorage Loads table on p. 28. 3. Anchorage to the supporting structure using welds or a minimum of (2) #12–24 self-drilling screws is required.

Anchorage to the supporting structure using weids or a minimum of (2) #12-24 self-drilling screws is required.
 Allowable loads are based on in-plane loads applied to the stud fasteners that are nearest the support with

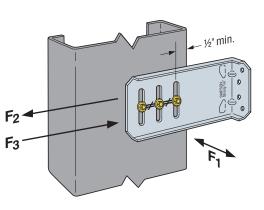
Childradie loads are based of himpliate loads applied to the studi fasteness that are nearest the support with complete rotational restraint at the studi. For a more extensive treatment of in-plane loads please refer to Simpson Strong-Tie engineering letter L-CF-CWCF1DIR17 at strongtie.com.

5. Reference pp. 63–67 for LRFD design strengths.





SCB/MSCB Installation wit Two Shouldered Screws



SCB/MSCB Installation with Three Shouldered Screws **Deflection Connectors**

SCB/MSCB Bypass Framing Slide-Clip Connector

SCB/MSCB Allowable Anchorage Loads

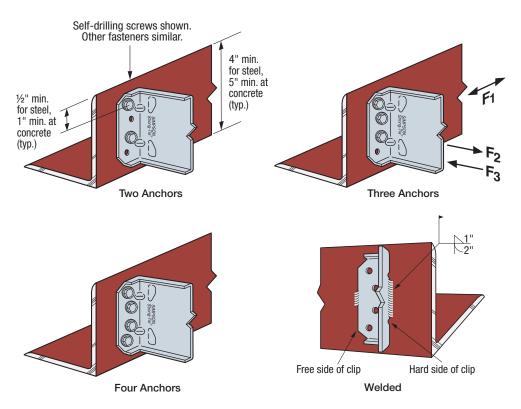
			F ₂ and F ₃		
Anchorage Type	Minimum Base Material	No. of Anchors	SCB	MSCB	
#12 self-drilling screws		2	795	1,020	
Simpson Strong-Tie [®] X Metal screws	A36 steel 346" thick	3	1,120	1,525	
XQ1S1214, X1S1214	710 11101	4	1,590	2,040	
Simpson Strong-Tie		2	1,115	1,150	
#12–24 x 11/4" Strong-Drive® XL Large-Head Metal screws	A36 steel 3/16" thick	3	1,645	1,725	
XLQ114T1224, XLQ114B1224-2K	, io unoit	4	2,230	2,300	
Simpson Strong-Tie		2	440	520	
0.157" x 5%" power-actuated fasteners	A36 steel 3⁄16" thick	3	585	780	
PDPAT-62KP	, io unoit	4	895	1,040	
Simpson Strong-Tie	A572 or	2	585	610	
0.157" x 5%" power-actuated fasteners	A992 steel	3	800	915	
PDPAT-62KP	3⁄16" thick	4	1,170	1,220	
Simpson Strong-Tie		2	380	380	
1⁄4" x 1 3⁄4" Titen® hex-head	Concrete f' _c = 2,500 psi	3	445	445	
masonry screws TTN25134H	,,	4	510	510	
Welded	A36 steel	Hard side: 2"	1,735	2,040	
E70XX electrodes	3⁄16" thick	Free side: 1"	1,700	2,040	

1. For additional important information, see General Information on p. 16.

2. Allowable loads are for clip anchorage only. The capacity of the connection system will be the minimum of the tabulated allowable anchorage loads the allowable load from the SCB/MSCB Allowable Connector Load table on p. 27.

3. Allowable loads for #12–24 self-drilling screws and PDPAT powder-actuated fasteners are based on installation in minimum $%_{e}^{t}$ thick structural steel with F_{y} = 36 ksi. PDPAT values are also provided for A572 steel. Values listed above maybe used where other thicknesses of steel are encountered provided that the fastener has equal or better tested values into thicker steel (see p. 16). It is the responsibility of the Designer to select the proper length fasteners based on the steel thickness installation.

4. For screw fastener installation into steel backed by concrete, predrilling of both the steel and the concrete is suggested. For predrilling use a maximum 3/6"-diameter drill bit.



Ideal for high-seismic areas, Simpson Strong-Tie® SC connectors are the optimal solution for slide-clip bypass framing. SC clips are often welded to the structure in high-seismic zones, but they also feature anchorage holes so that concrete screws or powder-actuated fasteners can be used to attach the clip to the structure. In addition to anchorage versatility, the SC clips include "No-Equal" stamps at the center of the slots to ensure proper shouldered screw placement. SC connectors are manufactured using heavy-duty 10- and 12-gauge steel to provide exceptional resistance to in-plane seismic load.

Features:

- The clips come in lengths of 3¹/₂", 6" and 8" for use with 3⁵/₈", 6" and 8" studs, respectively
- The maximum stand-off distance is 1" for 3%" studs and 11/2" for 6" and 8" studs
- Provides a full 3/4" of both upward and downward deflection
- Embossments in the bend line provide increased strength and stiffness in the F_1 and F_2 load directions, but are positioned towards the center of the clip so that 1 ½" long welds can be applied at the top and bottom of the clip
- Prepunched large-diameter anchor holes accommodate ¼"-diameter concrete screws like the Simpson Strong-Tie Titen HD[®]
- Prepunched small-diameter anchor holes accommodate powder-actuated fasteners like the 0.157"-diameter Simpson Strong-Tie PDPAT or the #12 self-drilling Simpson Strong-Tie Strong-Drive® XL large-head metal screw
- Precision-manufactured shouldered screws, provided with SC connectors, are designed to prevent overdriving and to ensure the clip functions properly

Material: 50 ksi

Finish: Galvanized (G90)

Installation:

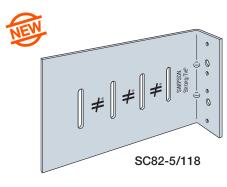
- Use the specified type and number of anchors.
- Use the specified number of XLSH78B1414 #14 shouldered screws (included). Install the screws in the slots adjacent to the "No-Equal" stamps.
- Use one shouldered screw per slot (maximum).

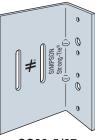
Codes: See p. 11 for Code Reference Key Chart

0	•				
Model No.	Ordering SKU	Thickness mil (ga.)	L (in.)	A (in.)	B (in.)
SC32-5/97	SC32-5/97-KT25	97 (12)	31⁄2	7⁄8	11⁄4
SC62-5/97	SC62-5/97-KT25	97 (12)	6	1 1⁄8	1½
SC62-5/118	SC62-5/118-KT25	118 (10)	6	1 1/8	1½
SC82-5/118	SC82-5/118-KT25	118 (10)	8	1%	1½

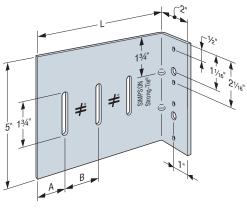
Ordering Information and Dimensions

1. Each box contains (25) connectors and enough shouldered screws for installation. 2. Replacement #14 shouldered screws for SC connectors are XLSH78B1414-RP83.

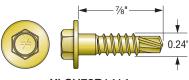




SC32-5/97



SC62-5/97, SC62-5/118



XLSH78B1414 #14 Shouldered Screw for Attachment to Stud Framing (included)

SIMPSON

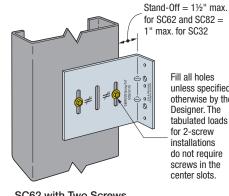
Strong-Tie

SC Allowable Connector Loads

		Fastener	s to Stud	Allowable Load				
	Stud	Allowable	No. of	F	1			
Model No.	Thickness mil (ga.)	Pullout per Single #14 Shouldered Screw	#14 Shouldered Screws	1" Stand-Off	1½" Stand-Off	F2	F3	Code Ref.
SC32-5/97			2	170	—	585	715	
SC62-5/97			2	100	115	585	715	
3002-3/3/			3	115	130	880	1,070	
SC62-5/118	33 (20)	100	2	100	115	585	710	
3002-3/110			3	115	130	880	1,070	
SC82-5/118			2	115	130	585	710	
3002-3/110			4	115	130	1,170	1,425	
SC32-5/97			2	220		765	930	
SC62-5/97			2	135	155	765	930	
0002-0/01			3	150	175	1,145	1,395	
SC62-5/118	43 (18)	145	2	135	155	765	930	
3002-3/110			3	150	175	1,145	1,395	
SC82-5/118			2	150	175	765	930	
3662-5/110			4	150	175	1,525	2,125	
SC32-5/97			2	300	—	1,145	1,645	
SC62-5/97			2	255	295	1,145	1,645	
3002-3/9/			3	265	305	2,120	2,345	
SC62-5/118	54 (16)	270	2	255	295	1,405	1,685	170
3602-3/110			3	265	305	2,110	2,530	
SC82-5/118			2	260	300	1,405	1,685	
3002-3/110			4	260	300	2,810	3,370	
SC32-5/97			2	375	—	1,695	1,645	
SC62-5/97			2	320	370	1,695	1,645	
3002-3/3/			3	335	385	2,540	2,345	
SC62-5/118	68 (14)	410	2	330	380	2,165	2,040	
3602-3/110			3	345	395	3,250	3,060	
SC82-5/118			2	325	375	2,165	2,085	
3662-3/110			4	325	375	4,330	4,165	
SC32-5/97			2	540	—	1,695	1,645	
SC62-5/97			2	555	555	1,695	1,645	
0002-0/97			3	555	555	2,540	2,345	
SC62-5/118	97 (12)	725	2	555	555	2,165	2,040	
0002-0/110			3	635	635	3,250	3,060	
SC82-5/118			2	465	465	2,165	2,085	
0002-0/110			4	465	465	4,330	4,165	

F1 ii **†**-∳-∳ 2 F_3

Typical SC Installation



SC62 with Two Screws (SC82 similar)

unless specified otherwise by the tabulated loads

1. For additional important information, see General Information on p. 16.

2. SC Allowable Connector Loads are also limited by the SC Anchorage Load tables on pp. 31 and 32. Use the minimum tabulated values from the connector and anchorage load tables as applicable.

3. See illustration for fastener placement when using only two shouldered screws to the stud.

4. Tabulated F1 loads are based on assembly tests with the load through the centerline of the stud. Tested failure modes were due to screw pullout; therefore compare F_1 against F_p calculated per ASCE 7-10 Chapter 13 with a_p = 1.25 and R_p = 1.0.

5. F1 loads are based on maximum stand-off distances of 1" or 11/2" as shown. Other loads are applicable to a 1" stand-off for SC32 and 1" or 11/2" stand-off for SC62 and SC82.

6. At the bend line, the gross allowable plastic moment in the F1 load direction for 97 mil (12 ga.) and 118 mil (10 ga.) SC connectors are 395 in.-lb. and 675 in.-lb., respectively.

7. At a vertical slot, the net allowable plastic moment in the F1 load direction for 97 mil (12 ga.) and 118 mil (10 ga.) SC connectors are 260 in.-lb. and 440 in.-lb., respectively.

Deflection Connectors

SC Allowable Anchorage Loads to Steel

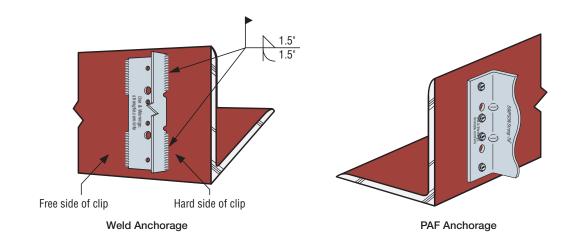
Anakaraga Tuna	Minimum	No. of	Allowat	ole Load
Anchorage Type	Base Material	Anchors	F ₁	F_2 and F_3
#12 self-drilling screws Simpson Strong-Tie® X Metal screws XQ1B1214, X1B1214	A36 steel ¾₅" thick	4	_	2,070
#12–24 x 1¼" Strong-Drive® XL Large-Head Metal screws XLQ114T1224, XLQ114B1224	A36 steel ¾6" thick	4	—	2,545
#14 self-drilling screws Simpson Strong-Tie E Metal screw E1B1414	A36 steel ¾6" thick	4	—	2,620
Simpson Strong-Tie 0.157" x %" powder-actuated fasteners PDPAT-62KP	A36 steel ¾6" thick	4	—	1,040
Simpson Strong-Tie 0.157" x %" powder-actuated fasteners PDPAT-62KP	A572 grade 50 or A992 steel ¾6" thick	4	_	1,710
Welded	A36 steel	Hard side: 3"	2,110	3,710
E70XX electrodes	⅔ı6" thick	Free side: 3"	2,110	0,110

1. For additional important information, see General Information on p. 16.

2. Allowable anchorage loads are also limited by the SC Connector Load Table on p. 30. Use the minimum tabulated values from the connector and anchorage load tables as applicable.

3. Allowable loads for #12–24 self-drilling screws and PDPAT powder-actuated fasteners are based on installation in minimum 3/6" thick structural steel with F_y = 36 ksi. PDPAT values are also provided for A572 steel. Values listed above may be used where other thicknesses of steel are encountered provided that the fastener has equal or better tested values into thicker steel (see p. 16). It is the responsibility of the Designer to select the proper length fasteners based on the steel thickness installation.

4. For screw fastener installation into steel backed by concrete, predrilling of both the steel and the concrete is suggested. For predrilling use a maximum %6"-diameter drill bit.





Allowable Titen HD® Anchorage Loads into Concrete with SC Clip

Simpson Strong-Tie 1⁄4" Titen HD Screw	Nominal Embedment Anchor Quantity				hedment Anchor Quantity T _C		Wind and in SD(l Seismic C A&B	Seismic in SDC C through F
Anchor	(in.)	and Size	(psi)	Direction	Uncracked Concrete	Cracked Concrete	Cracked Concrete ⁶		
			3,000	F ₁	335	240	280		
THDB25178H	15%	(0) 1/ " v 17/ "	3,000	F_2 and F_3	660	630	550 325		
100231700	1 78	(2) ¼" x 17/8"	4.000	F1	390	280	325		
			4,000	F_2 and F_3	760	725	635		
			2.000	F ₁	370	265	280 550 325		
THDB25234H	2½	(0) 1/ 1/ 03/ 1	3,000	F_2 and F_3	475	695			
100232340	∠ 1/2	(2) ¼" x 2¾" -	4,000 -	F ₁	430	305	360		
				F ₂ and F ₃	550	805	705		

 Allowable anchor capacities have been determined using ACI 318-14 Chapter 17 calculations with a minimum concrete compressive strength (f_c) of 3,000 and 4,000 psi in normal-weight concrete. Tabulated values shall be multiplied by a factor (λ_a) of 0.6 for sand light-weight concrete.

2. Edge distance is assumed to be $1\frac{1}{2}$ ", and end distance is $2\frac{5}{3}$ ".

3. Load values are for group anchors based on ACI 318, condition B, load factors from ACI 318-14 Section 5.3, no supplement edge reinforcement, Ψ_c , v = 1.0 for cracked concrete and periodic special inspection.

4. Allowable Stress Design (ASD) values were determined by multiplying calculated LRFD capacities by a conversion factor, Alpha (α), of 0.70 for seismic load and 0.6 for wind loads. ASD values for other combinations may be determined using alternate conversion factors.

 Tabulated allowable ASD loads for Wind and Seismic in SDC A&B are based on using wind conversion factors and may be increased by 1.17 for SDC A&B only.

6. Design loads shall include the over-strength factor per ASCE7 Section 12.4.3. For fasteners in exterior wall connection systems, $\Omega_0 = 1.5$ per Table 13.5-1.

7. Allowable loads for F_1 are based on the governing loading direction which is toward the end of slab.

8. For anchor subjected to both tension and shear loads, it shall be designed to satisfy following:

• For $N_a / N_{all} \le 0.2$, the full allowable load in shear is permitted.

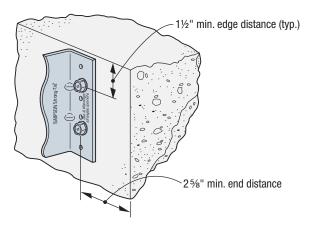
• For V_a / $V_{all} \le$ 0.2, the full allowable load in tension is permitted.

• For all other cases: N_a / N_{all} + V_a / V_{all} \leq 1.2 where: N_a = Applied ASD tension load

 N_a = Applied ASD tension load N_{all} = Allowable Anchorage Loads for Concrete table V_a = Applied ASD shear load

 $V_{all} =$ Allowable F₁ load from the SC Allowable Anchorage Loads for Concrete table

9. Tabulated allowable loads are based on anchorage only. The capacity of the connection system shall be the minimum of the allowable anchorage load and the SC Allowable Connector Loads.



Titen HD[®] Anchorage

SSB Bypass Framing Slide-Clip Strut Connector

The SSB connector is a versatile strut connector that is commonly used at the bottom of a steel beam to accommodate large stand-off conditions. It accommodates 1" of upward and 1" of downward movement.

Material: 54 mil (16 ga.)

Finish: Galvanized (G90)

Installation:

- Use the specified type and number of anchors.
- Use the specified number of XLSH34B1414 #14 shouldered screws (included). Install shouldered screws in the slots adjacent to the No-Equal stamp.
- Use a maximum of one screw per slot.
- If the SSB intrudes on interior space, it can be trimmed. The trimmed part shall allow an edge distance from the center of the nearest anchor to the end of the trimmed part of 1/2" or greater.

Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

SSB3.518-KT contains:

- Box of 25 connectors
- (83) XLSH34B1414 #14 shouldered screws

Note: Replacement #14 shouldered screws for SSB connectors are XLSH34B1414-RP83.

SSB Allowable Connector Loads

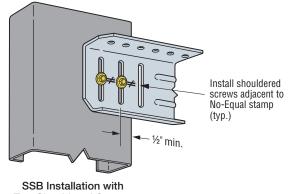
	Connector	No. of #14			Stud Th	ickness			
Model No.	Material Thickness	Shouldered	33 mil	(20 ga.)	43 mil	(18 ga.)	54 mil	(16 ga.)	F3 IP2,
	mil (ga.)	Screws	F ₂	F3	F ₂	F3	F ₂	F3	
CCD2 E10	E4 (16)	2 ¹	520	520	690	690	1,075	960	IP2,
SSB3.518	54 (16)	3	815	815	1,030	1,080	1,335	1,225	L8, FL

1. When the SSB connector is used with two shouldered screws, the screws may be installed in any two slots.

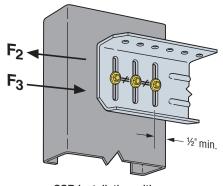
2. Allowable loads are based on clips installed with (3) #12-24 screws in the anchor leg. For other anchorage

installations, the capacity of the connection system will be the minimum of the tabulated value and the allowable load from the SSB Allowable Anchorage Loads table on p. 34.

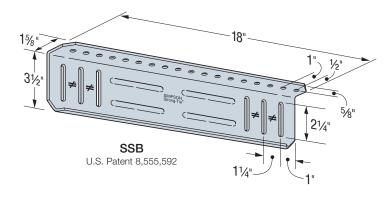
3. Reference pp. 63–67 for LRFD design strengths.

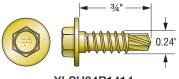


Two Shouldered Screws



SSB Installation with **Three Shouldered Screws**





XLSH34B1414 #14 Shouldered Screw

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

SSB Bypass Framing Slide-Clip Strut Connector

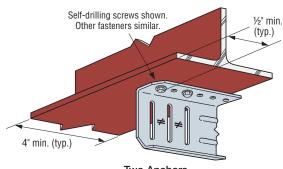
SSB Allowable Anchorage Loads

Anchorage Type	No. of Anchors	Allowable Load F ₂ and F ₃
#12-24 self-drilling screws ³	2	1,250
#12-24 Sell-utiliting Screws*	3	1,335
Simpson Strong-Tie® 0.157" PDPAT	2	1,320
powder-actuated fasteners	3	1,335
Welded	Hard side: 2" Free side: 1"	1,335

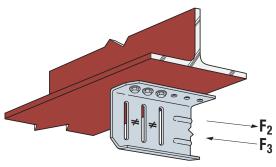
1. For additional important information, see General Information on p. 16.

2. Allowable loads are for clip anchorage only. The capacity of the connection system will be the minimum of the tabulated value and the allowable load from the SSB Allowable Connector Loads table on p. 33.

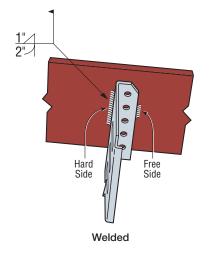
3. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



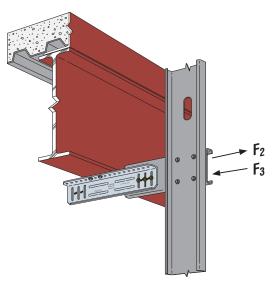
Two Anchors



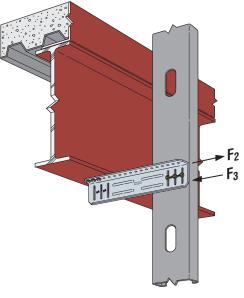
Three Anchors



SSB Anchor Layout



Typical SSB Installation with Stud Strut



Typical SSB3.518 Installation

Design Aid – SCB, SSB and FCB

Curtain-Wall Connector Stand-Off Distances

Madal Na	Fastener Dettern			Depth of Stud		
Model No.	Fastener Pattern	35%"	6"	8"	10"	12"
SCB43.5	А	0" to 1"	0" to 1"	0" to 1"	0" to 1"	0" to 1"
	В	1 1⁄8" to 23⁄4"	0" to 2¾"	0" to 2¾"	0" to 2¾"	0" to 2¾"
SCB45.5/MSCB45.5	C, D and E	—	0" to 1½"	0" to 1½"	0" to 1 1⁄2"	0" to 1 ½"
	В	37⁄8" to 43⁄4"	1 ½" to 4¾"	0" to 4¾"	0" to 4¾"	0" to 4¾"
SCB47.5/MSCB47.5	C, D and E	—	1 ½" to 3½"	0" to 31⁄2"	0" to 31⁄2"	0" to 31⁄2"
CCD 40 5	В	57%" to 634"	3½" to 6¾"	1 ½" to 6¾"	0" to 6¾"	0" to 6¾"
SCB49.5	C, D and E		31⁄2" to 51⁄2"	1 ½" to 5½"	0" to 51⁄2"	0" to 51⁄2"
000411.5	В	71⁄8" to 83⁄4"	5½" to 8¾"	31⁄2" to 83⁄4"	1 1⁄2" to 8 3⁄4"	0" to 8¾"
SCB411.5	C, D and E	_	5½" to 7½"	3½" to 7½"	1 ½" to 7 ½"	0" to 71⁄2"
SSB3.518/FSB3.518	B ²	0" to 121⁄4"	0" to 121⁄4"	0" to 121⁄4"	0" to 121⁄4"	0" to 121⁄4"
5583.518/F583.518	C, D and E^3		0" to 11"	0" to 11"	0" to 11"	0" to 11"
FOD 42 F	Min.4	0" to 1"	0" to 1"	0" to 1"	0" to 1"	0" to 1"
FCB43.5	Max. ⁴	0" to 1"	0" to 1"	0" to 1"	0" to 1"	0" to 1"
	Min.4	—	0" to 1½"	0" to 1 ½"	0" to 1 1⁄2"	0" to 1 ½"
FCB45.5	Max.4	—	0" to 1 ½"	0" to 1 ½"	0" to 1 1⁄2"	0" to 1½"
FOD 47 F	Min.4	_	1 ½" to 3½"	0" to 31⁄2"	0" to 31⁄2"	0" to 3½"
FCB47.5	Max.4		_	0" to 1"	0" to 1"	0" to 1"
	Min. ⁴	—	31⁄2" to 51⁄2"	1 1⁄2" to 5 1⁄2"	0" to 5½"	0" to 5½"
FCB49.5	Max.4		—		0" to 1"	0" to 1"
FOD 411 F	Min. ⁴	_	5½" to 7½"	31⁄2" to 71⁄2"	1 1⁄2" to 7 1⁄2"	0" to 7½"
FCB411.5	Max. ⁴	_	_	_	_	0" to 1"

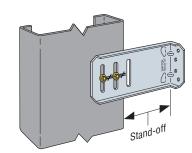
1. The stand-off is the distance from the interior flange of the stud to the face of the supporting structure (see illustration below). All dimensions assume fasteners are installed a minimum of ½" from the edge of stud, and the connector does not extend

beyond the exterior flange of the stud; "-" designates conditions where fastener pattern will not fit on stud.

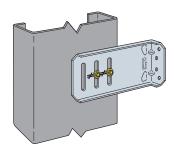
2. A tabulated maximum stand-off of 12^¼" is based on three anchors. For two anchors, maximum stand-off is 13^¼".

3. A tabulated maximum stand-off of 11" is based on three anchors. For two anchors, maximum stand-off is 12".

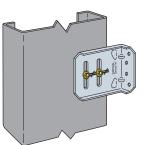
4. For FCB fixed connectors (pp. 47–49), minimum indicates fill all round holes, and maximum indicates fill all round and triangle holes. 5. Fastener patterns are as follows:



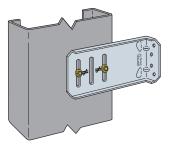
Stand-Off Distance



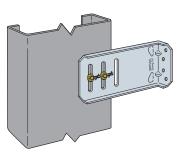
Pattern C Two screws installed in slots closest to the anchor leg



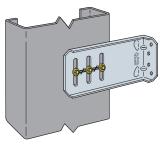
Pattern A Two screws installed with one in each slot



Pattern D Two screws installed in outer slots with center slot empty



Pattern B Two screws installed in slots farthest from the anchor leg



Pattern E Three screws

SCW Head-of-Wall Slide-Clip Connector

The SCW connectors offer 1" of upward and 1" of downward movement. They are primarily used in head-of-wall applications that require vertical movement relative to the structure. SCW connectors are often used to strengthen window and door jambs for projects that utilize slip track.

Material: 54 mil (16 ga.)

Finish: Galvanized (G90)

Installation:

Deflection Connectors

- Use the specified type and number of anchors.
- Use the specified number of #14 shouldered screws (included). Install shouldered screws in the slots adjacent to the No-Equal stamp.
- Use a maximum of one screw per slot.

Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

SCW3.25-KT contains:

Box of 25 connectors

55 XLSH34B1414 #14 shouldered screws

SCW5.5-KT contains:

- Box of 25 connectors
- 83 XLSH34B1414 #14 shouldered screws

Note: Replacement #14 shouldered screws for SCW connectors are XLSH34B1414-RP83.

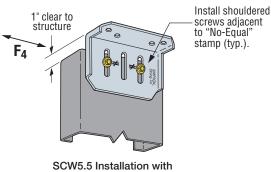
SCW Allowable Connector Loads

	Connector				Stud Thickness		a.) Ref.
Model No.	Connector Material Thickness mil (ga.)	W (in.)	No. of #14 Shouldered Screws	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	
	iiii (ga.)			F4	F4	F4	
SCW3.25	54 (16)	3¼	2	455	630	755	
COWE E	E4 (1C)	E 1/	2 ¹	455	630	995	IP2, L8, FL
SCW5.5	54 (16)	5½	3	455	630	1,220	,

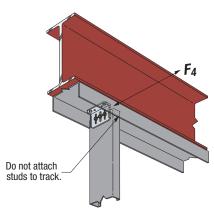
1. When the SCW5.5 connector is used with two shouldered screws, install screws in the outermost slots. 2. Allowable loads are based on clips installed with all holes in the anchor leg filled with #12-14 screws.

For other anchorage installations, the capacity of the connection system will be the minimum of the tabulated value and the allowable load from the SCW Allowable Anchorage Loads table on p. 37.

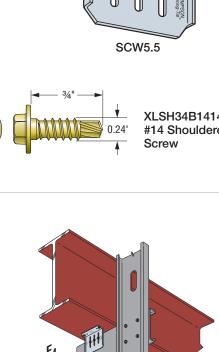
3. Reference pp. 63–67 for LRFD design strengths.



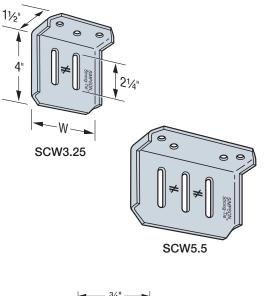
Two Shouldered Screws (Three shouldered screws and SCW3.25 similar)



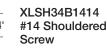
Typical SCW Installation at Stud



Typical SCW Installation with Stud Strut







SIMPSON

Strong-

36

SCW Head-of-Wall Slide-Clip Connector

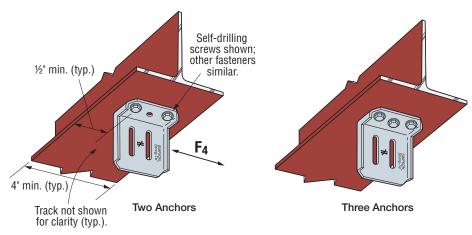
SCW Allowable Anchorage Loads

Model No.	Anchorage Type	No. of Anchors	Allowable Load F4
	#12-24 self-drilling screws ³	2	640
SCW3.25	#12-24 sen-uning screws	3	755
36W3.25	Simpson Strong-Tie [®] 0.157" PDPAT	2	520
	powder-actuated fasteners	3	560
	#10.04 colf drilling corours	2	1,200
SCW5.5	#12–24 self-drilling screws ³	4	1,220
30,00.5	Simpson Strong-Tie [®] 0.157" PDPAT	2	920
	powder-actuated fasteners	4	1,220

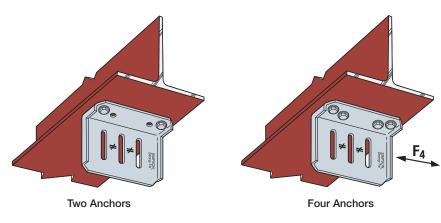
1. For additional important information, see General Information on p. 16.

 Allowable loads are for clip anchorage only. The capacity of the connection system will be the minimum of the tabulated value and the allowable load from the SCW Allowable Connector Loads table on p. 36.

3. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



SCW3.25 Anchor Layout



SCW5.5 Anchor Layout

DTC Head-of-Wall Slide-Clip Application

DTC clips are a cost-effective solution for light-duty, head-of-wall slide clip applications. The 1%" slot will allow 3%" movement in each direction.

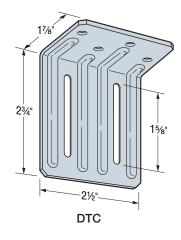
Material: 43 mil (18 ga.)

Finish: Galvanized (G90)

Installation:

- Use specified type and number of anchors per the installation drawing below
- Install (2) #8 screws centered in the vertical slots
- Once tightened, back-out screws ½ turn to ensure slip

Codes: See p. 11 for Code Reference Key Chart



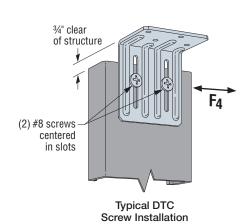
Model No.	Connector Material mil (ga.)	Fasteners	Anchorage	Stud Thickness mil (ga.)	Stud Steel Strength F _y (ksi)	Allowable Load F ₄	Code Ref.
				15 (25 EQ)	50	60	
				18 (25)	33	70	
		(0) // 0	(2) 0.157" PDPAT powder-actuated	19 (20 EQ)	65	0.0	
DTC	43 (18) (2) #8 self-drilling screws ⁴	fasteners or (2) #12	20 (20 EQ)	57	80	170	
		SCIEWS	self-drilling screws ⁴	30 (20 DW)	33	165	
				33 (20 STR)	33	170	
				43 (18)	33	215	

F4

4" min. (typ.)

Track not shown

for clarity (typ.).

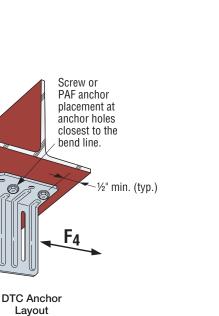


1. Allowable loads may not be increased for wind or seismic load.

Typical DTC Installation at Stud

2. Clips do not replace stud lateral or stability bracing. Design of bracing is the responsibility of the Designer.

- 3. It is the responsibility of the Designer to verify the adequacy of the stud. Allowable loads are based on clips installed an adequate distance away from penetrations, notches, ends of studs and other conditions that may affect the clip performance.
- 4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Do not attach

studs to track.

SCHA Slide-Clip Connectors for Horizontal Anchorage



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

SCHA connectors are an ideal solution for panelized or stick-frame construction where the CFS framing anchors to the top of a concrete floor slab or the bottom of a steel beam. The connector features a wide support leg to decrease eccentricity on anchors and provide a variety of anchorage options. The included SCVC vertical slider helps to strengthen the connector for the highest tension (F_2) and compression (F_3) loads in the industry.

Features:

- Provides a full 1" of both upward and downward movement
- Tabulated design values for anchorage help mitigate risk and provide ease of specification
- Either face of anchorage leg can be used against the support
- Accommodates stand-off distances up to 43/4"
- Can be used with 35/8", 4", 6" and 8" studs
- Prepunched anchor holes accommodate ¼"-diameter Titen HD® or other ¼"-diameter concrete screw anchors, and 0.157"-diameter powder-actuated fasteners such as the Simpson Strong-Tie® PDPAT-62KP
- Prepunched anchor holes also eliminate the need for pre-drilling and help ensure accurate anchor placement

Material: SCHA — 118 mil (10 ga., 33 ksi); SCVS — 97 mil (12 ga., 33 ksi)

Coating: Galvanized (G90)

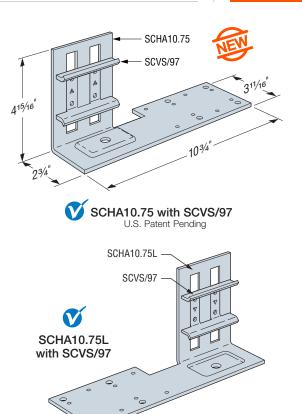
Installation:

- Use all specified fasteners and anchors. Note that the smaller diameter anchor holes are provided for PAF installation, and the larger diameter anchor holes are for ¼"-diameter concrete screw anchors.
- Ensure that the SCVS vertical slider is centered in the SCHA vertical slots by aligning the tic-marks adjacent to the triangle holes on the slider with the ≠ stamp on the SCHA clip.

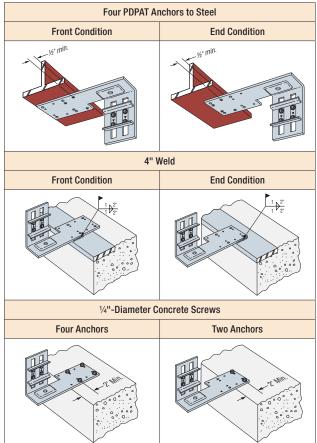
Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

- SCHA10.75-KT15 contains (15) SCHA10.75 connectors and (15) SCVS/97 sliders
- SCHA10.75L-KT15 contains (15) SCHA10.75L connectors and (15) SCVS/97 sliders



SCHA Anchorage Types and Conditions



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SCHA Slide-Clip Connectors for Horizontal Anchorage

SCHA Allowable Loads

Primary Structure	Anchora	ige		rs to Stud ng Screws	Stud Thickness	Maximum Stand-Off	All	owable Load (I	b.)	Code
Base Material	Qty./Type/Size	Condition	Min./ Max.	No. #12–14	mil (ga.)	Distance (in.)	F1 ⁷	F ₂	F ₃	Ref.
					33 (20)		150	645	490	
			Min.	4	43 (18)	2	195	860	610	
		Front	IVIIII.	4	54 (16)	۷	235	990	880	
		condition			68 (14)		235	990	880	
	(4) PAF –		Max.	6	54 (16)	2	350	1,300	1,045	
Structural steel A36	0.157" dia. x %" PDPAT-62KP		ινιαλ.	0	68 (14)	۷	350	1,495	1,045	
%ı6" thick minimum	or (2) welds –				33 (20)		105	625	470	
	2" length		Min.	4	43 (18)	43/4	110	830	570	
		End	IVIIII.		54 (16)	474	165	830	720	
		condition			68 (14)		165	830	720	
			Max.	6	54 (16)	43/4	350	1,060	775	
			iviax.	6	68 (14)	474	350	1,060	775	170
					33 (20)		105	625	470	
			Min.	4	43 (18)	2	110	830	570	
	(4) concrete screw anchors –	4 anchors	iviiri.		54 (16)	۷.	165	830	720	
	1/4" diameter ³				68 (14)		165	830	720	
Normal or			Max.	6	54 (16)	2	350	1,060	775	
lightweight			ινιαλ.	0	68 (14)	۷	350	1,060	775	
f' _c = 2,500 psi					33 (20)		105	625	470	
t°c = 2,500 psi minimum			Min.	4	43 (18)	43/4	105	830	570	
	(2) concrete screw anchors –	2 anchore	iviiri.		54 (16)	7/4	165	830	720	
	1/4" diameter ³	rs – 2 anchors		_	68 (14)		165	830	720	
			Max.	6	54 (16)	43⁄4	350	860	745	
			WidA.	0	68 (14)	-1/4	350	860	745	

1. For additional important information, see the General Information on p. 16.

2. Allowable loads are based on connectors installed with tabulated anchorage type, quantity and size into structural steel. For anchorage installations into concrete, the capacity of the connection system will be the minimum of the tabulated value and the allowable load using concrete screws indicated on the table on p. 41. Note that if the Designer chooses to calculate concrete anchorage with alternate ¼"-diameter anchors, then the maximum load shall not exceed the tabulated values in this table. Refer to the figures on p. 39 for anchorage conditions.

3. Please refer to the table on p. 41 for Simpson Strong-Tie® Titen HD anchorage loads.

4. Min. fasteners quantity and tabulated values - fill round holes; max. fasteners quantity and tabulated values - fill round and triangular holes.

5. The stand-off is the distance from the interior flange of the stud to the face of the supporting structure. Note that the interior flange of the stud is assumed to align with the inside vertical edge of the connector as indicated in the illustrations on p. 41.

6. Tabulated values are based on 3%" studs. Web crippling checks for deeper members are the responsibility of the Designer.
 7. Allowable loads are based on in-plane loads applied to the stud fasteners that are nearest the support, with complete

rotational restraint at the studs.

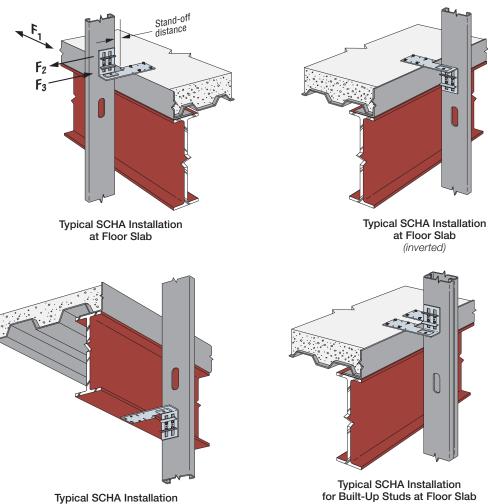
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Strong

Connectors for Cold-Formed Steel Construction

SCHA Slide-Clip Connectors for Horizontal Anchorage

Deflection Connectors



at Beam Flange



Allowable 1/4" Titen HD® Concrete Screw Anchorage Loads

			Allowable Anchor I	Load (Ib.) F_2 and F_3	
Anchorage Type	Anchors	Wind and Seisr	nic in SDC A&B	Seismic in SD	C C through F
	Quantity and Size	Uncracked Concrete	Cracked Concrete	Cracked Concrete $(\Omega = 1.0)$	Cracked Concrete ⁷ ($\Omega = 2.5$)
Simpson Strong-Tie® Titen HD screw anchor	(4) ¼" x 1 1⁄8"	1,025	730	855	350
model THDB25178H	(2) ¼" x 1 1/8"	510	365	425	175

1. Allowable anchor capacities have been determined using ACI 318-14 Chapter 17 calculations with a minimum concrete compressive strength (f'c) of 2,500 psi and 5" slab thickness in normal-weight concrete. Tabulated values

can be multiplied by a factor (λ_a) of 0.6 for sand-lightweight concrete.

2. Nominal Embedment Depth/Effective Embedment Depth relationship is 1.75" (hnom) / 1.30" (hef).

3. Edge distance is assumed to be 2", and end distance is 71%".

4. Load values are for group anchors based on ACI 318-14, condition B, load factors from ACI 318-14 Section 5.3,

no supplement edge reinforcement, ψ_c , v = 1.0 for cracked concrete and periodic special inspection.

5. Allowable Stress Design (ASD) values were determined by multiplying calculated LRFD capacities by a conversion factor, Alpha (α), of 0.70 for seismic load and 0.6 for wind loads. ASD values for other combinations may be determined using alternate conversion factors.

6. Tabulated allowable ASD loads for Wind and Seismic in SDC A&B are based on using wind conversion factors and may be increased by 1.17 for SDC A&B only.

7. Allowable loads have been divided by an Omega (Ω) seismic factor of 2.5 for brittle failure as required by ACI 318-14 Chapter 17.

8. Allowable F_2 and F_3 loads are based on the governing loading direction, which is toward the edge of slab.

9. Tabulated capacities are based on maximum allowable anchorage loads only. The capacity of the connection system shall be the minimum of the tabulated value and the SCHA Allowable Connector Loads.

Drift Clips

IDCB Drift-Clip Bypass Framing Connector

SIMPSON Strong⁻

21/2'

(3) XLSH78B1414

centered in each

4" ^{min.}

vertical slot.

51/2"

1¼" typ.

IDCB45.5

(2) XLQ114B1224 screw

after installation of clip.

centered in each horizontal

slot. Back out screw 1/2 turn

The IDCB drift-clip connector is used to secure bypass stud framing to the edge of a slab. The connector will accommodate 1" of lateral drift in each direction and 1" of upward and downward vertical deflection. Tested load values are provided for anchorage to a steel-edge angle using #12 x 1 1/4" Strong-Drive® XL Large-Head Metal screws.

Features:

- · Horizontal embossments and corner gussets optimize performance in the F₂ load direction
- · Precision-manufactured shouldered screws provided with the IDCB connector are designed to prevent overdriving and to ensure that the clip functions properly
- Simpson Strong-Tie® No-Equal stamps mark the center of the slots to help ensure correct shouldered screw and anchor placement

Material: 97 mil (12 ga.)

Coating: Galvanized (G90)

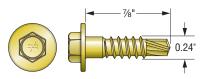
Installation:

- Use the specified type and number of fasteners and anchors.
- In the vertical slots, use the specified number of #14 shouldered screws (included) for attachment to the stud. Install screws to align with the No-Equal stamp.
- For attachment to a minimum 3/16"- and maximum 1/2"-thick steel edge angle, use Simpson Strong-Tie® Strong-Drive XL Large-Head Metal screws (XLQ114B1224). Use one screw centered in each horizontal slot. Install screws to align with the No-Equal stamp and back out 1/2 turn.
- For fastener installation into steel backed by concrete, predrilling of both the steel and the concrete may be required. For predrilling, use a maximum 3/16"-diameter drill bit.

Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

IDCB45.5-KT25 contains (25) IDCB45.5 connectors and (83) XLSH78B1414 #14 shouldered screws

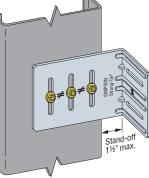


XLSH78B1414 #14 Shouldered Screw for Attachment to Stud Framing (included)



11/4" 0.22

XLQ114B1224 Screw for Anchorage to Steel Edge Angle (sold separately)



Stand-Off Distance

IDCB45.5 Connector Loads

Model No.	No. of #14	No. of #12 XLQ	Load	Stud Thickness	Stre	ngth ³	Servic	e Limit ³	Code
WOUEI NO.	Shouldered Screws ¹	Screw Anchors ²	Direction	mil (ga.)	ASD	LRFD	1/8" Deformation	3/16" Deformation	Ref.
				33 mil (20 ga.)	600	900	410	650	
IDCB45.5	3	2	F_2 and F_3	43 mil (18 ga.)	680	1,060	455	695	170
				54 mil (16 ga.)	760	1,220	500	745	

1. #14 x 7/6" shouldered screw (model no. "XLSH78B1414") provided with the clips are ASTM C1513 compliant.

2. For additional information on the #12 XL screw (model no. "XLQ114B1224") refer to p. 149.

3. The capacity of the connection will be the minimum of Strength Load and applicable Service Limit Load as determined by the Designer.

4. See additional important general information on p. 16.

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DSSCB Drift Strut Sliding Clip Bypass



Drift Clips

This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

Simpson Strong-Tie introduces the DSSCB for cold-formed steel construction. The DSSCB is used to anchor cold-formed steel bypass framing to the edge of a floor slab for situations that accomodate horizontal building drift. The DSSCB is also used by contractors that perform panelized construction. For this application the DSSCB makes it a snap to anchor finished panels to the slab edge by enabling the installer to work off of the top of the slab without having to waste time drilling and installing anchors. It also eliminates the coordination difficulties associated with pre-anchorage of standard bypass clips.

Features:

- The clips come in lengths of 31/2", 6", and 8".
- Prepunched slots provide a full 3/4" of both upward and downward deflection.
- Precision manufactured shouldered screws, provided with DSSCB connectors, are designed to prevent overdriving and to ensure the clip functions properly.
- Works with ¹³/₁₆" and 1 ⁵/₈" strut channels as given in the figures below. Common manufacturers are Unistrut®, PHD and B-Line. Struts are not supplied by Simpson Strong-Tie.
- The maximum stand-off distance for 13/16" struts is 31/8", and for 15%" struts is 41/4".
- Depending on the application and the Designer's specifications, struts can be mechanically anchored or welded.
- · Pre-engineered design solutions are provided for channel strut anchorage.

Material: 97 mil (12 ga.), 50 ksi

Coating: Galvanized (G90)

Installation:

· Use the specified type and number of fasteners

Ordering Information:

• DSSCB43.5-KT25, DSSCB46-KT25, DSSCB48-KT25 contains: 25 connectors and enough shouldered screws for installation

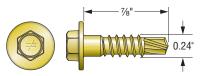


Available in the summer of 2017

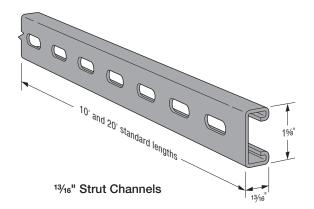
SIMPSON

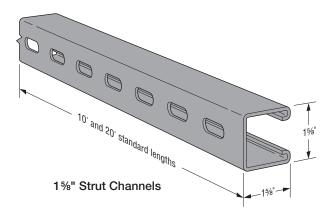
Strong





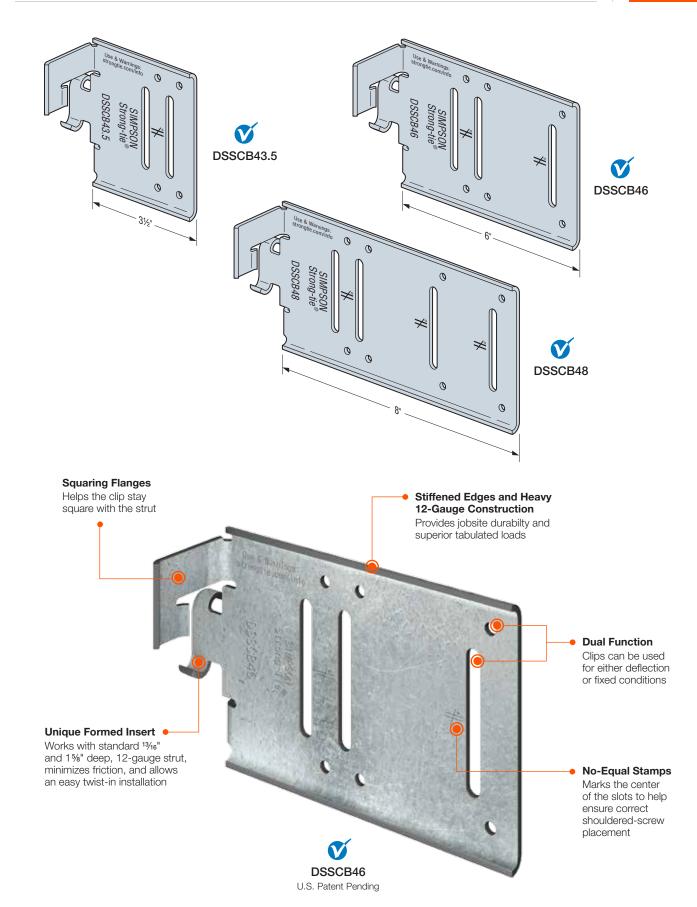
XLSH78B1414 #14 Shouldered Screw for Attachment to Stud Framing (included)





Connectors for Cold-Formed Steel Construction

DSSCB Drift Strut Sliding Clip Bypass



45

Rigid Connectors

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FCB Bypass Framing Fixed-Clip Connector



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The FCB clip is an economical, high-performance fixed-clip connector that can be used for a variety of framing applications. It is rated for tension, compression, shear and in-plane loads and offers the Designer the flexibility of specifying different screw and anchorage patterns that conform to desired load levels.

Features:

- Rated for tension, compression, shear and in-plane loads
- Provides design flexibility with varying screw and anchorage patterns that achieve different load levels
- Strategically placed stiffeners, embossments and anchor holes maximize connector performance

Material: 54 mil (16 ga.)

Finish: Galvanized (G90)

Installation:

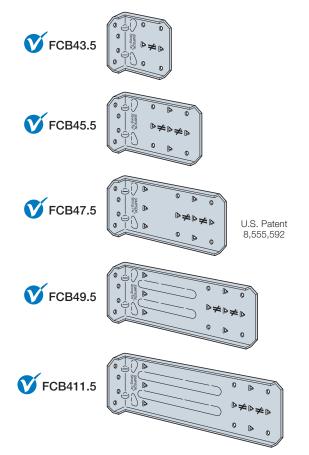
- Use the specified type and number of anchors.
- Use the specified number of #12 self-drilling screws to CFS framing. Note that #10 self-drilling screws can be used per the load tables given on pp. 54, 55.

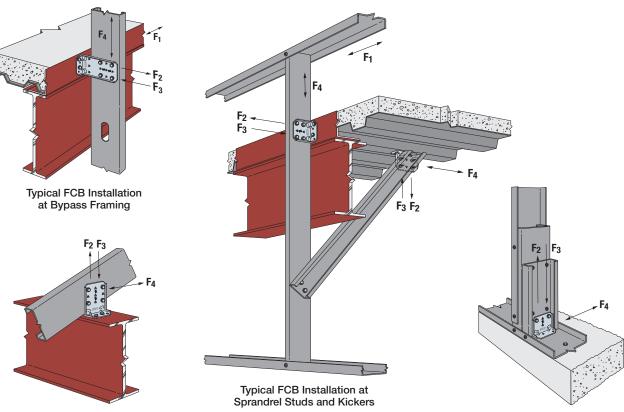
Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

FCB43.5-R25, FCB45.5-R25, FCB47.5-R25, FCB49.5-R25, and FCB411.5-R25 contain:

· Box of 25 connectors (screws not included)





Typical FCB Installation for Roof Rafters **Rigid Connectors**

FCB Bypass Framing Fixed-Clip Connector



FCB Allowable Connector Loads

	Connector			No. of						Stud Th	ickness						
Model No.	Material Thickness	L (in.)	Min./ Max.	#12–14 Self-Drilling		33 mil	(20 ga.)			43 mil	(18 ga.)			54 mil	(16 ga.)		Code Ref.
	mil (ga.)			Screws⁵	F1 ^{3,4}	F ₂ ²	F3 ²	F4 ²	F1 ^{3,4}	F ₂ ²	F3 ²	F4 ²	F1 ^{3,4}	F ₂ ²	F3 ²	F4 ²	
FCB43.5	E4 (16)	3½	Min.	4	135	755	755	755	205	1,105	975	1,120	235	1,250	975	1,490	
FUD43.0	54 (16)	372	Max.	6	205	1,100	1,130	1,130	205	1,105	1,260	1,455	235	1,250	1,735	1,910	
	E4 (10)	E1/	Min.	4	120	755	755	755	175	1,105	975	945	175	1,105	975	1,325	
FCB45.5	54 (16)	5½	Max.	9	155	1,100	1,260	1,180	175	1,105	1,260	1,485	175	1,105	1,735	1,925	
FCB47.5	E4 (16)	7½	Min.	4	90	755	755	220	90	1,105	945	330	90	1,105	945	365	IP2,
FUD47.0	54 (16)	1 /2	Max.	12	205	1,100	1,260	705	235	1,105	1,260	1,050	235	1,105	1,735	1,445	L8, FL
	E4 (10)	9½	Min.	4	60	755	755	170	60	1,105	945	255	60	1,105	945	365	
FCB49.5	54 (16)	972	Max.	12	205	1,100	1,260	750	235	1,105	1,260	1,115	235	1,105	1,735	1,200	
	E4 (10)	441/	Min.	4	45	755	755	140	45	1,105	920	205	45	1,105	920	365	
FCB411.5	54 (16)	11½	Max.	12	205	1,100	1,260	795	235	1,105	1,260	860	235	1,105	1,735	860	

1. Min. fastener quantity and load values - fill all round holes; max. fastener quantity and load values - fill all round and triangular holes.

2. Allowable loads are based on clip capacity only and do not consider anchorage. The capacity of the connection system will be

the minimum of the tabulated value and the allowable load from the FCB Allowable Anchorage Loads table on p. 49.

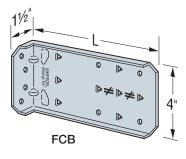
3. Anchorage to the supporting structure using welds or a minimum of (2) #12-24 self-drilling screws is required.

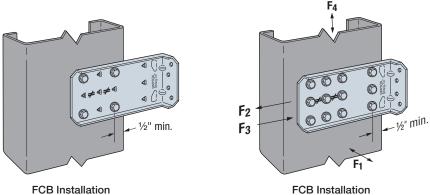
4. Allowable loads are based on in-plane loads applied to the stud fasteners that are nearest the support with complete rotational restraint at the studs. For a more extensive treatment of in-plane loads please refer to Simpson Strong-Tie engineering letter L-CF-CWCF1DIR17 at strongtie.com.

5. Reference pp. 54, 55 for loads for #10-16 screws.

6. Reference pp. 63–67 for LRFD design strengths.

with Min. Fasteners





with Max. Fasteners

FCB Bypass Framing Fixed-Clip Connector

FCB Allowable Anchorage Loads

						Allowat	ole Load (II	b.)			
A b	Minimum	No. of					F4				
Anchorage Type	Base Material	Anchors	F_2 and F_3	FCB43.5	FCB45.5	FCB	47.5	FCB	49.5	FCB4	111.5
				Min./Max.	Min./Max.	Min.	Max.	Min.	Max.	Min.	Max.
#12 self-drilling screws	400.1.1	2	795	625	410	255	445	185	265	120	190
Simpson Strong-Tie® X Metal screws	A36 steel ¾6" thick	3	1,120	690	450	280	490	200	295	135	210
XQ1S1214, X1S1214	710 think	4	1,590	1,255	820	365	890	355	535	275	380
Simpson Strong-Tie #12–24 x 11/4"		2	1,115	625	410	255	445	185	265	120	190
Strong-Drive® XL Large-Head Metal screws	A36 steel ¾6" thick	3	1,645	690	450	280	490	200	295	135	210
XLQ114T1224, XLQ114B1224-2K		4	2,230	1,255	820	365	890	355	535	275	380
Simpson Strong-Tie		2	390	410	265	165	290	120	175	75	125
0.157" x 5%" power-actuated fasteners	A36 steel ¾6" thick	3	715	465	305	190	330	135	195	85	140
PDPAT-62KP	710 think	4	970	840	550	340	595	245	355	145	255
Simpson Strong-Tie	A572 or	2	585	410	265	165	290	120	175	75	125
0.157" x 5%" power-actuated fasteners	A992 steel	3	800	465	305	190	330	135	195	85	140
PDPAT-62KP	3⁄16" thick	4	1,170	840	550	340	595	245	355	145	255
Simpson Strong-Tie		2	380	415	270	165	295	120	175	105	125
1/4" x 13/4" Titen® hex-head masonry screws	Concrete f' _c = 2,500 psi	3	445	470	310	190	335	140	200	120	145
TTN25134H	2,000 par	4	510	645	420	260	455	190	275	165	195
Welded	A36 steel H	Hard side: 2"	1.735	1.910	1,925	365	1.445	365	1.200	365	860
E70XX electrodes	⅔ı6" thick	Free side: 1"	1,735	1,910	1,920	300	1,445	300	1,200	300	000

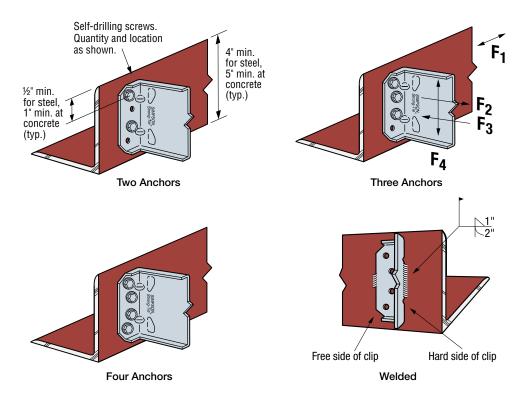
1. For additional important information, see General Information on p. 16.

2. Allowable loads are for clip anchorage only. The capacity of the connection system will be the minimum of the tabulated allowable anchorage loads the allowable load from the FCB Allowable Connector Load table on p. 48.

3. Allowable loads for #12–24 self-drilling screws and PDPAT powder-actuated fasteners are based on installation in minimum %="thick structural steel with F_y = 36 ksi. PDPAT values are also provided for A572 steel. Values listed above maybe used where other thicknesses of steel are encountered provided that the fastener has equal or better tested values into thicker steel (see p. 16). It is the responsibility of the Designer to select the proper length fasteners based on the steel thickness installation.

4. For screw fastener installation into steel backed by concrete, predrilling of both the steel and the concrete is suggested.

For predrilling use a maximum 3/16"-diameter drill bit.



FCB Anchor Layout

The following FCB supplemental information is given to help Designers with value-engineered solutions for our FCB connectors. Loads are given for fastener patterns other than our standard "min." (fill all round holes) and "max." (fill all round and triangle holes). In addition, the tables give LRFD loads and loads for #10 screws as well as #12 screws.

Table 1: FCB Screw Patterns Pattern "Min." Pattern "Max."

Rigid Connectors

	Pattern "Min."	Pattern "Max."						
FCB43.5								
	Pattern "Min."	Pattern 1	Pattern 2	Pattern "Max."				
FCB45.5								
	Pattern "Min."	Pattern 3	Pattern 4	Pattern 5	Pattern 6	Pattern "Max."		
FCB47.5								
	Patter	n "Min."	Pa	attern 7	Pattern 8			
FCB49.5	Pat	ttern 9	Pat	ttern 10	Pattern "Max."			
					0 0 0 0+0+0 0 0 0			
	Patter	m "Min."	Pat	ttern 11	Pat	tern 12		
FCB411.5	Patt	ern 13	Pat	ttern 14	Pattern "Max."			



Footnotes for Tables 2, 3, 4 and 5

- Calculated values are per AISI RP15-2, AISI S-100, or generally accepted industry standards. Shaded values for #12–14 screws are tabulated in this catalog and are based on testing per ICC-ES AC261. For #12–14 screws unshaded tabulated values are conservatively based on the maximum value from calculations and from the minimum (4-screw) tested values.
- 2. The tabulated values do not account for anchorage to the support. Anchor strength must be calculated separately and may reduce the capacity of the connection when compared to the tabulated values.
- 3. Tabulated values do not include shear, web crippling, buckling or other local effects in the member. The Designer must check member limit states separately.
- 4. For load combinations that include F1 and/or F2 and/or F3, use an appropriate interaction equation.
- 5. #10–16 screws shall have P_{SS} \geq 1,620 lb. #12–14 screws shall have P_{SS} \geq 2,520. Calculated values are per AISI S-100. Screws must be installed with three (min.) exposed threads.
- 6. The number of screws listed is for one clip leg that is attached to the supported stud.
- 7. For the minimum screw pattern, fill all round holes. For the maximum screw pattern fill all round and triangle holes. Reference Table 1 on p. 50.
- 8. Reference p. 48 for load direction definitions.
- 9. In addition to calculations of net and gross section tension, and screw shear of the clip leg attached to the stud, F₂ values are also calculated for weak-axis bending of the anchored clip leg with the line of bending at the holes farthest from the bend radius of the angle. The Designer is responsible for calculating pull-over, pullout and tension strength of the anchors and this may reduce F₂ strength compared to the tabulated values.
- 10. F₃ values are computed using the plate buckling provisions of AISI RP15-2.
- 11. For the F4 values it is assumed that all of the connection eccentricity is taken by screws in the supported stud. F4 values are also limited by plate shear buckling per AISI RP15-2. The Designer is responsible for calculating the shear capacity of the anchorage which may reduce F4 strength compared to the tabulated values.
- 12. In addition to the limit states given in notes 9, 10 and 11, F₂, F₃ and F₄ are also limited by screw shear according to the thinnest connected part of the connector and stud.
- 13. Where test data is not available, service load limits for F₂ and F₃ are not given since there are no generally accepted industry methods available to compute these values. Calculated F₄ service load limits are based on AISI Research Report RP15-2 for ½" deflection.
- 14. For 50 ksi studs, 68 mil (14 ga.) and thicker, use tabulated values for 54 mil (16 ga.) 50 ksi studs.

Strong



Table 2: FCB Bypass Framing Fixed-Clip Connectors (FCB43.5, FCB45.5, FCB47.5 with #12–14 Screws) — Service Load Limits and Strengths (lb.)

								Stud Thick	ness and Yie	eld Strength			
Model No.	No. of #12	Screw Pattern	Load Direction	Service Load	33 m	il (20 ga.) – :	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi
	Screws			Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,250	3,040	4,655
	4	Min.	F3	1,185	755	1,130	2,180	975	1,565	2,180	975	1,565	2,180
500 40 5			F4	2,170	755	1,130	1,980	1,120	1,680	2,590	1,490	2,380	3,380
FCB43.5			F ₂	1,100	1,100	1,690	3,245	1,105	2,425	3,720	1,250	3,040	4,655
	6	Max.	F3	1,260	1,130	1,690	3,245	1,260	2,425	3,720	1,735	3,040	4,010
			F4	2,415	1,130	1,690	2,635	1,455	2,330	3,315	1,910	3,055	4,345
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	4	Min.	F3	1,185	755	1,130	2,180	975	1,565	2,180	975	1,565	2,180
			F4	1,225	755	1,130	1,720	945	1,515	2,150	1,325	2,120	3,015
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	1	F3	1,185	755	1,130	2,180	975	1,565	2,180	975	1,565	2,180
500455			F4	1,225	755	1,130	1,720	945	1,515	2,150	1,325	2,120	3,015
FCB45.5			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	2	F3	1,185	755	1,130	2,180	975	1,565	2,180	975	1,565	2,180
			F4	1,225	755	1,130	1,720	945	1,515	2,150	1,325	2,120	3,015
			F2	1,100	1,100	2,025	2,880	1,105	2,425	3,720	1,105	2,680	4,110
	9	Max.	F3	1,260	1,260	2,285	3,245	1,260	2,425	3,720	1,735	2,870	4,010
			F4	2,570	1,180	1,890	2,685	1,485	2,375	3,380	1,925	3,080	4,380
			F ₂	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	4	Min.	F3	1,185	755	1,130	2,115	945	1,515	2,115	945	1,515	2,115
			F4	380	200	330	660	330	495	830	365	580	830
			F ₂	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	3	F3	1,185	755	1,130	2,115	945	1,515	2,115	945	1,515	2,115
			F4	380	260	390	775	380	575	1,060	380	585	1,060
			F ₂	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	4	F3	1,185	755	1,130	2,115	945	1,515	2,115	945	1,515	2,115
FOD 47 5			F4	380	275	410	820	380	585	1,060	380	585	1,060
FCB47.5			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	5	F3	1,185	785	1,275	2,550	945	1,515	2,550	945	1,515	2,550
			F4	465	385	580	1,155	575	860	1,720	900	1,385	2,520
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	9	6	F3	1,185	755	1,195	2,390	945	1,515	2,390	945	1,515	2,390
			F4	465	530	800	1,595	790	1,185	2,370	900	1,385	2,520
			F ₂	1,100	1,100	2,025	2,880	1,105	2,425	3,720	1,105	2,680	4,110
	12	Max.	F3	1,260	1,360	2,285	3,245	1,260	2,425	3,720	1,735	2,870	3,780
			F4	1,875	705	1,060	2,120	1,060	1,575	3,155	1,445	2,310	3,440

See footnotes on p. 51.

Table 3: FCB Bypass Framing Fixed-Clip Connectors (FCB49.5, FCB411.5 with #12–14 Screws) — Service Load Limits and Strengths (lb.)

								Stud Thick	ness and Yie	eld Strength			
Model No.	No. of #12	Screw Pattern	Load Direction	Service Load	33 m	il (20 ga.) – :	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi
	Screws			Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	4	Min.	F3	1,185	755	1,130	2,180	945	1,565	2,180	945	1,565	2,180
			F4	380	170	255	510	255	380	760	365	580	830
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	7	F3	1,185	755	1,130	2,180	945	1,565	2,180	945	1,565	2,180
			F4	380	195	295	590	265	410	760	365	580	830
			F ₂	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	8	F3	1,185	755	1,130	2,180	945	1,565	2,180	945	1,565	2,180
50D40 5			F4	380	210	315	630	265	410	760	365	580	830
FCB49.5			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	9	F3	1,185	785	1,275	2,550	945	1,565	2,550	945	1,565	2,550
			F4	465	390	585	1,170	580	870	1,735	900	1,385	2,520
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	9	10	F3	1,185	755	1,195	2,390	945	1,565	2,390	945	1,565	2,390
			F4	465	555	835	1,670	830	1,240	2,485	900	1,385	2,520
		Max.	F2	1,100	1,100	2,025	2,880	1,105	2,425	3,720	1,105	2,680	4,110
	12	Max.	F3	1,260	1,360	2,285	3,245	1,260	2,425	3,720	1,735	2,870	4,010
		Max.	F4	2,290	750	1,120	2,245	1,115	1,670	3,020	1,200	2,125	3,020
			F ₂	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	4	Min.	F3	1,185	755	1,130	1,720	920	1,470	1,720	920	1,470	1,720
			F4	380	140	210	415	205	310	620	365	580	830
			F ₂	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	11	F3	1,185	755	1,130	1,720	920	1,470	1,720	920	1,470	1,720
			F4	380	160	240	480	205	315	620	365	580	830
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	12	F3	1,185	755	1,130	1,720	920	1,470	1,720	920	1,470	1,720
500 444 5			F4	380	170	255	510	205	315	620	365	580	830
FCB411.5			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	6	6 13	F3	1,185	785	1,275	2,550	920	1,470	2,550	920	1,470	2,550
	0		F4	465	405	605	1,210	600	900	1,795	900	1,385	2,520
			F2	1,100	755	1,130	2,255	1,105	1,680	3,360	1,105	2,680	4,110
	9	14	F3	1,185	755	1,195	2,390	920	1,470	2,390	920	1,470	2,390
			F4	465	585	880	1,760	870	1,310	2,520	900	1,385	2,520
			F ₂	1,100	1,100	2,025	2,880	1,105	2,425	3,720	1,105	2,680	4,110
	12	Max.	F ₃	1,260	1,260	2,285	3,245	1,260	2,425	3,645	1,735	2,870	3,645
			F4	2,290	795	1,195	2,390	860	1,780	3,020	860	2,125	3,020

SIMPSON

Strong-Tie

See footnotes on p. 51.

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Table 4: FCB Bypass Framing Fixed-Clip Connectors (FCB43.5, FCB45.5, FCB47.5 with #10–16 Screws) — Service Load Limits and Strengths (lb.)

								Stud Thick	ness and Yie	eld Strength			
Model No.	No. of #10	Screw Pattern	Load Direction	Service Load	33 m	il (20 ga.) – 3	33 ksi	43 m	il (18 ga.) – 3	33 ksi	54 m	il (16 ga.) –	50 ksi
	Screws			Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F2		490	735	815	490	735	815	490	735	815
	4	Min.	F3	_	440	715	1,430	440	715	1,430	440	715	1,430
500 40 5			F4	465	340	515	1,025	530	795	1,585	900	1,385	2,520
FCB43.5			F2	_	490	735	815	490	735	815	490	735	815
	6	Max.	F3	_	660	1,075	2,145	660	1,075	2,145	660	1,075	2,145
			F4	465	415	620	1,240	640	960	1,915	900	1,385	2,520
			F2	_	490	735	815	490	735	815	490	735	815
	4	Min.	F3		660	990	1,980	670	1,085	2,170	670	1,085	2,170
			F4	350	270	410	815	420	630	1,260	700	1,075	1,955
			F2	_	490	735	815	490	735	815	490	735	815
	6	1	F3		625	1,015	2,035	625	1,015	2,035	625	1,015	2,035
50045.5			F4	350	350	525	1,055	545	815	1,630	700	1,075	1,955
FCB45.5			F2		490	735	815	490	735	815	490	735	815
	6	2	F3	_	670	1,085	2,170	670	1,085	2,170	670	1,085	2,170
			F4	_	370	555	1,110	570	855	1,710	700	1,075	1,955
			F2	_	490	735	815	490	735	815	490	735	815
	9	Max.	F3	_	625	1,015	2,035	625	1,015	2,035	625	1,015	2,035
			F4	350	460	690	1,380	700	1,065	1,955	700	1,075	1,955
			F2	_	490	735	815	490	735	815	490	735	815
	4	Min.	F3	_	455	735	1,470	455	735	1,470	455	735	1,470
			F4	175	195	290	580	300	450	895	380	585	1,060
			F ₂	_	490	735	815	490	735	815	490	735	815
	6	3	F3	_	425	690	1,380	425	690	1,380	425	690	1,380
			F4	175	245	365	730	375	565	1,060	380	585	1,060
			F ₂	_	490	735	815	490	735	815	490	735	815
	6	4	F3	_	455	735	1,470	455	735	1,470	455	735	1,470
ECD 47 5			F4	175	260	390	775	380	585	1,060	380	585	1,060
FCB47.5			F2	_	490	735	815	490	735	815	490	735	815
	6	5	F3	_	785	1,275	2,550	785	1,275	2,550	785	1,275	2,550
			F4	465	365	545	1,090	560	845	1,685	900	1,385	2,520
			F2	_	490	735	815	490	735	815	490	735	815
	9	6	F ₃	_	735	1,195	2,390	735	1,195	2,390	735	1,195	2,390
			F4	465	500	750	1,505	775	1,165	2,325	900	1,385	2,520
			F ₂	—	490	735	815	490	735	815	490	735	815
	12	Max.	F ₃	—	735	1,195	2,390	735	1,195	2,390	735	1,195	2,390
			F4	465	620	930	1,855	900	1,385	2,520	900	1,385	2,520

465

See footnotes on p. 51.

Table 5: FCB Bypass Framing Fixed-Clip Connectors (FCB49.5, FCB411.5 w/ #10–16 Screws) — Service Load Limits and Strengths (lb.)

								Stud Thick	ness and Yie	eld Strength			
Model	No. of #10 Screws	Screw Pattern	Load Direction	Service Load Limit	33 mi	il (20 ga.) – 3	33 ksi	43 m	il (18 ga.) – :	33 ksi	54 m	il (16 ga.) –	50 ksi
	3016W3			LIIIII	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F2		490	735	815	490	735	815	490	735	815
	4	Min.	F3	—	360	585	1,175	360	585	1,175	360	585	1,175
			F4	115	150	225	450	230	345	690	265	410	745
			F ₂	_	490	735	815	490	735	815	490	735	815
	6	7	F3	_	340	550	1,100	340	550	1,100	340	550	1,100
			F4	115	185	280	560	265	410	745	265	410	745
			F ₂	—	490	735	815	490	735	815	490	735	815
	6	8	F3	—	360	585	1,175	360	585	1,175	360	585	1,175
FOR 40 F			F4	115	200	295	595	265	410	745	265	410	745
FCB49.5			F ₂	_	490	735	815	490	735	815	490	735	815
	6	9	F3	—	785	1,275	2,550	785	1,275	2,550	785	1,275	2,550
			F4	465	365	550	1,100	570	850	1,705	900	1,385	2,520
			F2	_	490	735	815	490	735	815	490	735	815
	9	10	F3	—	735	1,195	2,390	735	1,195	2,390	735	1,195	2,390
			F4	465	525	790	1,575	810	1,220	2,435	900	1,385	2,520
			F2	—	490	735	815	490	735	815	490	735	815
	12	Max.	F3	—	735	1,195	2,390	735	1,195	2,390	735	1,195	2,390
			F4	465	655	985	1,965	900	1,385	2,520	900	1,385	2,520
			F ₂	—	490	735	815	490	735	815	490	735	815
	4	Min.	F3	—	210	320	375	210	320	375	210	320	375
			F4	90	120	180	365	190	280	565	205	315	575
			F ₂	—	490	735	815	490	735	815	490	735	815
	6	11	F3	_	210	320	375	210	320	375	210	320	375
			F4	90	150	225	450	205	315	575	205	315	575
			F ₂	_	490	735	815	490	735	815	490	735	815
	6	12	F3	—	210	320	375	210	320	375	210	320	375
FCB411.5			F4	90	160	240	480	205	315	575	205	315	575
F66411.3			F2	_	490	735	815	490	735	815	490	735	815
	6	13	F3	—	785	1,275	2,550	785	1,275	2,550	785	1,275	2,550
			F4	465	380	570	1,140	585	880	1,760	900	1,385	2,520
			F2	—	490	735	815	490	735	815	490	735	815
	9	14	F3		735	1,195	2,390	735	1,195	2,390	735	1,195	2,390
			F4	465	555	830	1,660	855	1,285	2,520	900	1,385	2,520
			F ₂	—	490	735	815	490	735	815	490	735	815
	12	Max.	F3	—	735	1,195	2,390	735	1,195	2,390	735	1,195	2,390
			F4	465	695	1,045	2,090	900	1,385	2,520	900	1,385	2,520

See footnotes on p. 51.

SIMPSON

Strong-Tie

Special Order Custom Clips and Connectors

simpso Strong'

Simpson Strong-Tie can make a variety of flat and bent steel clips and connectors for cold-formed steel framing. Most custom clips can be punched with different holes and slots.

Material: 229 mil (3 ga.) maximum, 43 mil (18 ga.) minimum mill-certified steel (carbon and type 316L stainless steel)

Finish: Galvanized, Simpson Strong-Tie® gray paint. Contact Simpson Strong-Tie for availability.

To Obtain Quote:

- Supply a CAD drawing in .dwg or .dxf format complete with all dimensions, hole diameter and centerline locations, bend angles, steel strength (min. F_v and F_u), thickness (mil and/or ga.) and finish: (galvanized to G90, G185) or Simpson Strong-Tie gray paint (specify)
- Total shape and size up to a maximum of 48" x 48" (approx. 1/16" tolerance)

Specification Example:

Quantity: XX pieces

Dimensions: Per the attached CAD drawing (.dwg or .dxf format)

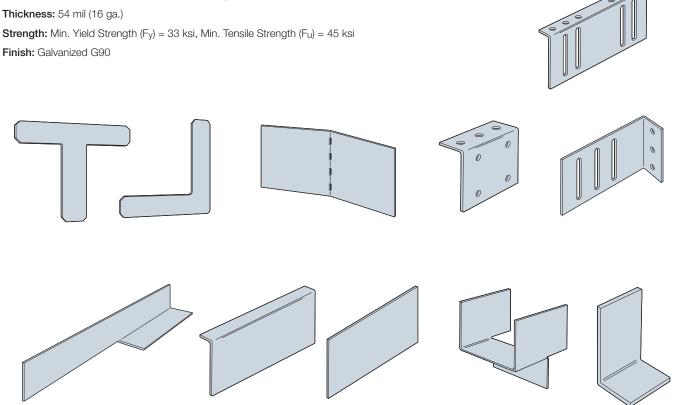
Drawing must be fully dimensioned, including:

- Overall dimensions
- · Leg dimensions
- · Bend angles (if required)
- · Hole/slot sizes and centerlines (if required)

Material Specification: Contact Simpson Strong-Tie for availability

Strength: Min. Yield Strength (Fy) = 33 ksi, Min. Tensile Strength (Fu) = 45 ksi

- Simpson Strong-Tie does not provide product engineering or load values for special-order custom clips and connectors
- Contact Simpson Strong-Tie for pricing information
- For additional information, please refer to Important Information and General Notes on pp. 12-16



Connectors for Cold-Formed Steel Construction

FC Bypass Framing Fixed-Clip Connector

Ideal for high-seismic areas, Simpson Strong-Tie[®] FC connectors are the optimal solution for fixed-clip bypass framing. FC clips are often welded to the structure in high-seismic zones, but they also feature anchorage holes so that concrete screws or powder-actuated fasteners can be used to attach the clip to the structure. In addition to its anchorage versatility, the FC clip features prepunched screw holes for the framing attachment, eliminating the need for predrilling holes or worrying that fastener placement doesn't match the Designer specifications. FC connectors are manufactured using heavy-duty 10- and 12-gauge steel to provide exceptional resistance to in-plane seismic load.

Features:

- The clips come in lengths of 3½", 6" and 8" and are intended to be used with 35%", 6" and 8" studs, respectively
- The maximum stand-off distance is 1" for 3%" studs and 1 $\frac{12}{2}$ for 6" and 8" studs
- Embossments in the bend line provide increased strength and stiffness in the F₁ and F₂ load directions, but are positioned towards the center of the clip so that 1 ½"-long welds can be applied at the top and bottom of the clip
- Prepunched large-diameter anchor holes accommodate ¼"-diameter concrete screws like the Simpson Strong-Tie Titen HD[®] screw anchor
- Prepunched small-diameter anchor holes accommodate powder-actuated fasteners like the 0.157"-diameter Simpson Strong-Tie PDPAT or #12 self-drilling Simpson Strong-Tie Strong-Drive[®] XL Large-Head Metal screw

Material: 50 ksi

Finish: Galvanized (G90)

Installation:

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

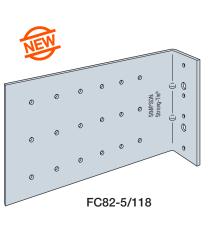
• Use the specified type and number of fasteners and anchors

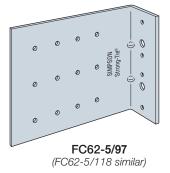
Ordering Information and Dimensions

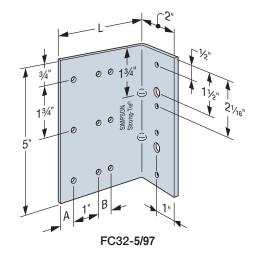
Codes: See p. 11 for Code Reference Key Chart

0.0.0					
Model No.	Ordering SKU	Thickness mil (ga.)	L (in.)	A (in.)	B (in.)
FC32-5/97	FC32-5/97-KT25	97 (12)	31⁄2	1/2	1/2
FC62-5/97	FC62-5/97-KT25	97 (12)	6	1	1
FC62-5/118	FC62-5/118-KT25	118 (10)	6	1	1
FC82-5/118	FC82-5/118-KT25	118 (10)	8	1	1

Note: Each box contains (25) connectors.





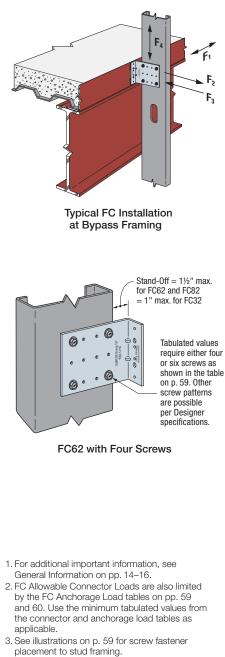


SIMPSON Strong-Tie

FC Bypass Framing Fixed-Clip Connector

FC Allowable Connector Loads (lb.)

		Fastener	s to Stud		Allow	able Load			
Model	Stud Thickness	Allowable Pullout	No. of #10	F	1				Code
No.	mil (ga.)	Per Single #10 Screw	Self-Drilling Screws	1" Stand-Off	1½" Stand-Off	F ₂	F3	F4	Ref.
FC32-5/97			4	165	—	705	1,130	705	
FU3Z-0/9/			6	225		1,060	1,355	1,060	
FC62-5/97			4	115	130	705	1,130	705	
1002-0/97	33 (20)	85	6	140	160	1,060	1,355	1,060	
FC62-5/118	33 (20)	00	4	115	130	705	1,130	705	
1002-0/110			6	140	160	1,060	1,355	1,060	
FC82-5/118			4	105	120	705	1,130	705	
1002-0/110			6	135	155	1,060	1,355	1,060	
FC32-5/97			4	215	—	1,050	1,470	1,050	
1002-0/9/			6	290		1,580	1,765	1,580	
FC62-5/97			4	150	175	1,050	1,470	1,050	
1002-3/97	43 (18)	110	6	185	215	1,580	1,765	1,580	
FC62-5/118	43 (10)	110	4	150	175	1,050	1,470	1,050	
FG02-3/110			6	185	215	1,580	1,765	1,580	
FC82-5/118			4	140	160	1,050	1,470	1,050	
FG0Z-0/110			6	175	200	1,580	1,765	1,580	
FC22 E/07			4	395	_	2,135	2,885	2,045	
FC32-5/97			6	530		2,690	2,885	2,195	1
F000 F/07			4	325	375	2,135	2,885	2,045	
FC62-5/97	E4 (10)	000	6	405	465	2,690	2,885	2,195	170
E000 E /110	54 (16)	200	4	345	395	2,135	2,885	2,045	170
FC62-5/118			6	370	425	3,205	2,885	2,195]
F000 F/110	1		4	325	375	2,135	2,885	2,045	1
FC82-5/118			6	440	505	3,205	2,885	2,195	
E000 E/07			4	495	_	2,160	2,885	2,045	
FC32-5/97			6	670		2,690	2,885	2,195	
E000 E/07			4	435	500	2,160	2,885	2,045	
FC62-5/97	CO (14)	050	6	465	535	2,690	2,885	2,195	1
F000 F/110	68 (14)	250	4	435	500	2,160	2,885	2,045	
FC62-5/118			6	465	535	3,240	3,780	2,195	1
F000 F/110			4	410	470	2,160	2,885	2,045	
FC82-5/118			6	555	640	3,240	3,780	2,195	
E000 E /07			4	710		2,160	2,885	2,045	
FC32-5/97			6	955		2,690	2,885	2,195	
F000 5/07	1		4	775	775	2,160	2,885	2,045	1
FC62-5/97	07 (10)	055	6	1295	1295	2,690	2,885	2,195	
E000 E ////C	97 (12)	355	4	775	775	2,160	2,885	2,045	
FC62-5/118			6	1150	1150	3,240	3,780	2,195	1
E000 E (110	1		4	585	585	2,160	2,885	2,045	1
FC82-5/118			6	790	790	3,240	3,780	2,195	1



- 4. Tabulated F₁ loads are based on assembly tests with the load through the centerline of stud. Tested failure modes were due to screw pullout; therefore compare F₁ against F_p calculated per ASCE 7-10 Chapter 13 with $a_p = 1.25$ and $R_p = 1.0$.
- 5. F₁ loads are based on maximum stand-off distances of 1" or 1½" as shown. Other loads are applicable to a 1" stand-off for FC32 and 1" or 1½" stand-off for FC62 and FC82.
- 6. The allowable plastic moment at the bend line in the F₁ load direction for 97 mil (12 ga.) and 118 mil (10 ga.) FC connectors are 395 in.-lb. and 675 in.-lb., respectively.

58

FC Bypass Framing Fixed-Clip Connector



FC Screw Patterns

Screw		Models	
Pattern	FC32-5/97	FC62-5/97 and FC62-5/118	FC82-5/118
4 screws	Constraints	• • <th></th>	
6 screws			

FC Allowable Anchorage Loads to Steel (lb.)

Anchorage Type	Minimum	No. of		Allowable Load	
Anchorage Type	Base Material	Anchors	F ₁	F_2 and F_3	F4
#12 self-drilling screws Simpson Strong-Tie® X Metal screws XQ1B1214, X1B1214	A36 steel ¾6" thick	4	_	2,070	2,200
#12–24 x 1¼" Strong-Drive® XL Large-Head Metal screws XLQ114T1224, XLQ114B1224	A36 steel ¾₅" thick	4	_	2,545	2,545
#14 self-drilling screws Simpson Strong-Tie E Metal screws E1B1414	A36 steel ¾6" thick	4	_	2,620	2,610
Simpson Strong-Tie 0.157" x 5%" powder-actuated fasteners PDPAT-62KP	A36 steel ¾6" thick	4		1,040	1,040
Simpson Strong-Tie 0.157" x 5%" powder-actuated fasteners PDPAT-62KP	A572 Gr. 50 or A992 steel ¾6" thick	4		1,710	1,710
Wolded E70VV electrodes	A36 steel	Hard side: 3"	0.040	0.710	4.000
Welded E70XX electrodes	¾6" thick	Free side: 3"	2,040	3,710	4,330

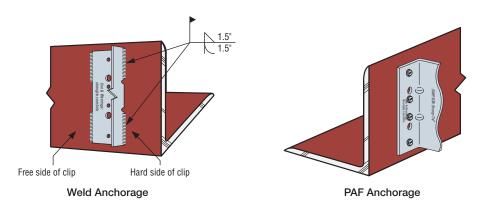
1. For additional important information, see General Information on p. 16.

2. Allowable anchorage loads are also limited by the FC Connector Load table on p. 58.

Use the minimum tabulated values from the connector and anchorage load tables as applicable.

3. Allowable loads for #12–24 self-drilling screws and PDPAT powder-actuated fasteners are based on installation in minimum %6" thick structural steel with Fy = 36 ksi. PDPAT values are also provided for A572 steel. Values listed above maybe used where other thicknesses of steel are encountered provided that the fastener has equal or better tested values into thicker steel (see p. 16). It is the responsibility of the Designer to select the proper length fasteners based on the steel thickness installation.

4. For screw fastener installation into steel backed by concrete, predrilling of both the steel and the concrete is suggested. For predrilling, use a maximum %"-diameter drill bit.



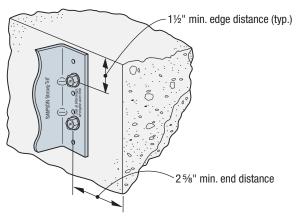
FC Bypass Framing Fixed-Clip Connector

Allowable Titen HD® Anchorage Loads into Concrete with FC Clip (lb.)

		<u> </u>					
Simpson Strong-Tie®	Nominal	Anchor	f'c	Load	Wind and Seisr	NIC IN SUC A&B	Seismic in SDC C through F
1/4" Titen HD® Screw Anchor	Embedment (in.)	Quantity and Size	(psi)	Direction	Uncracked Concrete	Cracked Concrete	Cracked Concrete ⁶
				F1	335	240	280
			3,000	F2 and F3	660	630	550
THDB25178H	15/	(0) 1/1 x 17/1		F4	565	405	470
THDR551/8H	1 5/8	(2) ¼" x 1 %"		F1	390	280	325
			4,000	F2 and F3	760	725	635
				F4	655	465	545
				F1	370	265	310
			3,000	F_2 and F_3	475	695	610
	01/	(0) 1/1 - 03/1		F4	515	445	520
THDB25234H	21⁄2	(2) ¼" x 2¾"		F1	430	305	360
			4,000	F_2 and F_3	550	805	705
				F4	590	515	600

 Allowable anchor capacities have been determined using ACI 318-14 Appendix D calculations with a minimum concrete compressive strength (f_c) of 3,000 and 4,000 psi in normal-weight concrete. Tabulated values shall be multiplied by a factor (λ_a) of 0.6 for sand light-weight concrete.

- 2. Edge distance is assumed to be $1\frac{1}{2}$ ", and end distance is $2\frac{5}{3}$ ".
- 3. Load values are for group anchors based on ACI 318, condition B, load factors from ACI 318-14 Section 5.3, no supplement edge reinforcement, ψ_c , v = 1.0 for cracked concrete and periodic special inspection.
- 4. Allowable Stress Design (ASD) values were determined by multiplying calculated LRFD capacities by a conversion factor, Alpha (α), of 0.70 for seismic load and 0.6 for wind loads. ASD values for other combinations may be determined using alternate conversion factors.
- 5. Tabulated allowable ASD loads for Wind and Seismic in SDC A&B are based on using wind conversion factors and may be increased by 1.17 for SDC A and B only.
- 6. Design loads shall include the over-strength factor per ASCE7 Section 12.4.3. For fasteners in exterior wall connection systems, $\Omega_0 = 1.5$ per Table 13.5-1.
- 7. Allowable loads for F_4 are based on the governing loading direction which is toward the edge of slab.
- 8. Allowable loads for F_1 are based on the governing loading direction which is toward the end of slab.
- 9. For anchor subjected to both tension and shear loads, it shall be designed to satisfy the following:
 - For Na / Nall \leq 0.2, the full allowable load in shear is permitted.
 - \bullet For V_a / V_{all} \leq 0.2, the full allowable load in tension is permitted.
 - For all other cases: Na / Nall + Va / Vall \leq 1.2 where:
 - $N_a = Applied ASD$ tension load
 - N_{all} = Allowable F₂ or F₃ load from the FC Allowable Anchorage Loads for Concrete table
 - Va = Applied ASD shear load
 - V_{all} = Allowable F₄ or F₁ load from the FC Allowable Anchorage Loads for Concrete table
- 10. Tabulated allowable loads are based on anchorage only. The capacity of the connection system shall be the minimum of the allowable anchorage load and the FC Allowable Connector Loads.



Titen HD[®] Anchorage

FSB Bypass Framing Fixed-Clip Strut Connector

SIMPSON Strong-Tie

Rigid Connectors

The FSB connector is the fixed-clip version of our popular SSB slide-clip strut connector. The FSB is commonly used at the bottom flange of a steel beam to accommodate large stand-off distances for bypass curtain-wall studs.

Material: 54 mil (16 ga.)

Finish: Galvanized (G90)

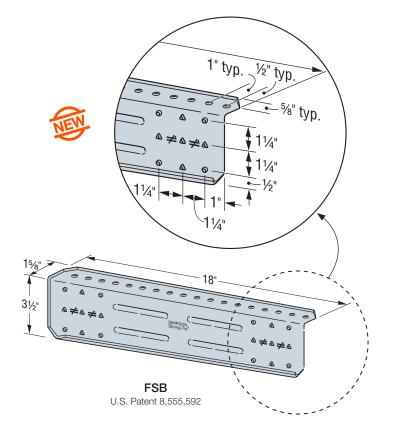
Installation:

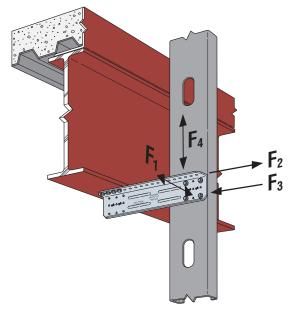
- Use the specified type and number of anchors.
- Use the specified type and number of screw fasteners to the stud.
- If the FSB intrudes on interior space, it can be trimmed. The trimmed part shall allow an edge distance of ½" or greater from the center of the nearest anchor to the end of the trimmed part.

Codes: See p. 11 for Code Reference Key Chart

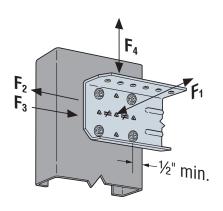
Ordering Information:

FSB3.518-R25 is a box of 25 connectors.





Typical FSB3.518 Installation



FSB Installation with the Min. Number of Fasteners

FSB Bypass Framing Fixed-Clip Strut Connector

FSB Allowable Connector Loads (lb.)

	NO. I NICKNESS Max.	No. of						Stud Th	ickness							
			MIN./ #10-16	33 mil (20 ga.)				43 mil (18 ga.)				54 mil (16 ga.)				Code Ref.
mil (ga.)	S	Screws	F1⁴	F ₂	F3	F4 ⁵	F1⁴	F ₂	F3	F4⁵	F1⁴	F ₂	F ₃	F₄⁵		
FSB3.518	54 (16)	Min.	4	120	705	705	160	150	1,050	1,050	210	200	1,670	1,615	210	170
F3D3.310	54 (10)	Max.	9	155	1,590	1,340	160	200	2,365	2,180	210	270	2,670	2,180	260	170

1. For additional important information, see General Information pp. 14-16.

2. FSB Allowable Connector Loads are also limited by the FSB Allowable Anchorage Loads table. Use the minimum value

from the connector and anchorage load tables as applicable.

3. Min. fasteners quantity and tabulated values - fill round holes; max. fasteners quantity and tabulated values - fill round and triangle holes.

4. Allowable F1 loads are based on in-plane loads applied to the stud fasteners that are nearest the support with complete rotational restraint at the studs.

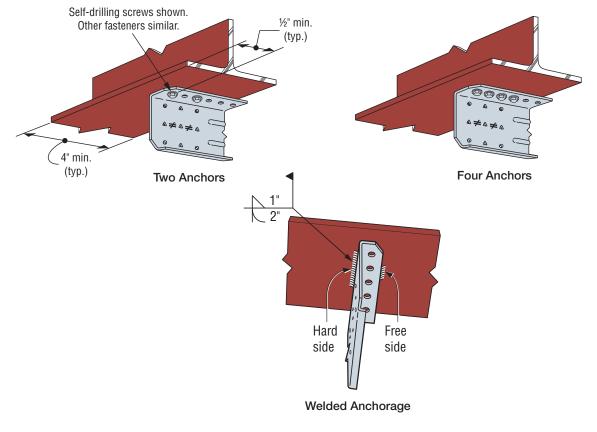
For more extensive treatment of in-plane loads please refer to Simpson Strong-Tie® engineering letter L-CF-CWCF1DIR at strongtie.com.

5. Tabulated F4 values are controlled by 1/4" deformation limit. The connector strength load in the F4 direction is 550 lb.

Anchorage Type	No. of Anchors	F1	F_2 and F_3	F4
#12-24 self-drilling screws	2	270	1,250	550
#12-24 Self-utiling Screws	4	270	2,500	550
Simpson Strong-Tie® 0.145" PDPT or 0.157" PDPAT	2	_	820	_
powder-actuated fasteners	4	270	1,640	550
Wolded	Hard side: 2"	270	0.455	550
Welded	Free side: 1"	270	2,455	550

FSB Allowable Anchorage Loads (lb.)

- 1. Allowable loads for #12–24 self-drilling screws and PDPT powder-actuated fasteners are based on installation in minimum $\frac{3}{16}$ " thick structural steel with $F_y = 36$ ksi. It is the responsibility of the Designer to select the proper length fasteners.
- Allowable loads for welded connections require E70XX electrodes with a minimum throat size equal to the clip thickness. Welding shall comply with AWS D1.3. Welding galvanized steel may produce harmful fumes; follow proper welding procedures and precautions.
- 3. Allowable loads are for anchorage only. It is the responsibility of the Designer to verify the strength and stability of the structure for loads imposed by the cold-formed steel framing connections.



Does your project require DoD-compliant blast design?

Although Allowable Strength Design (ASD) is widely used by Designers of Cold-Formed Steel (CFS) construction, some projects require additional connector limit states beyond the typical ASD values that are normally tabulated in our load tables. For example, many Department of Defense (DoD) projects require blast design of exterior wall framing and connections. Such projects may require the LRFD strength or nominal strength for the blast calculations. For a more comprehensive background on blast design please reference the Simpson Strong-Tie[®] Structural Engineering Blog at **seblog.strongtie.com** and enter the search term "DoD".



Not finding what you need? Please contact Simpson Strong-Tie.

Table 1: SCB/MSCB Bypass Framing Slide-Clip Connector — Service Load Limits, LRFD Design Strengths and Nominal Strengths

	Dimer	isions						Stud Thickness								
Madal	(ii	n.)	No. of		3:	3 mil (20 ga	ι.)	4:	3 mil (18 ga	a.)	54	4 mil (16 ga	a.)	6	8 mil (14 ga	a.)
Model No.	L	S	#14 Shldr. Screws	Load Dir.	Service Load Limit (Ib.)	LRFD Design Strength (Ib.)	Nominal Strength (Ib.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)
SCB43.5	31/2	1½	2	F ₂	860	830	1,180	975	975	1,475	990	1,215	1,795	990	1,215	1,795
30043.0	572	1 72	2	F3	1,260	830	1,180	1,260	1,105	1,570	1,260	1,565	2,180	1,260	1,565	2,180
			2	F ₂	860	785	1,115	975	975	1,475	990	1,215	1,795	990	1,215	1,795
SCB45.5	5½	2	2	F3	1,260	830	1,180	1,260	1,105	1,570	2,360	1,565	2,180	2,360	1,565	2,180
00040.0	572	2	3	F ₂	860	1,080	1,535	975	1,605	2,280	990	2,205	3,135	990	2,205	3,135
			0	F3	1,260	1,080	1,535	1,260	1,605	2,280	1,260	2,025	2,825	1,260	2,025	2,825
			2	F ₂	860	785	1,115	990	1,245	1,770	1,055	1,855	2,640	1,195	1,920	2,730
MSCB45.5	5½	2	2	F3	1,260	830	1,180	1,260	1,105	1,570	2,210	1,925	2,685	2,195	2,360	2,760
1000040.0	072	2	3	F ₂	860	1,080	1,535	1,105	1,715	2,435	1,220	2,570	3,655	1,365	2,575	3,660
				F3	1,260	1,080	1,535	1,260	1,605	2,280	2,910	3,090	3,610	2,910	3,090	3,610
			2	F ₂	860	785	1,115	975	975	1,475	990	1,215	1,795	990	1,215	1,795
SCB47.5	7½	4		F3	1,260	830	1,180	1,260	1,105	1,570	2,260	1,515	2,115	2,260	1,515	2,115
00047.0	1 /2	-	3	F2	860	1,080	1,535	975	1,605	2,280	990	2,205	3,135	990	2,205	3,135
			Ŭ	F3	1,260	1,080	1,535	1,260	1,605	2,280	2,750	2,025	2,825	2,750	2,025	2,825
			2	F2	860	785	1,115	990	1,245	1,770	1,055	1,855	2,640	1,195	1,920	2,730
MSCB47.5	71/2	4		F3	1,260	830	1,180	1,260	1,105	1,570	2,210	1,925	2,685	2,195	2,360	2,760
M00D-11.0	1 /2	-	3	F2	860	1,080	1,535	1,105	1,715	2,435	1,220	2,570	3,655	1,365	2,575	3,660
			Ŭ	F3	1,260	1,080	1,535	1,260	1,605	2,280	2,910	3,090	3,610	2,910	3,090	3,610
			2	F2	860	785	1,115	975	1,105	1,570	990	1,215	1,795	990	1,215	1,795
SCB49.5	91⁄2	6		F3	860	830	1,180	860	1,105	1,570	2,295	1,565	2,180	2,295	1,565	2,180
	- /-		3	F2	860	1,080	1,535	975	1,605	2,280	990	2,205	3,135	990	2,205	3,135
				F3	860	1,080	1,535	1,260	1,605	2,280	2,620	2,025	2,710	2,620	2,025	2,710
			2	F2	860	785	1,115	860	1,105	1,570	990	1,690	2,405	990	1,690	2,405
SCB411.5	11 1/2	8		F3	860	830	1,180	860	1,105	1,570	1,890	1,470	1,720	1,890	1,470	1,720
		-	3	F2	860	1,080	1,535	860	1,605	2,280	990	2,205	3,135	990	2,205	3,135
				F3	860	1,080	1,535	1,260	1,605	2,280	2,335	2,025	2,375	2,335	2,025	2,375

1. Tabulated values are for the connector and attachment to the stud-wall framing.

The assembly strengths are the minimum of those listed and the anchorage values listed in Table 5 on p. 66.

2. Service Load Limit is the load at 1/8" deflection for use in evaluating the performance under service-level loads.

3. LRFD Design Strength is the Nominal Strength multiplied by a resistance factor, ϕ .

4. Nominal Strength is defined in AISI S100-07.

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Table 2: SCW Head-of-Wall Slide-Clip Connector — Service Load Limits, LRFD Design Strengths and Nominal Strengths

	No. of							Stud Thickness	;				
Madal Na	Andel No. W #14 Load		Load		33 mil (20 ga.)			43 mil (18 ga.)		54 mil (16 ga.)			
Model No.	(in.)	Shidr. Screws	Direction	Service Load Limit (lb.)	LRFD Design Strength (Ib.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	
SCW3.25	31⁄4	2	F4	935	730	865	1,030	1,010	1,175	1,095	1,200	1,680	
SCW5.5	51/2	2	F4	1,125	730	1,020	1,655	1,010	1,580	1,950	1,590	2,220	
30,005.5	0 1/2	3	F4	1,125	730	1,020	1,655	1,010	1,580	2,630	1,950	2,725	

1. Tabulated values are for the connector and attachment to the stud-wall framing.

The assembly strengths are the minimum of those listed and the anchorage values listed in Table 6 on p. 67.

2. Service Load Limit is the load at 1/8" deflection for use in evaluating the performance under service-level loads.

3. LRFD Design Strength is the Nominal Strength multiplied by a resistance factor, $\boldsymbol{\phi}.$

4. Nominal Strength is defined in AISI S100-07.

Table 3A: SSB Bypass Framing Slide-Clip Strut Connector —Service Load Limits, LRFD Design Strengths and Nominal Strengths

	No. of						Stud Thickness					
Model	No. of #14	Load		33 mil (20 ga.)			43 mil (18 ga.)		54 mil (16 ga.)			
No.	Screws	Direction	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	
	2	F ₂	1,085	830	1,185	1,220	1,105	1,570	1,830	1,715	2,440	
0000 510	2	F3	1,085	830	1,185	1,220	1,105	1,570	2,240	1,540	2,150	
3303.018	33.518	F2	1,815	1,305	1,855	1,500	1,650	2,345	1,365	2,140	3,280	
	3 F3	F ₃	1,815	1,305	1,855	2,650	1,730	2,415	2,915	1,960	3,050	

See footnotes below.

Table 3B: FSB Bypass Framing Fixed-Clip Connector — Service Load Limits, LRFD Design Strengths and Nominal Strengths

				Stud Thickness										
Model	No. of #10–16	Load		33 mil (20 ga.)			43 mil (18 ga.)		54 mil (16 ga.)					
No.	#10-16 Screws	Direction	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)			
		F2	1,850	1,060	2,115	2,540	1,575	3,150	3,165	2,670	4,090			
	4	F3	2,045	1,060	2,115	2,855	1,575	3,150	3,405	2,585	3,965			
FSB3.518		F4	160	875	1,285	210	875	1,285	210	875	1,285			
F3D3.010		F ₂	2,815	2,385	4,770	3,900	3,550	7,095	4,170	4,090	6,075			
	9	F3	2,515	2,140	2,420	3,820	3,485	4,130	3,955	3,485	4,130			
		F4	160	875	1,285	210	875	1,285	260	875	1,285			

1. Tabulated values are for the connector and attachment to the stud-wall framing.

The assembly strengths are the minimum of those listed and the anchorage values listed in Table 5A and 5B on p. 66.

2. Service Load Limit is the load at 1/8" deflection for use in evaluating the performance under service-level loads.

3. LRFD Design Strength is the Nominal Strength multiplied by a resistance factor, ϕ .

4. Nominal Strength is defined in AISI S100-07.



Table 4: FCB Bypass Framing Fixed-Clip Connector — Service Load Limits, LRFD Design Strengths and Nominal Strengths

	Di	mensio	ons		No. of		Stud Thickness								
Model		(in.)		Min.	No. of #12–14	Load	:	33 mil (20 ga.)			43 mil (18 ga.)			54 mil (16 ga.)	
No.	L	S _{min}	Smax	or Max.	Self- Drilling Screws	Direction	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (Ib.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (Ib.)	Service Load Limit (lb.)	LRFD Design Strength (lb.)	Nominal Strength (lb.)
						F2	1,100	1,130	2,255	1,120	1,680	3,360	1,250	3,040	4,655
				Min.	4	F3	1,185	1,130	2,180	1,185	1,565	2,180	1,185	1,565	2,180
FCB43.5	3½	11/2	1½			F4	2,170	1,130	1,980	2,515	1,680	2,590	3,360	2,380	3,380
10040.0	072	1/2	1/2			F2	1,100	1,690	3,245	1,120	2,425	3,720	1,250	3,040	4,655
				Max.	6	F3	1,260	1,690	3,245	1,260	2,425	3,720	3,365	3,040	4,010
						F4	2,415	1,690	2,635	2,860	2,330	3,315	2,865	3,055	4,345
						F2	1,100	1,130	2,255	1,120	1,680	3,360	1,105	2,680	4,110
				Min.	4	F3	1,185	1,130	2,180	1,185	1,565	2,180	1,185	1,565	2,180
FCB45.5	5½	2	2			F4	1,225	1,130	1,720	1,500	1,515	2,150	2,100	2,120	3,015
10040.0	J 72	2	2			F2	1,100	2,025	2,880	1,120	2,425	3,720	1,105	2,680	4,110
				Max.	9	F3	1,260	2,285	3,245	1,260	2,425	3,720	3,365	2,870	4,010
						F4	2,570	1,890	2,685	2,965	2,375	3,380	3,505	3,080	4,380
						F2	1,100	1,130	2,255	1,120	1,680	3,360	1,105	2,680	4,110
				Min.	4	F3	1,185	1,130	2,115	1,185	1,515	2,115	1,185	1,515	2,115
FCB47.5	7½	4	1½			F4	380	330	660	380	495	830	380	580	830
10047.0	1 72	4	172			F2	1,100	2,025	2,880	1,120	2,425	3,720	1,105	2,680	4,110
				Max.	12	F3	1,260	2,285	3,245	1,260	2,425	3,720	4,170	2,870	3,780
						F4	1,875	1,060	2,120	1,875	1,575	3,155	1,875	2,3w10	3,440
						F2	1,100	1,130	2,255	1,120	1,680	3,360	1,105	2,680	4,110
				Min.	4	F3	1,185	1,130	2,180	1,185	1,565	2,180	1,185	1,565	2,180
FCB49.5	9½	6	1½			F4	380	255	510	380	380	760	380	580	830
10049.0	572		172			F2	1,100	2,025	2,880	1,120	2,425	3,720	1,105	2,680	4,110
				Max.	12	F3	1,260	2,285	3,245	1,260	2,425	3,720	4,345	2,870	4,010
						F4	2,290	1,120	2,245	2,290	1,670	3,020	2,290	2,125	3,020
						F2	1,100	1,130	2,255	1,120	1,680	3,360	1,105	2,680	4,110
				Min.	4	F3	1,185	1,130	1,720	1,185	1,470	1,720	1,185	1,470	1,720
FCB411.5	11 1/2	8	11/2			F4	380	210	415	380	310	620	380	580	830
100411.0	1172	0	1 72			F2	1,100	2,025	2,880	1,120	2,425	3,720	1,105	2,680	4,110
				Max.	12	F3	1,260	2,285	3,245	1,260	2,425	3,645	4,015	2,870	3,645
						F4	2,290	1,195	2,390	2,290	1,780	3,020	2,290	2,125	3,020

1. Tabulated values are for the connector and attachment to the stud-wall framing.

The assembly strengths are the minimum of those listed and the anchorage values listed in Table 5 and 6 on pp. 66, 67.

2. Service Load Limit is the load at 1/8" deflection for use in evaluating the performance under service-level loads.

3. LRFD Design Strength is the Nominal Strength multiplied by a resistance factor, $\boldsymbol{\phi}.$

4. Nominal Strength is defined in AISI S100-07.



Table 5A: Anchor Values (lb.) in the F_2 and F_3 Direction for SCB, FCB, MSCB and SSB

	Minimum		SCB ar	nd FCB	MS	СВ	S	SB
Anchorage Type	Base Material	No. of Anchors	LRFD Design Strength	Nominal Strength	LRFD Design Strength	Nominal Strength	LRFD Design Strength	Nominal Strength
#12 self-drilling screws		2	1,270	2,010	1,635	2,505	1,875	3,280
Simpson Strong-Tie® X Metal screws	A36 steel ¾6" thick	3	1,790	2,745	2,440	3,740	2,140	3,280
XQ1S1214, X1S1214		4	2,540	4,015	3,265	5,005	_	_
Simpson Strong-Tie #12–24 x 11⁄4"		2	1,780	2,730	2,175	3,330	_	—
Strong-Drive® XL Large-Head Metal screws	A36 steel ¾6" thick	3	2,630	4,035	3,260	5,000	—	—
XLQ114T1224, XLQ114B1224-2K		4	3,565	5,465	4,345	6,660	—	—
Simpson Strong-Tie		2	700	1,130	830	1,710	1,980	3,435
0.157" x 5%" power-actuated fasteners	A36 steel 3/16" thick	3	935	1,550	1,250	2,430	2,240	3,435
PDPAT-62KP		4	1,430	2,175	1,665	3,415	_	—
Simpson Strong-Tie	A572 or	2	935	1,435	975	1,710	—	—
0.157" x 5%" power-actuated fasteners	A992 steel	3	1,275	2,000	1,465	2,430	_	_
PDPAT-62KP	⅔ı6" thick	4	1,875	2,870	1,950	3,415	_	_
Simpson Strong-Tie		2	605	1,515	605	1,515	_	—
1/4" x 13/4" Titen® Hex-Head Masonry screws	Concrete f' _c = 2,500 psi	3	710	1,775	710	1,775	_	_
TTN25134H		4	815	2,035	815	2,035	_	_
Welded E70XX electrodes	A36 steel ¾6" thick	Hard side: 2" Free side: 1"	2,780	4,265	3,265	5,005	3,320	5,085

See footnotes below.

Table 5B: Anchor Values (lb.) in F_1 , F_2 , F_3 , and F_4 Direction for FSB

		FSB								
Anchorage Type	No. of	F	1	F2 /	′ F3	F4				
	Anchors	LRFD Design Strength	Nominal Strength	LRFD Design Strength	Nominal Strength	LRFD Design Strength	Nominal Strength			
#12-14 self-drilling screws	2	435	495	1,875	3,200	875	1,285			
#12-14 sen-unning screws	4	435	495	3,745	6,075	875	1,285			
Simpson Strong-Tie 0.157" x 5%" power-actuated fasteners	2	—	—	1,310	2,530	—	—			
PDPAT-62KP	4	435	495	2,620	6,075	875	1,285			
Welded	Hard side: 2" Free side: 1"	435	495	5,330	6,020	875	1,285			

1. Tabulated values are for the anchorage only. The assembly strengths are the minimum of those listed above

and the connector capacity and attachment to the stud-wall framing listed in Tables 1-4 on pp. 63-65.

2. LRFD Design Strength is the Nominal Strength multiplied by a resistance factor, $\boldsymbol{\phi}.$

3. Nominal Strength is defined in AISI S100-07.

4. For the Service Load Limit, use the values listed for the connectors in Tables 1–4 on pp. 63–65.

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Table 6: Anchor Values (lb.) in the F₄ Direction

Anoberezza		land	FCB43.5	FCB45.5	FCB	47.5	FCB	49.5	FCB4	411.5		
Anchorage Type	No. of Anchors	Load Type	Min./ Max.	Min./ Max.	Min.	Max.	Min.	Max.	Min.	Max.	SCW3.25	SCW5.5
	_	LRFD Design Strength	1,000	655	410	710	295	425	190	305	1,020	1,875
	2	Nominal Strength	1,530	1,005	625	1,090	455	650	295	465	1,565	2,945
#12-24 self-drilling	3	LRFD Design Strength	1,105	720	450	785	320	470	215	335	1,330	
screws	5	Nominal Strength	1,690	1,105	685	1,200	490	725	330	515	2,040	_
	4	LRFD Design Strength	2,010	1,310	785	1,425	570	855	440	610	_	2,295
	т -	Nominal Strength	3,075	2,010	915	2,180	665	1,310	515	930	_	3,805
	2	LRFD Design Strength	655	425	265	465	190	280	120	200	830	1,470
	2	Nominal Strength	1,005	650	405	710	295	430	185	305	1,270	2,255
Simpson Strong-Tie [®] 0.157" PDPAT	3	LRFD Design Strength	745	490	305	530	215	310	135	225	895	_
powder-actuated fasteners		Nominal Strength	1,140	745	465	810	330	480	210	345	1,370	—
	4	LRFD Design Strength	1,345	880	545	950	390	570	230	410	_	2,385
		Nominal Strength	2,060	1,350	835	1,460	600	870	355	625	_	3,660
	2	LRFD Design Strength	580	380	230	415	170	245	300	175	_	
	Z	Nominal Strength	1,660	1,080	660	1,180	480	700	400	500	_	_
Simpson Strong-Tie ¼" x 1¾"	3	LRFD Design Strength	660	435	265	470	195	280	140	205	_	
Titen [®] hex-head screws	5	Nominal Strength	1,880	1,240	700	1,340	515	800	400	580	_	_
screws	4	LRFD Design Strength	905	590	365	635	265	385	390	275	_	
	4	Nominal Strength	2,580	1,680	915	1,820	670	1,100	525	780	_	_
Welded	Hard side: 2"	LRFD Design Strength	5,070	3,320	1,265	3,210	910	1,920	720	1,375	_	
Woldou	Free side: 1"	Nominal Strength	7,765	5,085	1,475	3,750	1,065	2,245	1,280	1,610	_	—

1. Tabulated values are for the anchorage only. The assembly strengths are the minimum of those listed above

and the connector capacity and attachment to the stud-wall framing listed in Tables 1-4 on pp. 63-65.

2. LRFD Design Strength is the Nominal Strength multiplied by a resistance factor, $\phi.$

3. Nominal Strength is defined in AISI S100-07.

4. For the Service Load Limit, use the values listed for the connectors in Tables 1-4 on pp. 63-65.

Raise Your Expectations, Lower Your Installed Costs!

SIMPSON Strong-Tie

Utility Clip Connectors

The SSC steel stud connector, the SJC steel joist connector and the SFC steel framing connector, are designed so that a minimum number of clips can be stocked to accommodate a wide array of applications. Prepunched holes and intuitive fastener hole patterns ensure that the structural needs of the Designer and the efficient installation goals of the contractor are both satisfied.

Rigid Connectors

Testing You Can Trust

Simpson Strong-Tie® utility clip connectors have undergone industry-first testing to provide maximum benefit to both the installer and the Designer. By testing these connectors as part of a complete system in the applications for which they are intended, rather than only testing the physical capabilities of the connector, Simpson Strong-Tie is able to provide comprehensive allowable loads for real-world conditions. This system-based approach eliminates the need for Designers to manually calculate connector performance and anchorage, and provides confidence that designs based on these values have been thoroughly evaluated by the industry leader in structural connector research and development.

	Р	roduct Catego	ſy
Tested Application	SSC O	SJC	SFC
Steel-to-steel	\checkmark	\checkmark	\checkmark
Bypass Framing	\checkmark		
Headers	\checkmark		
Base of Jamb	\checkmark		
Rafter	\checkmark		
Kneewall	\checkmark		
U-Channel Bridging	\checkmark		\checkmark
Kicker		\checkmark	
Soffit Hanger	\checkmark	\checkmark	

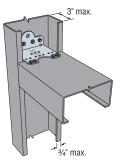
Innovative Design Lets You Work Smarter — Not Harder!

Simpson Strong-Tie® utility clip connectors have been designed with both the contractor and Designer in mind. Connector dimensions and fastener/anchor locations have been developed to maximize design flexibility and installation efficiency.

Intelligent Connector Dimensions

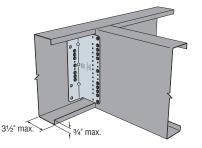
SSC Steel Stud Connectors

Designed to accommodate open-side connections with flanges up to 3" wide and stiffener lips up to 3/4" *



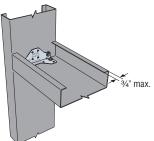
SJC Steel Joist Connectors

Designed to accommodate open-side connections with flanges up to 31/2" wide and stiffener lips up to 3/4"



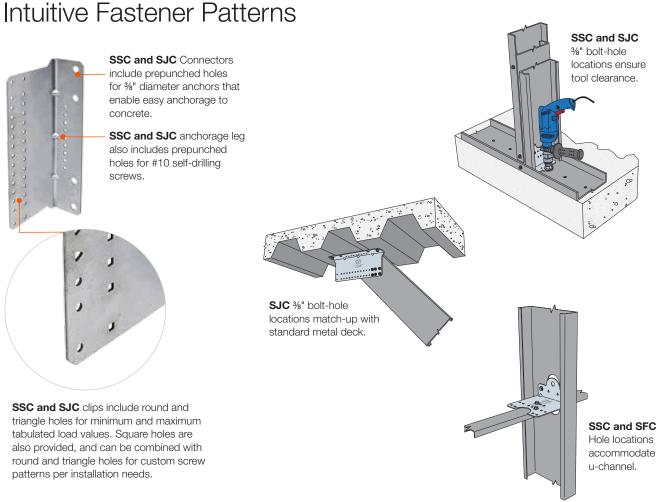
SFC Steel Framing Connectors

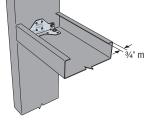
Designed to accommodate open-side connections with stiffener lips up to 3⁄4" long **



For detailed product dimensions, refer to pp. 90-91.

*SSC2.25 clips will accommodate 2" wide flange and 5%" stiffener lips.





**SFC2.25 clips will accommodate %" long stiffener lips.

SIMPSON

Strong-

SSC Steel-Stud Connector



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

SSC connectors are versatile utility clips ideal for a variety of stud-to-stud and stud-to-structure applications in cold-formed steel

construction. The clips have been designed to enable easy installation on the open side of studs or joists with flanges up to 3" long and return lips up to 3". A wide pattern of strategic fastener locations allows the SSC to accommodate a variety of traditional and custom designs.

Features:

Rigid Connectors

- Prepunched holes reduce installation cost by eliminating predrilling
- Intuitive fastener hole positions ensure accurate clip installation in accordance with design, support a wide range of design and application requirements and provide installation flexibility
- Angle lengths accommodate either hard-side or soft-side attachment for studs and joists with return lips up to $3\!\!4"^*$
- + 4" leg length enables soft-side connections for studs and joists with flanges up to 3"*
- Also suitable for u-channel bridging

Product Information:

Material: LSSC - 54 mil (50 ksi); SSC - 68 mil (50 ksi); MSSC - 97 mil (50 ksi)

Finish: Galvanized (G90)

Installation: Use all specified fasteners/anchors

Codes: See p. 11 for Code Reference Key Chart

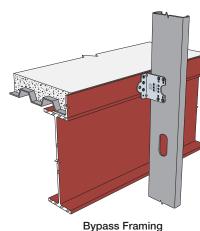
For detailed product dimensions, refer to p. 90.

Ordering Information

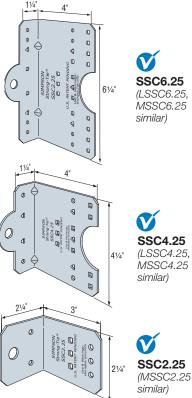
5		
Model No.	Ordering SKU	Package Quantity
SSC2.25	SSC2.25-R1251	Bucket of 125
MSSC2.25	MSSC2.25-R901	Bucket of 90
LSSC4.25	LSSC4.25-R50 ²	
SSC4.25	SSC4.25-R50 ²	Bucket of 50
MSSC4.25	MSSC4.25-R50 ²	
LSSC6.25	LSSC6.25-R30	
SSC6.25	SSC6.25-R30	Bucket of 30
MSSC6.25	MSSC6.25-R30	

1. By leaving off the "-RXXX" suffix, items can also be ordered in cartons of 50.

2. By leaving off the "-RXX" suffix, items can be ordered in cartons of 25.



Bypass Framing with Stud Strut

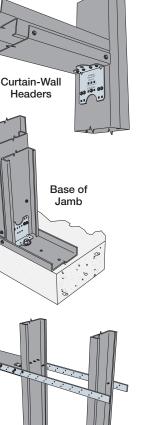


SIMPSON

Strong-Tie

U.S. Patent Pending

.oad–Bearing Headers



*SSC2.25 clips will accommodate 2" wide flange and %" stiffener lips.

SSC Steel-Stud Connector



SSC Connectors — Steel-to-Steel Allowable Loads

	Connector		Framing		Fasteners			Allowable F	4 Load (lb.)		
Model	Material	L	Member		Carried	Carrying	Minim	um Member Thi	ckness	Maximum	Code
No.	Thickness mil (ga.)	(in.)	Depth (in.)	Pattern ¹	Pattern ¹ Member Member		33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	Connector Load ³	Ref.
SSC2.25	68 (14)	21⁄4	3%	Min.	(3) #10	(2) #10	165	225	345	690	
MSSC2.25	97 (12)	21⁄4	3%	Min.	(3) #10	(2) #10	165	225	345	690	
				Min.	(2) #10	(2) #10	215	440	675		
LSSC4.25	54 (16)	41⁄4	6	Max.	(5) #10	(4) #10	215	440	725	1,615	
				Outer	(4) #10	(4) #10	200	310	520	1	
				Min.	(2) #10	(2) #10	355	525	890		
SSC4.25	68 (14)	4¼	6	Max.	(5) #10	(4) #10	365	600	1,005	1,615	
				Outer	(4) #10	(4) #10	235	330	625		
	97 (12)	41⁄4		Min.	(2) #10	(2) #10	355	525	890		
MSSC4.25			6	Max.	(5) #10	(4) #10	365	600	1,005	1,615	IP2
				Outer	(4) #10	(4) #10	235	330	625		IFZ
				Min.	(4) #10	(4) #10	265	660	1,190		
LSSC6.25	54 (16)	6¼	8	Max.	(7) #10	(6) #10	265	660	1,190	2,590	
				Outer	(6) #10	(4) #10	270	375	695		
				Min.	(4) #10	(4) #10	385	720	1,190		
SSC6.25	68 (14)	6¼	8	Max.	(7) #10	(6) #10	385	720	1,190	2,590	
				Outer	(6) #10	(4) #10	270	460	725		
				Min.	(4) #10	(4) #10	385	720	1,190		
MSSC6.25	97 (12)	6¼	8	Max.	(7) #10	(6) #10	385	720	1,365	2,590	
				Outer	(6) #10	(4) #10	270	460	725		

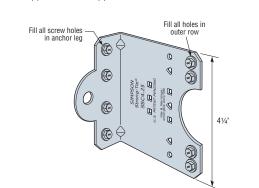
1. Min. fastener quantity and load values - fill all round holes; Max. fastener quantity and load values - fill all round and triangular holes; Outer fastener quantity and load values - see illustrations for fastener placement.

2. Allowable loads are based on bracing of the members located within 12" of the connection.

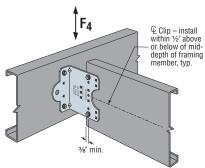
3. Maximum allowable load for connector that may not be exceeded when designing custom installations.

Designer is responsible for member and fastener design.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners. 5. Reference pp. 76-81 for supplemental information and alternate screw patterns.



SSC4.25 - Outer Fastener Pattern (LSSC4.25 and MSSC4.25 similar)



Typical SSC Installation

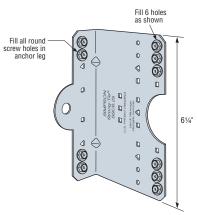
SSC Installation with Carried Member Fasteners in Outer Row

3∕8" min

F4

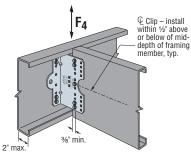
8 8 8

3" max



SSC6.25 - Outer Fastener Pattern (LSSC6.25 and MSSC6.25 similar)

€ Clip – install within ½" above or below of mid-depth of framing member, typ.



SSC Installation with Carried Member Fasteners in Inner Row

SSC Steel-Stud Connector



SSC Connectors - Bypass Framing Allowable Loads

	Connector		Fastanar	o1.4	Stud Thickness															
Model No.	Material L Thickness (in.) _ mil (ga.)	L (in.)	Fasteners ^{1,4}		33 mil (20 ga.)			43 mil (18 ga.)			54 mil (16 ga.)				Code Ref.					
		Anchorage ²	Stud	F1 ³	F ₂	F ₃	F4	F1 ³	F ₂	F3	F4	F1 ³	F ₂	F3	F4					
SSC4.25	68 (14) 41⁄4	68 (14) 41⁄4	68 (14) 41⁄4	68 (14) 41⁄4	69 (14) 414	(3) #10	(4) #10	40	705	705	700	40	870	1,050	850	40	935	1,210	850	IP2
3304.20					00 (14) 4/4	68 (14) 4 1/4	(3) PDPAT-62K	(4) #10	40	705	705	700	40	780	1,050	850	40	780	1,210	850
MCCCADE	ISSC4.25 97 (12) 41/4	41/	(3) #10	(4) #10	105	705	705	705	105	1,050	1,050	880	105	1,385	1,210	880	IP2			
1013304.25		97 (12) 41/4		(3) PDPAT-62K	(4) #10	105	705	705	705	105	780	1,050	880	105	780	1,210	880	160		

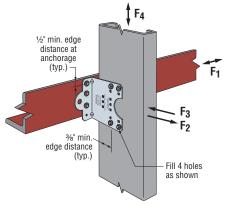
1. See illustration for fastener placement.

2. Allowable loads are based on anchors installed in

minimum $\%_{\rm F}$ -thick structural steel with $F_y=36$ ksi. 3. Allowable loads based on in-plane loads applied at the centroid of the fasteners to the stud, with no

at the centroid of the fasteners to the stud, with no rotational restraint of stud.
4. See pp. 138 through 171 for more information

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Typical SSC Installation

Connector		Connector		Fasteners ⁴			Jamb and	Allowable F	3 Load (lb.)	Allowable	
Model No.	Material Thickness mil (ga.)	L (in.)	Stud Depth (in.)	Pattern	Jamb	Header	Header Thickness mil (ga.)	Nested Stud and Track Header ³	Back to Back Header²	F4 Load (lb.)	Code Ref.
LSSC4.25	E4 (10)	41/	6	Max	(5) #10	(4) #10	33 (20)	140	455	215	
LSSU4.20	54 (16)	41⁄4	0	Max.	(5) #10	(4) #10	43 (18)	220	660	440	
SSC4.25	68 (14)	41⁄4	6	Max.	(5) #10	(4) #10	54 (16)	375	1,055	1,005	
3364.20	00 (14)	4 /4	0	Iviax.	(3) #10	(4) #10	68 (14)	570	1,055	1,005	IP2
LSSC6.25	E4 (1C)	6¼	8	Max.	(7) #10	(6) #10	33 (20)	160	455	265	IF Z
L3300.20	54 (16)	0 /4	0	IVIAX.	(7) #10	(6) #10	43 (18)	250	730	660	
SSC6.25	68 (14)	6¼	8	Max.	(7) #10	(6) #10	54 (16)	410	1,110	1,190	
0000.20	00 (14)	0 /4	0	IVIdX.	(7)#10	(0) #10	68 (14)	640	1,110	1,190	

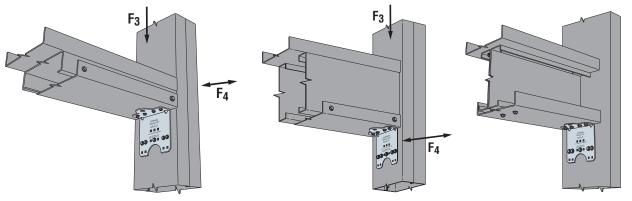
SSC Connectors — Headers Allowable Loads

1. Max. fastener quantity and load values-fill all round and triangular holes.

2. Designer is responsible for checking web crippling of the header and reducing allowable loads accordingly.

3. Also applies to box header per illustration below.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Box Header

Back-to-Back Header

SSC Steel-Stud Connector



SSC Connectors - Base of Jamb Allowable Loads

Model	Connector		Stud Member	Faste	eners	Stud	Allowable	Code						
No.	Material Thickness mil (ga.)	(in.)	Depth (in.)	Anchor Diameter			F4 Load (Ib.)	Ref.						
						33 (20)	390							
SSC2.25	68 (14)	21⁄4	3%	3⁄8	(3) #10	43 (18)	605							
						54 (16)	940							
						33 (20)	420							
SSC4.25	68 (14)	41⁄4	6	3⁄8	(5) #10	43 (18)	685	IP2						
						54 (16)	975							
						33 (20)	470							
SSC6.25	68 (14)	61⁄4	8	3⁄8	(7) #10	43 (18)	715							
												54 (16)	1,020	

1. Allowable loads are based on minimum 33 mil (20 ga.) track for 33 mil (20 ga.) and 43 mil (18 ga.) studs,

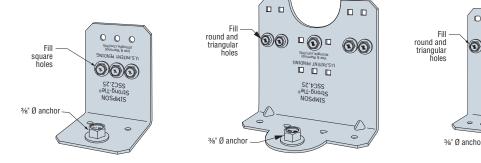
and minimum 43 mil (18 ga.) track for 54 mil (16 ga.) studs, with one #10 screw into each stud flange.

2. Allowable loads assume adequate torsional bracing is provided. Bracing design is the responsibility of the Designer.

3. See illustrations for fastener placement.

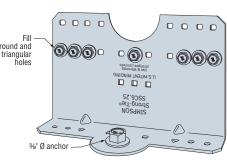
4. Designer is responsible for anchorage design.

5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



SSC2.25 Fastener Pattern

SSC4.25 Fastener Pattern



SSC6.25 Fastener Pattern

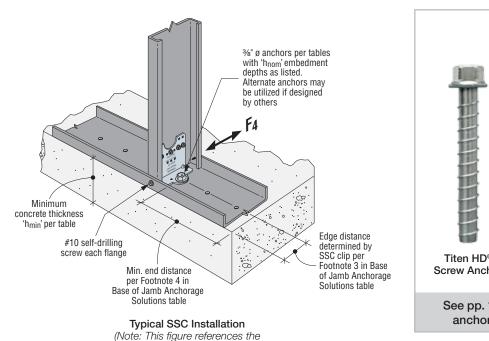


table on the following page.)



anchor solutions for this application.

SSC Steel-Stud Connector

Base of Jamb Anchorage Solutions

	Uncracked Concrete, Wind and Seismic in SDC A&B [®]													
Model No.	Minimum Concrete Thickness (h _{min})	¾" Diameter Simpson Strong-Tie®	Nominal Embedment	Allow	able Anchor Load, F	4 (lb.)								
Model No.	(in.)	Anchor Type	Depth (h _{nom}) (in.)	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC								
	4	Titen HD®	21/2	275	455	530								
SSC2.25		Titen HD	31⁄4	290	485	560								
5562.25	6	SET-XP®	4	345	510	590								
	6	AT-XP®	4	345	510	590								
	4	Titen HD	21/2	550	920	975								
SSC4.25		Titen HD	31⁄4	620	975	975								
3364.23	6	SET-XP	4	735	880	880								
		AT-XP	4	735	880	880								
	4	Titen HD	21/2	735	1,020	1,020								
SSC6.25		Titen HD	31⁄4	960	1,020	1,020								
3300.20	6	SET-XP	4	880	880	880								
		AT-XP	4	880	880	880								

Cracked Concrete, Wind and Seismic in SDC A&B⁸

Model No.	Minimum Concrete	3/8" Diameter	Nominal Embedment	Allowable Anchor Load, F ₄ (lb.)					
would no.	Thickness (h _{min}) (in.)	Simpson Strong-Tie® Anchor Type	Depth (h _{nom}) (in.)	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC			
	4	Titen HD	21/2	195	325	375			
SSC2.25		Titen HD	31⁄4	210	345	400			
5562.25	6	SET-XP	4	245	360	420			
		AT-XP	4	245	360	420			
	4	Titen HD	21/2	395	655	760			
SSC4.25		Titen HD	31⁄4	445	740	855			
5564.20	6	SET-XP	4	525	775	880			
		AT-XP	4	525	775	880			
	4	Titen HD	21/2	525	875	1,010			
SSC6.25		Titen HD	31⁄4	685	1,020	1,020			
5500.25	6	SET-XP	4	810	880	880			
		AT-XP	4	810	880	880			

Cracked Concrete, Seismic in SDC C through F⁹

Model No.	Minimum Concrete Thickness (h _{min})	3/8" Diameter	Nominal Embedment	Allow	able Anchor Load, F	4 (lb.)
Model No.	(in.)	Simpson Strong-Tie® Anchor Type	Depth (h _{nom}) (in.)	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC
	4	Titen HD	21/2	90	150	175
SSC2.25		Titen HD	31⁄4	95	160	185
5562.25	6	SET-XP	4	115	170	195
		AT-XP	4	115	170	195
	4	Titen HD	21/2	185	305	355
SSC4.25		Titen HD	31⁄4	205	345	400
5564.20	6	SET-XP	4	245	355	355
		AT-XP	4	245	350	350
	4	Titen HD	21/2	245	410	470
0000.05		Titen HD	31⁄4	320	480	480
SSC6.25	6	SET-XP	4	355	355	355
		AT-XP	4	350	350	350

1. Allowable anchor capacities have been determined using ACI 318-14 Chapter 17 calculations with the minimum concrete compressive strength, f⁺_c and slab thickness listed. Sand-lightweight concrete is abbreviated as "SLWC" while normal-weight concrete is abbreviated as "NWC".

2. Nominal Embedment Depth/Effective Embedment Depth relationships:

- %" Titen HD® in 4" Slab : 2.50" (h_{nom}) / 1.77" (h_{ef})

- %" Titen HD in 6" Slab or thicker : 3.25" (hnom) / 2.40" (hef)

- SET-XP® or AT-XP® Adhesive with %" F1554 Gr. 36 All-Thread Rod in 6" Slab or thicker : 4.0" (hnom) = 4" (hef)

3. Edge distances are assumed to be 1.81", 3.0" and 4.0" (½ of stud width) as determined for 35%", 6" and 8" studs, respectively.

4. End distances are assumed as 1.5 x Min. Edge Distance in one direction and 'N/A' in the other direction. See figure on p. 73.

5. Load values are for a single anchor based on ACI 318-14, condition B, load factors from ACI 318-14 Section 5.3, no supplemental edge reinforcement, ψ_{C} , v = 1.0 for cracked concrete and periodic special inspection. Reference ICC-ES or IAPMO-UES evaluation reports for further information.

 Load values are based on a short-term temperature range of 150°F and 180°F for SET-XP and AT-XP. Long-term temperature range is assumed to be 110°F for both SET-XP and AT-XP. Dry hole conditions are assumed. Other conditions may be evaluated using Anchor Designer[™] Software for ACI 318, ETAG and CSA. See strongtie.com/software.

 Allowable Stress Design (ASD) values were determined by multiplying calculated LRFD capacities by a conversion factor, Alpha (α), of 0.7 for seismic loads and 0.6 for wind loads. ASD values for other load combinations may be determined using alternate conversion factors.

8. Tabulated allowable ASD loads for Wind and Seismic in SDC A&B are based on using wind conversion factors and may be increased by 1.17 for SDC A & B only.

9. Allowable loads have been divided by an Omega (Ω) seismic factor of 2.5 for brittle failure as required by ACI 318-14 Chapter 17.

10. Allowable F₄ load based on loading direction towards the edge of slab.

11. Tabulated capacities are based on maximum allowable anchorage loads only. The capacity of the connection system shall be the minimum of the tabulated value and the allowable load value from the SSC Connectors: Base of Jamb Allowable Load Tables.

SSC Steel-Stud Connector

SSC Connectors - Rafters Allowable Loads

	Connector		Faste	ners ^{1,4}	ļ	Allowable Load (Ib	.)		
Model No.	Material Thickness	L (in.)	Anchorage to	Supported		43 mil (18 ga.)		Code Ref.	
	mil (ga.)	()	Steel ²	Member	F ₂	F ₃	F4		
SSC4.25	69 (14)	/1/	(2) #12	(5) #10	710	1,075	595		
3304.25	68 (14)	41⁄4	4 /4	(4) PDPAT	(5) #10	1,020	1,075	630	IP2
MSSC4 OF	07 (10))7 (12)	(2) #12	(5) #10	710	1,335	595	1172	
1010004.20	ISSC4.25 97 (12)		(4) PDPAT	(5) #10	1,025	1,335	815		

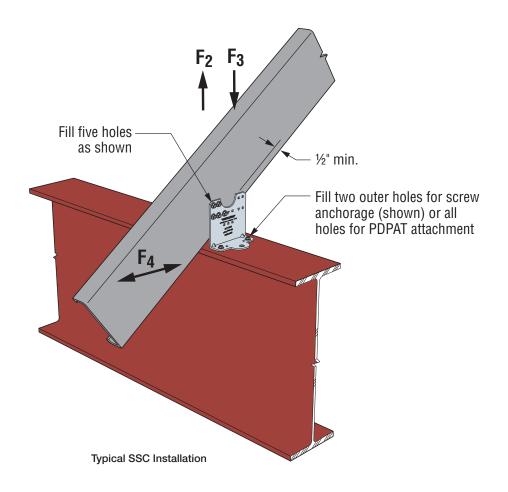
1. See illustrations for fastener placement.

2. Allowable loads are based on anchors installed in minimum $\frac{1}{16}$ -thick structural steel with $F_y = 36$ ksi.

3. Allowable loads are based on a 6"-deep member. For deeper members, Designer must consider web crippling

of the member and reduce loads accordingly.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

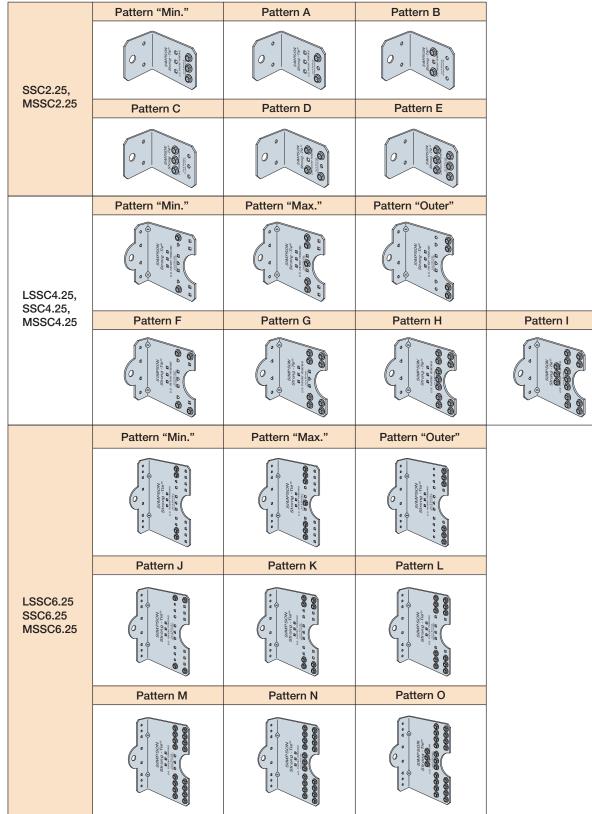


SIMPSON

Strong-Tie

The following SSC supplemental information is given to help Designers with value-engineered solutions for our SSC connectors. Loads are given for fastener patterns other than our standard "min." (fill all round holes) and "max." (fill all round and triangle holes). The tables give service, ASD, LRFD and nominal loads.

Table 1: SSC Screw Patterns

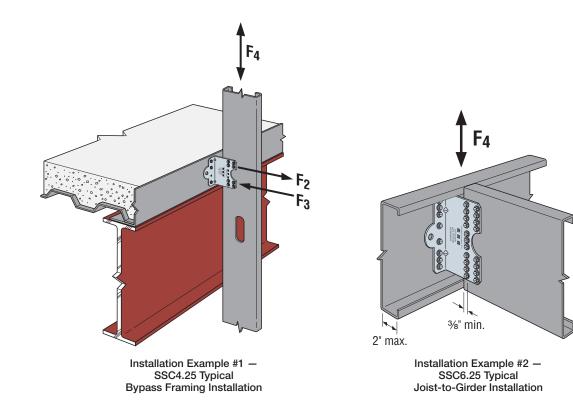


SIMPSON

Strong-Tie

Footnotes for Tables 2, 3, 4 and 5

- Calculated values are per AISI RP15-2, AISI S-100, or generally accepted industry standards. Shaded values for F4 are derived from test data. Whenever possible, unshaded F4 values are based on the maximum calculated value and applicable tested value.
- The tabulated values do not account for anchorage to the support. Anchor strength must be calculated separately and may reduce the capacity of the connection when compared to the tabulated values.
- 3. Tabulated values do not include shear, web crippling, buckling, or other local effects in the member. The Designer must check member limit states separately.
- 4. For load combinations that include F4 and/or F2 and/or F3, use an appropriate interaction equation.
- 5. #10–16 screws shall have $P_{SS} \ge 1,620$ lb. Calculated values are per AISI S-100. Screws must be installed with three (min.) exposed threads.
- 6. The number of screws is for one clip leg that is attached to the supported stud.
- 7. For the minimum screw pattern, fill all round holes. For the maximum screw pattern, fill all round and triangle holes. Reference p. 76.
- 8. In addition to calculations of net and gross section tension, and screw shear of the clip leg attached to the stud, F₂ values are also calculated for weak-axis bending of the anchored clip leg with the line of bending at the smaller anchor holes. The Designer is responsible for calculating pullover, pullout, and tension strength of the anchors, and this may reduce F₂ strength compared to the tabulated values.
- 9. F₃ values are computed using the plate buckling provisions of AISI RP15-2.
- 10. For the F_4 calculated values, it's assumed that the connection eccentricity is taken by screws in the supported stud.
- 11. Service load limits for F₂ and F₃ are not given since there are no generally accepted industry methods available to compute these values. F₄ service load limits are based on AISI Research Report RP15-2 for ½" deflection or applicable test data.
- 12. For 50 ksi studs, 68 mil (14 ga.) and thicker, use tabulated values for 54 mil (16 ga.) 50 ksi studs.



Strong

77



Table 2: SSC Steel Stud Connectors (SSC2.25 and MSSC2.25) - Service Load Limits and Strengths (lb.)

	No. of			0 amrila a				Stud Thick	ness and Yie	d Strength			
Model No.	No. of #10	Screw Pattern	Load Direction	Service Load	33 m	il (20 ga.) – 3	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi
NO.	Screws	ration	Direction	Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F ₂		235	350	390	235	350	390	235	350	390
	3	Min.	F3		455	740	1,475	455	740	1,475	455	740	1,475
			F4	810	165	265	295	225	360	410	345	550	675
			F2		235	350	390	235	350	390	235	350	390
	2	А	F3	_	330	495	990	355	575	1,150	355	575	1,150
			F4	190	65	95	190	100	145	295	195	295	585
			F ₂		235	350	390	235	350	390	235	350	390
	2	В	F3	_	330	495	990	465	755	1,515	465	755	1,515
0000.05			F4	310	100	150	300	155	235	465	310	465	935
SSC2.25			F ₂	_	235	350	390	235	350	390	235	350	390
	3	С	F3		495	745	1,485	600	975	1,945	600	975	1,945
			F4	810	165	265	310	225	360	480	345	550	960
			F2		235	350	390	235	350	390	235	350	390
	4	D	F3		465	755	1,515	465	755	1,515	465	755	1,515
			F4	810	180	270	535	275	415	830	555	830	1,660
			F ₂		235	350	390	235	350	390	235	350	390
	6	Е	F3	_	600	975	1,945	600	975	1,945	600	975	1,945
			F4	810	230	350	695	360	540	1,075	690	1,075	1,830
			F ₂		475	710	790	475	710	790	475	710	790
	3	Min.	F3		495	745	1,485	765	1,150	2,295	785	1,280	2,560
			F4	810	165	265	295	225	360	410	345	550	675
			F2	—	330	495	790	475	710	790	475	710	790
	2	А	F ₃		330	495	990	510	765	1,530	610	995	1,990
			F4	270	65	95	190	100	145	295	195	295	585
			F ₂		330	495	790	475	710	790	475	710	790
	2	В	F3	_	330	495	990	510	765	1,530	810	1,315	2,625
			F4	445	100	150	300	155	235	465	310	465	935
MSSC2.25			F ₂		475	710	790	475	710	790	475	710	790
	3	С	F3		495	745	1,485	765	1,150	2,295	1,040	1,690	3,375
			F4	810	165	265	310	225	360	480	345	550	960
			F2		475	710	790	475	710	790	475	710	790
	4	D	F ₃		660	990	1,980	810	1,315	2,625	810	1,315	2,625
			F4	810	180	270	535	275	415	830	555	830	1,660
			F2		475	710	790	475	710	790	475	710	790
	6	E	F3		990	1,485	2,970	1,040	1,690	3,375	1,040	1,690	3,375
			F4	810	230	350	695	360	540	1,075	690	1,075	1,830

See footnotes on p. 77.

Connectors for Cold-Formed Steel Construction

SSC Supplemental Information

Table 3: SSC Steel Stud Connectors (LSSC4.25 and SSC4.25) -Service Load Limits and Strengths (lb.)

No of							Stud Thick	tud Thickness and Yield Strength						
Model No.	No. of #10	Screw Pattern	Load Direction	Load	33 mi	il (20 ga.) – 3	33 ksi	43 m	il (18 ga.) – 3	33 ksi	54 m	il (16 ga.) –	50 ksi	
	Screws			Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal	
			F ₂	—	330	495	725	435	650	725	435	650	725	
	2	Min.	F3	—	250	410	815	250	410	815	250	410	815	
			F4	2,135	215	345	490	440	705	800	675	1,080	1,205	
			F2	—	435	650	725	435	650	725	435	650	725	
	5	Max.	F3	—	610	990	1,980	610	990	1,980	610	990	1,980	
			F4	2,135	215	345	490	440	705	800	725	1,160	1,465	
			F2	—	435	650	725	435	650	725	435	650	725	
	4	Outer	F3	—	420	680	1,360	420	680	1,360	420	680	1,360	
			F4	1,550	200	320	375	310	495	555	520	830	1,020	
			F ₂	—	435	650	725	435	650	725	435	650	725	
LSSC4.25	4	F	F3	—	250	410	815	250	410	815	250	410	815	
			F4	1,550	300	450	895	460	690	1,385	670	1,035	1,880	
			F2	—	435	650	725	435	650	725	435	650	725	
	8	G	F3	—	500	815	1,630	500	815	1,630	500	815	1,630	
			F4	1,550	495	740	1,480	670	1,035	1,880	670	1,035	1,880	
			F ₂	—	435	650	725	435	650	725	435	650	725	
	11	Н	F3	—	610	990	1,980	610	990	1,980	610	990	1,980	
			F4	1,550	545	820	1,640	670	1,035	1,880	670	1,035	1,880	
			F2	—	435	650	725	435	650	725	435	650	725	
	14	I	F3	—	610	990	1,980	610	990	1,980	610	990	1,980	
			F4	1,550	670	1,015	1,880	670	1,035	1,880	670	1,035	1,880	
			F2	—	330	495	990	510	765	1,105	660	995	1,105	
	2	Min.	F3	—	330	495	990	350	565	1,130	350	565	1,130	
			F4	2,450	355	570	635	525	840	990	890	1,425	1,590	
			F ₂	—	660	995	1,105	660	995	1,105	660	995	1,105	
	5	Max.	F3	_	825	1,240	2,475	845	1,375	2,750	845	1,375	2,750	
			F4	2,450	365	585	650	600	960	1,070	1,005	1,610	1,795	
			F ₂	—	660	990	1,105	660	995	1,105	660	995	1,105	
	4	Outer	F3	—	580	945	1,890	580	945	1,890	580	945	1,890	
			F4	1,930	235	375	430	330	530	650	625	1,000	1,225	
			F2		660	990	1,105	660	995	1,105	660	995	1,105	
SSC4.25	4	F	F3	—	350	565	1,130	350	565	1,130	350	565	1,130	
			F4	1,930	300	450	895	460	690	1,385	920	1,385	2,745	
			F2	—	660	995	1,105	660	995	1,105	660	995	1,105	
	8	G	F3	—	695	1,130	2,265	695	1,130	2,265	695	1,130	2,265	
	L		F4	1,930	495	740	1,480	765	1,145	2,290	980	1,510	2,745	
			F ₂		660	995	1,105	660	995	1,105	660	995	1,105	
	11	Н	F3	_	845	1,375	2,750	845	1,375	2,750	845	1,375	2,750	
			F4	1,930	545	820	1,640	845	1,270	2,535	980	1,510	2,745	
			F ₂	—	660	995	1,105	660	995	1,105	660	995	1,105	
	14	I	F ₃		845	1,375	2,750	845	1,375	2,750	845	1,375	2,750	
			F4	1,930	675	1,015	2,030	980	1,510	2,745	980	1,510	2,745	

See footnotes on p. 77.





Table 4: SSC Steel Stud Connectors (MSSC4.25 and LSSC6.25) - Service Load Limits and Strengths (lb.)

				a .				Stud Thick	ness and Yie	eld Strength			
Model No.	No. of #10	Screw Pattern	Load Direction	Service Load	33 m	il (20 ga.) –	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi
	Screws		2	Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F2	—	330	495	990	510	765	1,530	1,020	1,530	2,235
	2	Min.	F3	_	330	495	990	510	765	1,530	605	980	1,965
			F4	2,450	355	570	635	525	840	990	890	1,425	1,590
			F2	_	825	1,240	2,235	1,275	1,915	2,235	1,340	2,015	2,235
	5	Max.	F3	_	825	1,240	2,475	1,275	1,915	3,825	1,465	2,385	4,770
			F4	2,450	365	585	650	600	960	1,070	1,005	1,610	1,820
ĺ			F2	_	660	990	1,980	1,020	1,530	2,235	1,340	2,015	2,235
	4	Outer	F3	_	660	990	1,980	1,010	1,530	3,060	1,010	1,640	3,280
			F4	1,930	235	375	430	330	530	650	625	1,000	1,225
			F2	_	660	990	1,980	1,020	1,530	2,235	1,340	2,015	2,235
MSSC4.25	4	F	F3	_	605	980	1,965	605	980	1,965	605	980	1,965
			F4	1,930	300	450	895	460	690	1,385	920	1,385	2,765
			F2	_	1,320	1,980	2,235	1,340	2,015	2,235	1,340	2,015	2,235
	8	G	F3	—	1,210	1,965	3,925	1,210	1,965	3,925	1,210	1,965	3,925
			F4	1,930	495	740	1,480	765	1,145	2,290	1,525	2,290	4,280
			F2	_	1,340	2,015	2,235	1,340	2,015	2,235	1,340	2,015	2,235
	11	Н	F3	_	1,465	2,385	4,770	1,465	2,385	4,770	1,465	2,385	4,770
			F4	1,930	545	820	1,640	845	1,270	2,535	1,615	2,535	4,280
			F2		1,340	2,015	2,235	1,340	2,015	2,235	1,340	2,015	2,235
	14	I	F3	_	1,465	2,385	4,770	1,465	2,385	4,770	1,465	2,385	4,770
			F4	1,930	675	1,015	2,030	1,045	1,570	3,135	1,615	2,585	4,280
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
	4	Min.	F3	_	500	815	1,630	500	815	1,630	500	815	1,630
			F4	2,440	265	425	635	660	1,055	1,185	1,190	1,905	2,125
			F ₂	_	640	960	1,065	640	960	1,065	640	960	1,065
	7	Max.	F3	_	880	1,425	2,855	880	1,425	2,855	880	1,425	2,855
			F4	2,440	265	425	635	660	1,055	1,185	1,190	1,905	2,350
			F ₂	_	640	960	1,065	640	960	1,065	640	960	1,065
	6	Outer	F3	_	630	1,020	2,040	630	1,020	2,040	630	1,020	2,040
	-		F4	1,940	270	430	480	375	600	670	695	1,110	1,365
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
	4	J	F3	_	250	410	815	250	410	815	250	410	815
		-	F4	415	405	610	1,215	625	940	1,880	1,015	1,565	2,850
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
LSSC6.25	8	K	F3	_	500	815	1,630	500	815	1,630	500	815	1,630
	-		F4	1,940	730	1,100	2,195	1,015	1,565	2,850	1,015	1,565	2,850
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
	12	L	F3	_	750	1,225	2,445	750	1,225	2,445	750	1,225	2,445
		-	F4	1,940	975	1,465	2,850	1,015	1,565	2,850	1,015	1,565	2,850
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
	16	М	F3		895	1,455	2,910	895	1,455	2,910	895	1,455	2,910
	.0		F4	1,940	1,015	1,565	2,850	1,015	1,565	2,850	1,015	1,565	2,850
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
	19	Ν	F3		895	1,455	2,910	895	1,455	2,910	895	1,455	2,910
	.0	14	F4	1,940	1,015	1,565	2,850	1,015	1,565	2,850	1,015	1,565	2,850
			F ₂		640	960	1,065	640	960	1,065	640	960	1,065
	22	0	F3		895	1,455	2,910	895	1,455	2,910	895	1,455	2,910
	22	U	гз F4	1,940	1,015	1,455	2,910	1,015	1,455	2,910	1,015	1,455	2,910

Rigid Connectors

See footnotes on p. 77.

See footnotes on p. 77.

SSC Supplemental Information

Table 5: SSC Steel Stud Connectors (SSC6.25 and MSSC6.25) -Service Load Limits and Strengths (lb.)

	No. of		Screw Load	Service	Stud Thickness and Yield Strength									
Model No.	#10	Screw Pattern	Load Direction	Load	33 m	il (20 ga.) –	33 ksi	43 m	il (18 ga.) – :	33 ksi	54 m	il (16 ga.) –	50 ksi	
	Screws			Limit	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal	
			F2	_	660	990	1,625	975	1,460	1,625	975	1,460	1,625	
	4	Min.	F3	—	660	990	1,980	695	1,130	2,265	695	1,130	2,265	
			F4	2,440	385	615	685	720	1,150	1,380	1,190	1,905	2,270	
			F2	_	975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
	7	Max.	F3	—	1,155	1,735	3,465	1,220	1,980	3,965	1,220	1,980	3,965	
			F4	2,440	385	615	685	720	1,150	1,375	1,190	1,905	2,205	
			F ₂	—	975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
	6	Outer	F3	—	870	1,420	2,835	870	1,420	2,835	870	1,420	2,835	
			F4	1,940	270	430	480	460	735	820	725	1,160	1,330	
			F2	—	660	990	1,625	975	1,460	1,625	975	1,460	1,625	
	4	J	F3	—	350	565	1,130	350	565	1,130	350	565	1,130	
			F4	515	405	610	1,215	625	940	1,880	1,255	1,880	3,760	
			F2	_	975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
SSC6.25	8	К	F3	_	695	1,130	2,265	695	1,130	2,265	695	1,130	2,265	
			F4	1,940	730	1,100	2,195	1,130	1,700	3,395	1,485	2,290	4,165	
			F ₂	_	975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
	12	L	F3	_	1,045	1,700	3,395	1,045	1,700	3,395	1,045	1,700	3,395	
			F4	1,940	975	1,465	2,930	1,485	2,265	4,165	1,485	2,290	4,165	
			F2		975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
	16	M	F3	_	1,245	2,020	4,045	1,245	2,020	4,045	1,245	2,020	4,045	
			F4	1,940	1,140	1,710	3,425	1,485	2,290	4,165	1,485	2,290	4,165	
			F ₂		975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
	19	N	F3	_	1,245	2,020	4,045	1,245	2,020	4,045	1,245	2,020	4,045	
			F4	1,940	1,210	1,815	3,630	1,485	2,290	4,165	1,485	2,290	4,165	
			F ₂		975	1,460	1,625	975	1,460	1,625	975	1,460	1,625	
	22	0	F3	_	1,245	2,020	4,045	1,245	2,020	4,045	1,245	2,020	4,045	
		Ū	F4	1,940	1,350	2,020	4,045	1,485	2,290	4,165	1,485	2,290	4,165	
			F2		660	990	1,980	1,020	1,530	3,060	1,970	2,960	3,290	
	4	Min.	F3		660	990	1,980	1,020	1,530	3,060	1,210	1,965	3,925	
			F4	2,440	385	615	685	720	1,150	1,380	1,190	1,905	2,270	
			F ₂		1,155	1,735	3,290	1,785	2,680	3,290	1,970	2,960	3,290	
	7	Max.	F3		1,155	1,735	3,465	1,785	2,680	5,355	2,115	3,435	6,870	
		- marti	F4	2,440	385	615	685	720	1,150	1,375	1,365	2,185	2,490	
			F ₂		990	1,485	2,970	1,530	2,295	3,290	1,970	2,960	3,290	
	6	Outer	F3		990	1,485	2,970	1,515	2,295	4,590	1,515	2,460	4,915	
	Ū	- Cutor	F4	1,940	270	430	480	460	735	820	725	1,160	1,330	
			F ₂		660	990	1,980	1,020	1,530	3,060	1,970	2,960	3,290	
	4	J	F3		605	980	1,965	605	980	1,965	605	980	1,965	
			F4	735	405	610	1,215	625	940	1,880	1,255	1,880	3,760	
			F ₂		1,320	1,980	3,290	1,970	2,960	3,290	1,970	2,960	3,290	
MSSC6.25	8	к	F3		1,210	1,965	3,925	1,210	1,965	3,925	1,210	1,965	3,925	
1110000.20	Ū		F4	1,940	730	1,100	2,195	1,130	1,700	3,395	2,265	3,395	6,790	
			F2		1,970	2,960	3,290	1,970	2,960	3,290	1,970	2,960	3,290	
	12	L	F3		1,810	2,900	5,890	1,810	2,900	5,890	1,810	2,900	5,890	
	12		F4	1,940	975	1,465	2,930	1,510	2,265	4,525	2,590	4,145	6,865	
			F ₂		1,970	2,960	3,290	1,970	2,203	3,290	1,970	2,960	3,290	
	16	М	F3		2,160	3,505	7,010	2,160	3,505	7,010	2,160	3,505	7,010	
	10		F4	1,940	1,140	1,710	3,425	1,765	2,645	5,290	2,590	4,145	6,865	
			F2	1,940	1,140	2,960	3,290	1,703	2,045	3,290	1,970	2,960	3,290	
	19	N	F2 F3		2,160	3,505	7,010	2,160	3,505	7,010	2,160	3,505	7,010	
	13		F4	1,940	1,210	1,815	3,630	1,870	2,805	5,610	2,100	4,145	6,865	
													3,290	
	22		F ₂		1,970	2,960	3,290	1,970	2,960	3,290	1,970	2,960		
	22	0	F3	1.040	2,160	3,505 2,020	7,010 4,045	2,160	3,505	7,010	2,160	3,505	7,010 6,865	
			F4	1,940	1,350	2,020	4,040	2,085	3,125	6,250	2,590	4,145	0,000	

81



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

SJC connectors have been specifically designed for various CFS joist, rafter and underside of metal-deck applications. The unique clip dimensions enable easy installation on the open side of joists and rafters with up to 3½" flanges and return lips up to ¾". For metal-deck applications, the prepunched ¾" holes easily accommodate 6", 8", 10" and 12" on-center metal-deck flutes.

Features:

Rigid Connectors

- Prepunched holes reduce installation cost by eliminating predrilling
- Intuitive fastener hole positions ensure accurate clip installation in accordance with design, support a wide range of design and application requirements and provide installation flexibility
- Angle lengths accommodate either hard-side or soft-side attachment for joists with return lips up to ³/₄"
- 41/2" leg length enables soft-side connections for joists with flanges up to 31/2"
- Also accommodates kicker-to-metal-deck applications

Material: SJC - 68 mil (50 ksi); MSJC - 97 mil (50 ksi)

Finish: Galvanized (G90)

Installation:

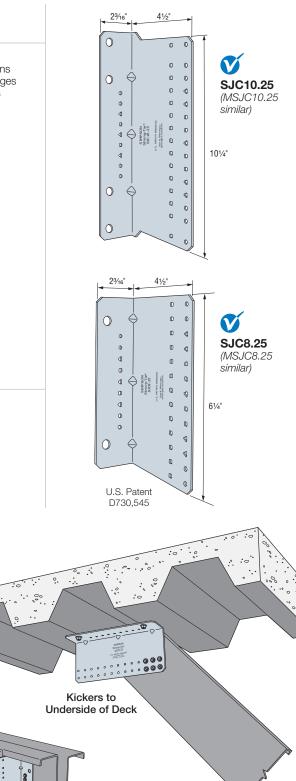
• Use all specified fasteners/anchors

Codes: See p. 11 for Code Reference Key Chart

For detailed product dimensions, refer to p. 91.

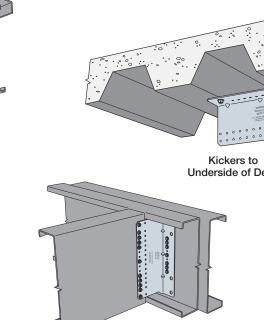
Ordering Information

Model No.	Ordering SKU	Package Quantity
SJC8.25	SJC8.25-R15	Box of 15
MSJC8.25	MSJC8.25-R15	DOX UL 13
SJC10.25	SJC10.25-R15	Box of 15
MSJC10.25	MSJC10.25-R15	DUX UI 13



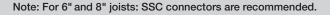
SIMPSON

Strong-I



Joists to I-Beam

Header to Jamb



Joist to Girder



SJC Connectors - Steel-to-Steel Allowable Loads

					Fasteners ⁵		Allov	vable F4 Load	(lb.) ²								
Model No.	Connector Material Thickness	L (in.)	Framing Member Depth⁴	Dottorni	Carried	Carrying		Member kness	Maximum	Code Ref.							
	mil (ga.)	()	(in.)	Pattern ¹	Member	Member	54 mil (16 ga.)	68 mil (14 ga.)	Connector Load ³								
				Min.	(4) #10	(4) #10	980	980									
SJC8.25	68 (14)	81⁄4	10	Max.	(9) #10	(7) #10	1,005	1,490	2,930								
				Inner	(5) #10	(4) #10	1,345	2,005		1							
				Min.	(4) #10	(4) #10	1,005	1,710	2,930								
MSJC8.25	97 (12)	81⁄4	10	Max.	(9) #10	(7) #10	1,135	1,765		2,930	2,930						
				Inner	(5) #10	(4) #10	1,535	2,220			IP2						
											Min.	(6) #10	(4) #10	1,170	1,625		IPZ
SJC10.25	68 (14)	10¼	12	Max.	(11) #10	(7) #10	1,265	1,625	3,935								
				Inner	(7) #10	(5) #10	1,620	2,170	-								
				Min.	(6) #10	(4) #10	1,200	2,045									
MSJC10.25	97 (12)	10¼	12	Max.	(11) #10	(7) #10	1,265	2,045	3,935	3,935	3,935						
	97 (12)	101⁄4		Inner	(7) #10	(5) #10	1,730	2,635									

1. Min. fastener quantity and load values - fill all round holes; Max. fastener quantity and load values - fill all

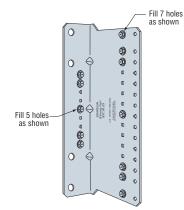
round and triangular holes; Inner fastener quantity and load values - see illustrations for fastener placement.

2. Allowable loads are based on bracing of the members located within 12" of the connection.

3. Maximum allowable load for connector that may not be exceeded when designing custom installations.

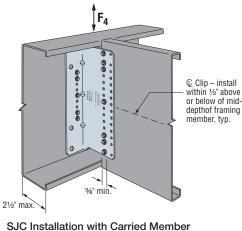
Designer is responsible for member and fastener design. 4. For 6" and 8" joists, SSC connectors are recommended.

5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

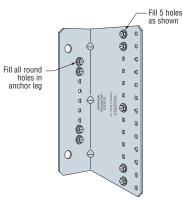




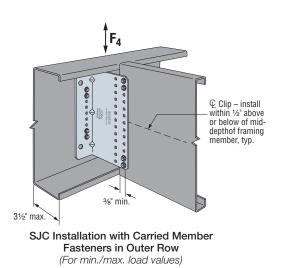
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Fasteners in Inner Row



SJC8.25-Inner Fastener Pattern (MSJC8.25 similar)





SJC Connectors - Kicker Allowable Loads

Model No.	Connector Material Thickness mil (ga.)	L (in.)	Fasteners to Kicker	Kicker Angle ²	Maximum Kicker Load (lb.)	Anchor Tension at Maximum Load (lb.)	Code Ref.
SJC8.25	68 (14)	81⁄4	(6) #10	30°	490	345	
5300.25	00 (14)	074	(0) #10	45°	535	570	
0.1010.05	CQ (14)	101/	(0) #10	30°	625	475	IP2
SJC10.25	68 (14)	10¼	(6) #10	45°	530	440	IPZ
MSJC10.25	5 97 (12) 1014		(6) #10	30°	950	675	
10.25	97 (12)	1074	(6) #10	45°	780	680	

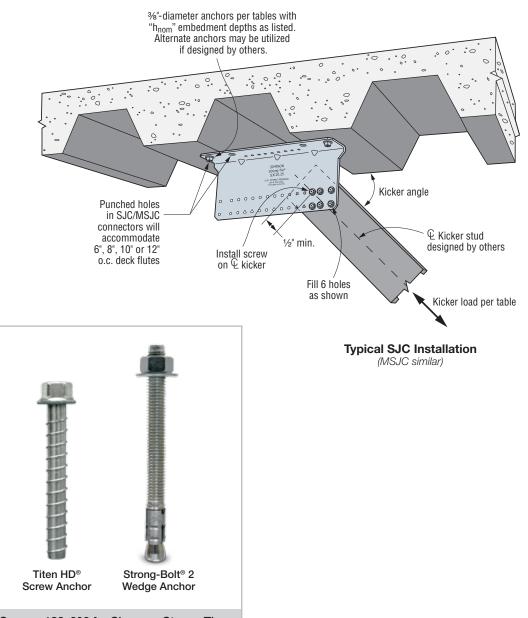
1. Loads apply to connectors installed perpendicular or parallel to metal-deck flutes, with minimum 33 mil (20 ga.) kicker.

2. Kicker angle is the acute angle measured relative to the horizontal plane of the metal deck.

3. The tabulated value for anchor tension is per anchor. Anchors must be designed for combined shear and tension.

Simpson Strong-Tie anchorage solutions are tabulated on pp. 85 and 86. Alternate anchors may be utilized if designed by others.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



See pp. 182–203 for Simpson Strong-Tie anchor solutions for this application.

SJC and MSJC Kicker Anchorage Solutions

		Uncracked Concrete, V	/ind and Seismic in SD	CA&B	
	I	Minimum 2.5" Slab (3,000 p	osi concrete min.) Over	Metal Deck	
Model No.	Kicker Angle	%" Diameter Simpson Strong-Tie® Anchor Type	Nominal Embedment Depth (h _{nom})	Anchor Tension at Max. Load	Reduced Max. Kicker Load (If Applicable)
	30°	STB2	21⁄2"	345	_
SJC8.25	50	Titen HD®	21⁄4"	345	_
5306.20	459	STB2	21⁄2"	570	
	45°	Titen HD	21⁄2"	570	
	30°	STB2	21⁄2"	475	
0.1010.05	30*	Titen HD	21⁄4"	475	
SJC10.25	45°	STB2	21⁄2"	440	
	45	Titen HD	21⁄4"	440	—
	20.9	STB2	2¾"	675	
MSJC10.25	30°	Titen HD	21⁄2"	575	805
	459	STB2	21⁄2"	680	_
	45°	Titen HD	21⁄2"	650	745

		Cracked Concrete, Wi	nd and Seismic in SDC	A&B	
	1	Minimum 2.5" Slab (3,000 p	osi concrete min.) Over	Metal Deck	
Model No.	Kicker Angle	⅔" Diameter Simpson Strong-Tie® Anchor Type	Nominal Embedment Depth (h _{nom})	Anchor Tension at Max. Load	Reduced Max. Kicker Load (If Applicable)
	30°	STB2	2¾"	345	—
SJC8.25	30	Titen HD	21⁄2"	320	455
5368.25	45°	STB2	23⁄4"	570	
	45	Titen HD	21⁄2"	340	320
	30°	STB2	2¾"	475	—
SJC10.25	30	Titen HD	21⁄2"	330	435
53610.25	45°	STB2	23⁄4"	440	
	45	Titen HD	21⁄2"	340	410
	20%	STB2	3%"	675	
MSJC10.25	30°	Titen HD	21⁄2"	325	460
	45°	STB2	2¾"	680	
	40	Titen HD	21⁄2"	340	390

1. Allowable loads have been determined using ACI 318-14 Chapter 17 anchorage calculations with the minimum concrete compressive strength, $f_{\rm C}$ and slab thickness listed.

2. STB2 = %" diameter Strong-Bolt® 2 carbon steel anchor, THD = %" diameter Titen HD® screw anchor.

3. Concrete over metal deck may be Normal Weight or Sand-Lightweight with f'_c of 3,000 psi minimum and 2.5" minimum slab height above upper flute.

 Minimum deck flute height is 1.5" (distance from top flute to bottom flute). All other anchor installation requirements shall follow ICC-ES ESR-3037 and ICC-ES ESR-2713.

5. Minimum Spacing, Edge and End distances for bottom of metal deck assemblies shall comply with those required in ICC-ES ESR-3037 for STB2 anchors and ICC-ES ESR-2713 for Titen HD anchors.

6. Load values are based on ACI 318-14, condition B, load factors from ACI 318-14 Section 5.3, no supplemental edge reinforcement for uncracked concrete, ψ_C , v = 1.0 for cracked concrete, and periodic special inspection. Reference ICC-ES ESR-3037 and ICC-ES ESR-2713 for further information.

7. Allowable Stress Design (ASD) values have been determined by multiplying Load Resistance Factor Design (LRFD) values by a conversion factor, Alpha (α), of 0.7 for seismic loads and 0.6 for wind loads. ASD values for other types or load combinations may be determined using alternate conversion factors.

SIMPSO

Strong

SJC and MSJC Kicker Anchorage Solutions

		Cr	acked Concrete, Se	eismic in SDC C thro	ugh F		
		Minimum	2.5" Slab (3,000 ps	i concrete min.) Ove	er Metal Deck		
		%" Diameter	Nominal	Ω =	= 1.5	Ω =	2.5
Model No.	Kicker Angle	Simpson Strong-Tie® Anchor Type	Embedment Depth (h _{nom})	Anchor Tension at Max. Load	Reduced Max. Kicker Load	Anchor Tension at Max. Load	Reduced Max. Kicker Load
	30°	STB2	3%"	345	—	310	435
SJC8.25		Titen HD®	21⁄2"	180	255	110	155
3300.23	5JC8.25 45°	STB2	3%"	570	—	355	330
		Titen HD	21⁄2"	200	185	120	110
	30°	STB2	3%"	475	—	320	420
SJC10.25	30*	Titen HD	21⁄2"	185	245	110	145
30010.20	45°	STB2	3%"	440	_	340	410
	45	Titen HD	21⁄2"	195	235	120	140
	30°	STB2	33⁄8"	525	740	315	445
MSJC10.25	30	Titen HD	21⁄2"	185	260	110	155
1/100010.25	45°	STB2	3%"	575	660	345	395
	40	Titen HD	21⁄2"	200	225	120	135

1. Allowable loads have been determined using ACI 318-14 Chapter 17 anchorage calculations with the minimum concrete compressive strength, f'_c and slab thickness listed.

2. STB2 = %"-diameter Strong-Bolt® 2 carbon steel anchor, THD = %"-diameter Titen HD® screw anchor.

3. Concrete over metal deck may be Normal Weight or Sand-Lightweight with f'c of 3,000 psi minimum and 2.5" minimum slab height above upper flute.

 Minimum deck flute height is 1.5" (distance from top flute to bottom flute). All other anchor installation requirements shall follow ICC-ES ESR-3037 and ICC-ES ESR-2713.

5. Minimum Spacing, Edge and End distances for bottom of metal deck assemblies shall comply with those required in ICC-ES ESR-3037 for STB2 anchors and ICC-ES ESR-2713 for Titen HD anchors.

6. Load values are based on ACI 318-14, condition B, load factors from ACI 318-14 Section 5.3, no supplemental edge reinforcement for uncracked concrete, ψ_C , v = 1.0 for cracked concrete, and periodic special inspection. Reference ICC-ES ESR-3037 and ICC-ES ESR-2713 for further information.

7. Allowable Stress Design (ASD) values have been determined by multiplying Load Resistance Factor Design (LRFD) values by a conversion factor, Alpha (α), of 0.7 for seismic loads and 0.6 for wind loads. ASD values for other types or load combinations may be determined using alternate conversion factors.

8. Allowable anchor loads have been divided by an Omega (Ω) seismic factor of 1.5 or 2.5 for brittle failure as required by ACI 318-14 Chapter 17.

Rigid Connectors

SJC and SSC Connectors — Soffit Stud Hanger Allowable Loads

	Connector			- ·	Allowable Tension Load	
Model No.	Thickness mil (ga.)	L (in.)	Anchors	Fasteners to Stud	No Bearing Plate	BP½-3 Bearing Plate
SJC8.25	68 (14)	81⁄4	(2) 3⁄8"	(4) #10	465	930
SJC10.25	68 (14)	101⁄4	(2) 3⁄8"	(4) #10	465	930
SSC4.25	68 (14)	41⁄4	(1) 3⁄8"	(4) #10	220	585

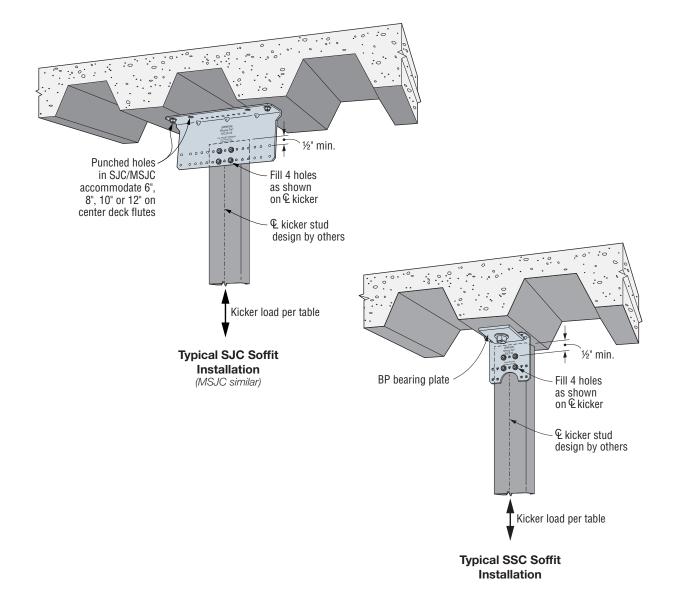
1. Loads apply to connectors installed perpendicular or parallel to metal-deck flutes.

2. Stud member design per Designer. Tabulated loads for stud fasteners are based on a minimum

stud thickness of 33 mil (20 ga.) with a yield stress of 33 ksi. For 30 mil interior studs with a yield strength of 33 ksi, multiply the tabulated values by 0.9.

3. Anchor design per Designer. Note that the SJC requires the symmetrical placement of one anchor on each side of the stud centerline.

4. For the bearing plate option, use Simpson Strong-Tie® BP½-3 bearing plates at each %"-diameter anchor. Bearing plates are sold separately.



SIMPSON

Strong-T

SFC Steel Framing Connectors



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

SFC connectors are a low-cost, multi-use utility clips for light to moderate loading conditions in CFS stud-to-stud and stud-to-structure applications where long leg lengths are not required.

Features:

- Reduced number of screws reduces installation cost
- Prepunched holes reduce installation cost by eliminating predrilling
- Intuitive fastener hole positions ensure accurate clip installation in accordance with design, support a wide range of design and application requirements and provide installation flexibility
- In soft-side stud installations, SFC will not interfere with stud lips up to ¾" long*
- Also suitable for U-channel bridging

Material: LSFC - 43 mil (33 ksi); SFC - 54 mil (50 ksi)

Finish: Galvanized (G90)

Installation:

• Use all specified fasteners/anchors.

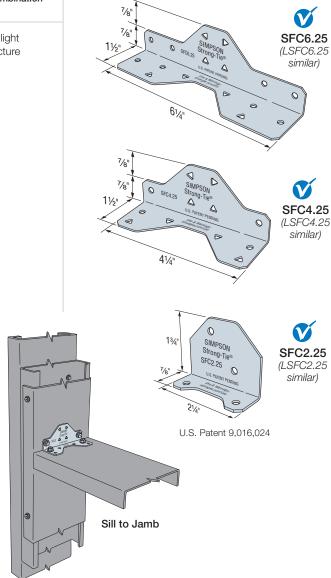
Codes: See p. 11 for Code Reference Key Chart

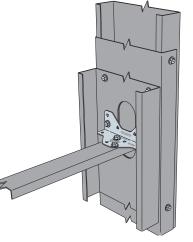
For detailed product dimensions, refer to p. 90.

Ordering Information

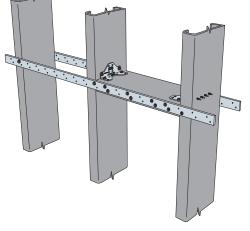
Model No.	Ordering SKU ¹	Package Quantity
SFC2.25	SFC2.25-R300	Bucket of 300
LSFC2.25	LSFC2.25-R300	DUCKEL OF SUU
SFC4.25	SFC4.25-R175	Bucket of 175
LSFC4.25	LSFC4.25-R175	DUCKEL OF 175
SFC6.25	SFC6.25-R100	Bucket of 100
LSFC6.25	LSFC6.25-R100	Bucket of 100

1. By leaving off the "-RXXX" suffix, items can also be ordered in cartons of 50.

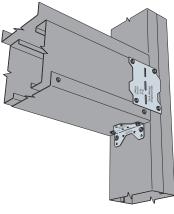




U-Channel to Jamb



Stud Blocking with CS Coiled Strap



Box Headers to Jambs with S/LS Angles

SIMPSON

Strong-

SFC Steel Framing Connectors



SFC Connectors — Steel-to-Steel Allowable Loads

	Connector		Framing		Fasteners			Allowable F	4 Load (lb.)		
Model	Material	L	Member		Carried	Carrying	Minimu	m Member Th	ickness	Maximum	Code
No.	Thickness mil (ga.)	(in.)	Depth (in.)	Pattern ¹	Member	Member	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	Connector Load ³	Ref.
LSFC2.25	43 (18)	21⁄4	35%8	Min.	(2) #10	(2) #10	295	310	475	630	
SFC2.25	54 (16)	2¼	3%	Min.	(2) #10	(2) #10	295	355	630	630	
LSFC4.25	43 (18)	41⁄4	6	Min.	(2) #10	(2) #10	355	525	525	1 750	
L3F64.20	43 (10)	4 /4	0	Max.	(6) #10	(6) #10	440	865	1,320	1,750	
SFC4.25	E4 (16)	41⁄4	6	Min.	(2) #10	(2) #10	355	525	745	1.750	
3664.20	54 (16)	4 /4	0	Max.	(6) #10	(6) #10	575	985	1,750	1,750	IP2
	40 (10)	01/	8	Min.	(4) #10	(4) #10	490	920	1,050	0.040	
LSFC6.25	43 (18)	6¼	ŏ	Max.	(8) #10	(8) #10	510	980	1,495	2,640	
	E4 (16)	6¼	8	Min.	(4) #10	(4) #10	590	1,035	1,840	2.640	
SFC6.25	54 (16)	0 /4	0	Max.	(8) #10	(8) #10	590	1,055	1,880	2,640	

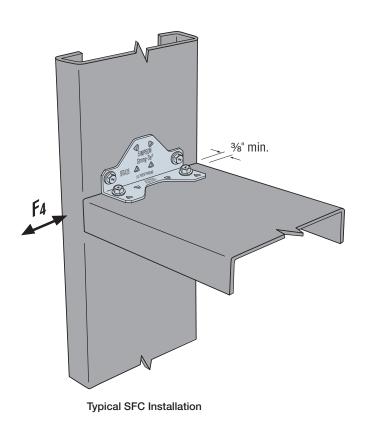
1. Min. fastener quantity and load values - fill all round holes; Max. fastener quantity and load values - fill all round and triangular holes.

2. Allowable loads are based on bracing of the members located within 12" of the connection.

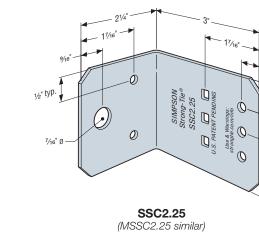
3. Maximum allowable load for connector that may not be exceeded when designing custom installations.

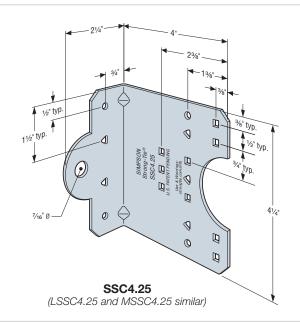
Designer is responsible for member and fastener design.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Utility Clip Dimensions





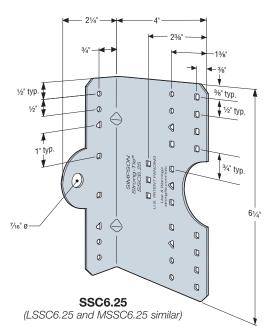
5⁄8"

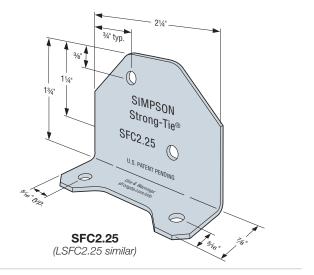
1⁄2'

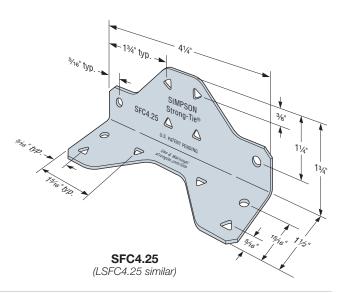
typ.

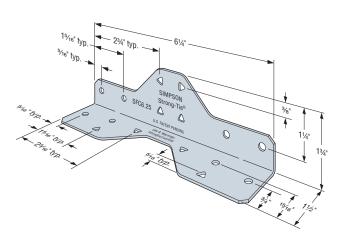
typ

21/21



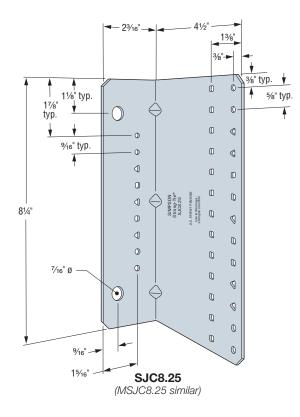


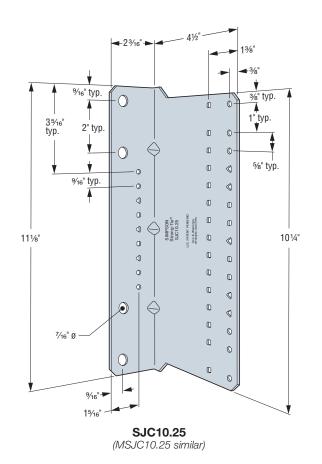




SFC6.25 (LSFC6.25 similar)

Utility Clip Dimensions





Rigid Connectors

The Simpson Strong-Tie® rigid connector angle is a generalpurpose clip angle designed for a wide range of cold-formed steel construction applications. With prepunched holes for fastener attachment, these L-shaped clips save time and labor on the job.

Features:

- Use with miscellaneous header/sill connections to jamb studs, jamb stud reinforcement at track, u-channel bridging, stud-blocking, bypass curtain-wall framing and more
- Easy to install, with prepunched holes for quick and accurate fastener attachment

Material: RCA223/54, RCA225/54, RCA227/54, RCA333/54, RCA335/54 — 54 mil (16 ga.), 50 ksi; RCA223/68, RCA225/68, RCA227/68, RCA333/68, RCA335/68 — 68 mil (14 ga.), 50 ksi; RCA223/97, RCA225/97, RCA227/97, RCA333/97, RCA335/97 — 97 mil (12 ga.), 50 ksi

Finish: Galvanized (G90)

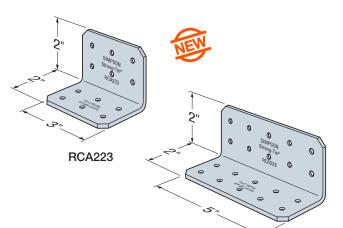
Installation:

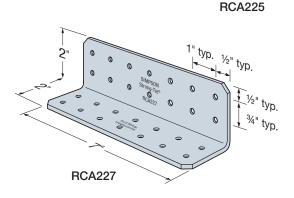
• Use all specified anchors/fasteners

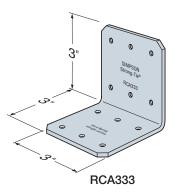
Codes: See p. 11 for Code Reference Key Chart

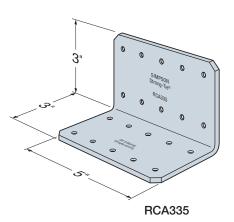
Ordering Information

Model No.	Ordering SKU	Bucket Quantity
RCA223/54	RCA223/54-R150	150
RCA223/68	RCA223/68-R125	125
RCA223/97	RCA223/97-R90	90
RCA225/54	RCA225/54-R90	90
RCA225/68	RCA225/68-R75	75
RCA225/97	RCA225/97-R55	55
RCA227/54	RCA227/54-R65	65
RCA227/68	RCA227/68-R55	55
RCA227/97	RCA227/97-R40	40
RCA333/54	RCA333/54-R100	100
RCA333/68	RCA333/68-R85	85
RCA333/97	RCA333/97-R60	60
RCA335/54	RCA335/54-R60	60
RCA335/68	RCA335/68-R50	50
RCA335/97	RCA335/97-R35	35



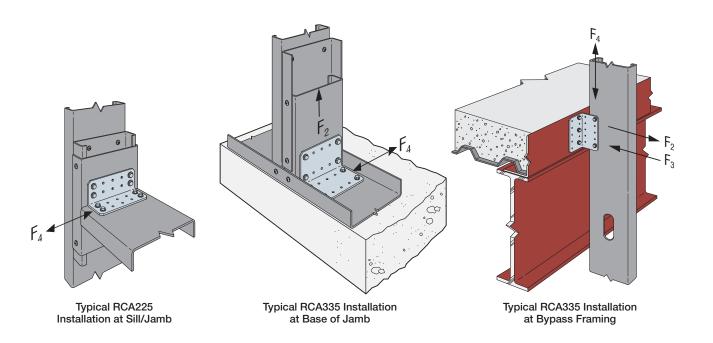






Connectors for Cold-Formed Steel Construction

RCA Rigid Connector Angles



Screw Patterns for Rigid Connector Angles

Models	Pattern 1	Pattern 2	Pattern 3		
RCA223/54 RCA223/68 RCA223/97 RCA333/54 RCA333/68 RCA333/97					
Models	Pattern 4	Pattern 5	Pattern 6	Pattern 7	Pattern 8
RCA225/54 RCA225/68 RCA225/97 RCA335/54 RCA335/68 RCA335/97					
Models	Pattern 9	Pattern 10	Pattern 11	Pattern 12	Pattern 13
RCA227/54 RCA227/68 RCA227/97					

Footnotes for RCA Load Tables

- 1. As applicable, the tabulated values are calculated based on AISI RP15-2, AISI S100 or generally accepted industry standards.
- 2. The tabulated values do not account for anchorage to the support. Anchor strength must be calculated separately and may reduce the capacity of the connection when compared to the tabulated values.
- 3. Tabulated values do not include shear, web crippling, buckling or other local effects in the member. The Designer must check member limit states separately.
- 4. For load combinations that include F_4 and/or F_2 and/or F_3 , use an appropriate interaction equation.
- #10–16 screws shall have P_{SS} ≥ 1,620 lb. Calculated values are per AISI S100. Screws must be installed with three (minimum) exposed threads.
- 6. The number of screws is for one clip leg that is attached to the supported stud.
- 7. In addition to calculations of net and gross section tension, F₂ values are also calculated and normally controlled by weak-axis bending of the anchored clip leg with the line of bending at the holes nearest the bend radius of the angle. The Designer is responsible for calculating pullover, pullout and tension strength of the anchors and this may reduce F₂ strength compared to the tabulated values.
- 8. F3 strength values are computed using the plate buckling provisions of AISI RP15-2.
- 9. For the F4 strength values it's assumed that all of the connection eccentricity is taken by the screws in the supported stud. F4 values are also limited by plate shear buckling per AISI RP15-2. The Designer is responsible for calculating the shear capacity of the anchorage, which may reduce F4 strength compared to the tabulated values.
- 10. In addition to the limit states given in notes 7, 8 and 9, F₂, F₃ and F₄ are also limited by screw shear according to the thinnest connected part of the connector and stud.
- 11. Service load limits for F₂ and F₃ are not given since there are no generally accepted industry methods available to compute these values. F₄ service load limits are based on AISI Research Report RP15-2, using a safety factor of three for 1/8" deflection.
- 12. For 50 ksi studs, 68 mil (14 ga.) and thicker, use the tabulated values for 54 mil (16 ga.) 50 ksi studs.

RCA Rigid Connector Angles (2"x2"x3" and 2"x2"x5") - Service Load Limits and Strengths (lb.)

	No. of			. .				Stud Thick	ness and Yie	eld Strength			
Model	No. of #10 Screws ^{5,6}	Screw Pattern	Load Direction	Service Load Limit ¹¹	33 mi	il (20 ga.) –	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi
	3016005-,-			LIIIII	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal
			F2	_	205	305	340	205	305	340	205	305	340
	3	1	F3	_	495	745	1,485	590	955	1,910	590	955	1,910
			F4	350	200	300	605	310	465	930	620	930	1,865
			F2	_	205	305	340	205	305	340	205	305	340
RCA223/54	4	2	F3	_	580	940	1,880	580	940	1,880	580	940	1,880
			F4	700	390	585	1,175	605	905	1,815	1,095	1,685	3,065
			F ₂		205	305	340	205	305	340	205	305	340
	6	3	F3	_	865	1,410	2,820	865	1,410	2,820	865	1,410	2,820
			F4	700	480	720	1,440	740	1,110	2,225	1,095	1,685	3,065
			F ₂		310	470	520	310	470	520	310	470	520
	3	1	F3		495	745	1,485	765	1,150	2,295	815	1,330	2,655
			F4	435	200	300	605	310	465	930	620	930	1,865
			F ₂		310	470	520	310	470	520	310	470	520
RCA223/68	4	2	F3		660	990	1,980	805	1,305	2,610	805	1,305	2,610
			F4	865	390	585	1,175	605	905	1,815	1,210	1,815	3,625
			F ₂		310	470	520	310	470	520	310	470	520
	6	3	F3		990	1,485	2,970	1,205	1,960	3,915	1,205	1,960	3,915
		-	F4	865	480	720	1,440	740	1,110	2,225	1,350	2,080	3,785
			F ₂		495	745	1,055	630	945	1,055	630	945	1,055
	3	1	F3		495	745	1,485	765	1,150	2,295	1,415	2,295	4,590
			F4	615	200	300	605	310	465	930	620	930	1,865
			F ₂		630	945	1,055	630	945	1,055	630	945	1,055
RCA223/97	4	2	F3		660	990	1,980	1,020	1,530	3,060	1,265	2,050	4,105
			F4	1,230	390	585	1,175	605	905	1,815	1,210	1,815	3,625
			F ₂		630	945	1,055	630	945	1,055	630	945	1,055
	6	3	F3		990	1,485	2,970	1,530	2,295	4,590	1,895	3,080	6,155
			F4	1,230	480	720	1,440	740	1,110	2,225	1,485	2,225	4,450
			F ₂		330	495	570	340	510	570	340	510	570
	2	4	F3		330	495	990	390	635	1,275	390	635	1,275
			F4	585	265	395	790	410	610	1,225	815	1,225	2,450
			F ₂		340	510	570	340	510	570	340	510	570
	4	5	F3		580	940	1,880	580	940	1,880	580	940	1,880
			F4	1,170	535	805	1,610	830	1,245	2,490	1,660	2,490	4,980
			F ₂		340	510	570	340	510	570	340	510	570
RCA225/54	5	6	F3		825	1,240	2,475	980	1,595	3,185	980	1,595	3,185
	5		F4	585	460	685	1,375	705	1,060	2,120	1,310	2,015	3,665
			F2		340	510	570	340	510	570	340	510	570
	8	7	F3		1,155	1,880	3,760	1,155	1,880	3,760	1,155	1,880	3,760
	5	ĺ ĺ	F4	1,170	915	1,375	2,750	1,420	2,125	4,255	1,825	2,810	5,110
			F2		340	510	570	340	510	570	340	510	570
	10	8	F3		1,445	2,350	4,695	1,445	2,350	4,695	1,445	2,350	4,695
	10	0	F4	1,170	1,035	1,555	3,105	1,600		4,800	1,825	2,810	5,110
			14	1,170	1,055	1,000	3,105	1,000	2,400	4,000	1,020	2,010	5,110

See footnotes on p. 94.

SIMPSON

Strong-Tie



RCA Rigid Connector Angles (2"x2"x5" and 2"x2"x7") - Service Load Limits and Strengths (lb.)

	No. of			Service				Stud Thick	ness and Yie	eld Strength			
Model	#10	Screw Pattern	Load Direction	Load	33 m	il (20 ga.) –	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi
	Screws ^{5,6}	rattern	Direction	Limit ¹¹	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nomina
			F2	—	330	495	865	510	765	865	520	780	865
	2	4	F3	—	330	495	990	510	765	1,530	545	885	1,770
			F4	720	265	395	790	410	610	1,225	815	1,225	2,450
			F ₂	—	520	780	865	520	780	865	520	780	865
	4	5	F3		660	990	1,980	805	1,305	2,610	805	1,305	2,610
			F4	1,440	535	805	1,610	830	1,245	2,490	1,660	2,490	4,980
			F ₂		520	780	865	520	780	865	520	780	865
RCA225/68	5	6	F3		825	1,240	2,475	1,275	1,915	3,825	1,360	2,215	4,425
			F4	720	460	685	1,375	705	1,060	2,120	1,415	2,120	4,245
		7	F2		520	780	865	520	780	865	520	780	865
	8	7	F3		1,320	1,980	3,960	1,605	2,610	5,220	1,605	2,610	5,220
			F4	1,440	915	1,375	2,750	1,420	2,125	4,255	2,255	3,470	6,310
	10	0	F2		520	780	865	520	780	865	520	780	865
	10	8	F3	1 4 4 0	1,650	2,475	4,950	2,010	3,265	6,525	2,010	3,265	6,525
			F4 F2	1,440	1,035 330	1,555 495	3,105 990	1,600 510	2,400 765	4,800 1,530	2,255 1,020	3,470 1,530	6,310 1,755
	2	4	F2 F3		330	495	990	510	765	1,530	945	1,530	3,060
	2	4	F3 F4	1,025	265	395	790	410	610	1,225	945 815	1,225	2,450
			F2		660	990	1,755	1,020	1,530	1,755	1,050	1,225	1,755
	4	5	F3		660	990	1,755	1,020	1,530	3,060	1,050	2,050	4,105
RCA225/97	-	0	F4	2,050	535	805	1,610	830	1,245	2,490	1,660	2,490	4,980
			F ₂		825	1,240	1,755	1,050	1,580	1,755	1,050	1,580	1,755
	5	5 6	F3		825	1,240	2,475	1,275	1,915	3,825	2,360	3,825	7,650
	Ŭ	Ū	F4	1,025	460	685	1,375	705	1,060	2,120	1,415	2,120	4,245
			F ₂		1,050	1,580	1,755	1,050	1,580	1,755	1,050	1,580	1,755
	8	7	F3		1,320	1,980	3,960	2,040	3,060	6,120	2,525	4,105	8,210
	-		F4	2,050	915	1,375	2,750	1,420	2,125	4,255	2,835	4,255	8,505
			F2		1,050	1,580	1,755	1,050	1,580	1,755	1,050	1,580	1,755
	10	8	F3	—	1,650	2,475	4,950	2,550	3,825	7,650	3,155	5,130	10260
			F4	2,050	1,035	1,555	3,105	1,600	2,400	4,800	3,200	4,800	8,980
	4 9		F2	—	475	715	795	475	715	795	475	715	795
		9	F3	—	660	990	1,980	785	1,275	2,550	785	1,275	2,550
			F4	820	545	815	1,630	840	1,260	2,515	1,675	2,515	5,030
			F ₂	—	475	715	795	475	715	795	475	715	795
	4	10	F3	_	580	940	1,880	580	940	1,880	580	940	1,880
			F4	1,635	595	890	1,785	920	1,380	2,760	1,840	2,760	5,515
			F2	—	475	715	795	475	715	795	475	715	795
RCA227/54	7	11	F3	_	1,155	1,735	3,465	1,280	2,080	4,160	1,280	2,080	4,160
			F4	720	765	1,150	2,300	1,185	1,780	3,560	1,685	2,595	4,720
			F2	_	475	715	795	475	715	795	475	715	795
	8	12	F3		1,155	1,880	3,760	1,155	1,880	3,760	1,155	1,880	3,760
			F4	1,635	1,120	1,680	3,360	1,730	2,595	5,190	2,555	3,935	7,155
		10	F ₂		475	715	795	475	715	795	475	715	795
	14	13	F3	1.005	2,025	3,290	6,575	2,025	3,290	6,575	2,025	3,290	6,575
			F4	1,635	1,685	2,530	5,055	2,555	3,905	7,155	2,555	3,935	7,155
	4	0	F ₂		660	990	1,215	725	1,090	1,215	725	1,090	1,215
	4	9	F3	1.010	660 545	990 815	1,980	1,020 840	1,530	3,060 2,515	1,090	1,770 2,515	3,540
			F4	1,010			1,630		1,260		1,675		5,030
	Δ	10	F ₂	—	660	990	1,215	725	1,090	1,215	725	1,090	1,215
	4	10	F3 F4	2 020	660 595	990 890	1,980 1,785	805 920	1,305 1,380	2,610 2,760	805 1,840	1,305 2,760	2,610
			F4 F2	2,020	725	1,090	1,785	920 725	1,380	1,215	725	1,090	5,515 1,215
RCA227/68	7	11	F2 F3		1,155	1,090	3,465	1,780	2,680	5,355	1,780	2,890	5,775
NGAZ21/00	/	11	F3 F4	890	765	1,735	2,300	1,780	1,780	3,560	2,370	3,560	6,900
			F4 F2		705	1,090	1,215	725	1,780	1,215	725	1,090	1,215
	8	12	F2 F3		1,320	1,090	3,960	1,605	2,610	5,220	1,605	2,610	5,220
	0	12	F3 F4	2,020	1,320	1,980	3,360	1,805	2,595	5,220	3,155	4,860	8,830
			F4 F2	2,020	725	1,080	1,215	725	1,090	1,215	725	1,090	1,215
	14 13	13	F2 F3		2,310	3,465	6,930	2,810	4,570	9,135	2,810	4,570	9,135
					2.010	0.400	0,300	2.010	4.370	0.100	6.010	4,1/11	9.100

See footnotes on p. 94.

RCA Rigid Connector Angles (2"x2"x7" and 3"x3"x3") - Service Load Limits and Strengths (lb.)

	No. of			Osmiss				Stud Thick	ness and Yi	eld Strength				
Model	No. of #10	Screw Pattern	Load Direction	Service Load	33 mi	l (20 ga.) –	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	il (16 ga.) –	50 ksi	
	Screws ^{5,6}		2	Limit ¹¹	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal	
			F2	—	660	990	1,980	1,020	1,530	2,455	1,470	2,210	2,455	
	4	9	F3		660	990	1,980	1,020	1,530	3,060	1,890	3,060	6,120	
			F4	1,435	545	815	1,630	840	1,260	2,515	1,675	2,515	5,030	
			F2	—	660	990	1,980	1,020	1,530	2,455	1,470	2,210	2,455	
	4	10	F3	_	660	990	1,980	1,020	1,530	3,060	1,265	2,050	4,105	
			F4	2,875	595	890	1,785	920	1,380	2,760	1,840	2,760	5,515	
			F2	—	1,155	1,735	2,455	1,470	2,210	2,455	1,470	2,210	2,455	
RCA227/97	7	11	F3	—	1,155	1,735	3,465	1,785	2,680	5,355	3,080	5,010	10015	
			F4	1,265	765	1,150	2,300	1,185	1,780	3,560	2,370	3,560	7,115	
		12	F2	—	1,320	1,980	2,455	1,470	2,210	2,455	1,470	2,210	2,455	
	8		F3	—	1,320	1,980	3,960	2,040	3,060	6,120	2,525	4,105	8,210	
			F4	2,875	1,120	1,680	3,360	1,730	2,595	5,190	3,460	5,190	10385	
			F2	—	1,470	2,210	2,455	1,470	2,210	2,455	1,470	2,210	2,455	
	14	13	F3	—	2,310	3,465	6,930	3,570	5,355	10710	4,420	7,180	14365	
			F4	2,875	1,685	2,530	5,055	2,605	3,905	7,815	4,490	6,915	12570	
			F2	—	205	305	340	205	305	340	205	305	340	
	3	1	F3	—	440	720	1,435	440	720	1,435	440	720	1,435	
			F4	210	130	190	385	195	295	590	395	590	1,185	
			F2	_	205	305	340	205	305	340	205	305	340	
RCA333/54	4	4 2	F3	—	580	940	1,880	580	940	1,880	580	940	1,880	
			F4	700	325	490	975	505	755	1,510	1,005	1,510	3,020	
				F2	—	205	305	340	205	305	340	205	305	340
	6	3	F3	—	865	1,410	2,820	865	1,410	2,820	865	1,410	2,820	
			F4	700	430	650	1,295	665	1,000	2,000	1,095	1,685	3,065	
			F ₂	—	310	470	520	310	470	520	310	470	520	
	3	1	F3	—	495	745	1,485	615	1,000	1,995	615	1,000	1,995	
			F4	260	130	190	385	195	295	590	395	590	1,185	
			F2	—	310	470	520	310	470	520	310	470	520	
RCA333/68	4	2	F3	—	660	990	1,980	805	1,305	2,610	805	1,305	2,610	
			F4	865	325	490	975	505	755	1,510	1,005	1,510	3,020	
			F ₂	—	310	470	520	310	470	520	310	470	520	
	6	3	F3	—	990	1,485	2,970	1,205	1,960	3,915	1,205	1,960	3,915	
			F4	865	430	650	1,295	665	1,000	2,000	1,335	2,000	3,785	
			F ₂	—	495	745	1,055	630	945	1,055	630	945	1,055	
	3	1	F3	—	495	745	1,485	765	1,150	2,295	1,065	1,730	3,460	
			F4	370	130	190	385	195	295	590	395	590	1,185	
			F ₂	—	630	945	1,055	630	945	1,055	630	945	1,055	
RCA333/97	4	2	F3	—	660	990	1,980	1,020	1,530	3,060	1,265	2,050	4,105	
			F4	1,230	325	490	975	505	755	1,510	1,005	1,510	3,020	
			F ₂	—	630	945	1,055	630	945	1,055	630	945	1,055	
	6	3	F3		990	1,485	2,970	1,530	2,295	4,590	1,895	3,080	6,155	
			F4	1,230	430	650	1,295	665	1,000	2,000	1,335	2,000	4,005	

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Strong-Tie

See footnotes on p. 94.

SIMPSON Strong-Tie

RCA Rigid Connector Angles

RCA Rigid Connector Angles (3"x3"x5") – Service Load Limits and Strengths (lb.)

	No. of			Corvioo	stud Thickness and Yield Strength									
Model	No. of #10	Screw Pattern	Load Direction	Load	33 m	il (20 ga.) –	33 ksi	43 m	il (18 ga.) –	33 ksi	54 m	54 mil (16 ga.) – 50 ksi		
	Screws ^{5,6}	rattorn	Direction	Limit ¹¹	ASD	LRFD	Nominal	ASD	LRFD	Nominal	ASD	LRFD	Nominal	
			F2	—	330	495	570	340	510	570	340	510	570	
	2	4	F ₃	—	295	480	955	295	480	955	295	480	955	
			F4	350	205	310	620	320	480	955	635	955	1,910	
			F ₂	—	340	510	570	340	510	570	340	510	570	
	4	5	F3	—	580	940	1,880	580	940	1,880	580	940	1,880	
			F4	1,170	450	675	1,350	695	1,045	2,085	1,390	2,085	4,175	
			F ₂	—	340	510	570	340	510	570	340	510	570	
RCA335/54	5	6	F3	—	735	1,195	2,395	735	1,195	2,395	735	1,195	2,395	
			F4	350	305	460	920	475	710	1,420	835	1,285	2,335	
			F ₂	—	340	510	570	340	510	570	340	510	570	
	8	7	F3	—	1,155	1,880	3,760	1,155	1,880	3,760	1,155	1,880	3,760	
			F4	1,170	755	1,135	2,265	1,170	1,750	3,505	1,825	2,810	5,110	
			F ₂	—	340	510	570	340	510	570	340	510	570	
10	10	8	F3	—	1,445	2,350	4,695	1,445	2,350	4,695	1,445	2,350	4,695	
			F4	1,170	860	1,290	2,585	1,330	1,995	3,990	1,825	2,810	5,110	
			F2	—	330	495	865	510	765	865	520	780	865	
	2	4	F ₃	—	330	495	990	410	665	1,330	410	665	1,330	
			F4	435	205	310	620	320	480	955	635	955	1,910	
	4		F ₂	—	520	780	865	520	780	865	520	780	865	
		5	F ₃	—	660	990	1,980	805	1,305	2,610	805	1,305	2,610	
			F4	1,440	450	675	1,350	695	1,045	2,085	1,390	2,085	4,175	
	5		F ₂	—	520	780	865	520	780	865	520	780	865	
RCA335/68		6	F ₃	—	825	1,240	2,475	1,025	1,665	3,325	1,025	1,665	3,325	
			F4	435	305	460	920	475	710	1420	945	1,420	2,840	
			F ₂	—	520	780	865	520	780	865	520	780	865	
	8	7	F3	—	1,320	1,980	3,960	1,605	2,610	5,220	1,605	2,610	5,220	
			F4	1,440	755	1,135	2,265	1,170	1,750	3,505	2,255	3,470	6,310	
			F ₂	—	520	780	865	520	780	865	520	780	865	
	10	8	F3	—	1,650	2,475	4,950	2,010	3,265	6,525	2,010	3,265	6,525	
			F4	1,440	860	1,290	2,585	1,330	1,995	3,990	2,255	3,470	6,310	
			F2	_	330	495	990	510	765	1,530	1,020	1,530	1,755	
	2	4	F3	_	330	495	990	510	765	1,530	710	1,155	2,305	
			F4	615	205	310	620	320	480	955	635	955	1,910	
			F ₂		660	990	1,755	1,020	1,530	1,755	1,050	1,580	1,755	
	4	5	F3		660	990	1,980	1,020	1,530	3,060	1,265	2,050	4,105	
			F4	2,050	450	675	1,350	695	1,045	2,085	1,390	2,085	4,175	
			F ₂	_	825	1,240	1,755	1,050	1,580	1,755	1,050	1,580	1,755	
RCA335/97	5	6	F3	_	825	1,240	2,475	1,275	1,915	3,825	1,775	2,885	5,765	
			F4	615	305	460	920	475	710	1,420	945	1,420	2,840	
			F ₂	_	1,050	1,580	1,755	1,050	1,580	1,755	1,050	1,580	1,755	
	8	7	F3	_	1,320	1,980	3,960	2,040	3,060	6,120	2,525	4,105	8,210	
			F4	2,050	755	1,135	2,265	1,170	1,750	3,505	2,335	3,505	7,010	
			F ₂	_	1,050	1,580	1,755	1,050	1,580	1,755	1,050	1,580	1,755	
	10	8	F3	_	1,650	2,475	4,950	2,550	3,825	7,650	3,155	5,130	10,260	
			F4	2,050	860	1,290	2,585	1,330	1,995	3,990	2,660	3,990	7,985	

See footnotes on p. 94.

Connectors for Cold-Formed Steel Construction

L, LS and S/LS Utility Clips and Skewable Angles

L, LS and S/LS angles are load rated and provide the correct thickness and number of fasteners the specifier is looking for compared with field fabricated clip angles. These angles also have well-defined fastener locations, and testing ensures that the tabulated load values account for connection eccentricities. The connectors are general utility reinforcing angles with multiple uses. LS and S/LS connectors are skewable and can be used to attach members intersecting at angles.

Material: L - 54 mil (16 ga.); LS - 43 mil (18 ga.); S/LS - 43 mil (18 ga.)

Finish: Galvanized (G90)

Installation:

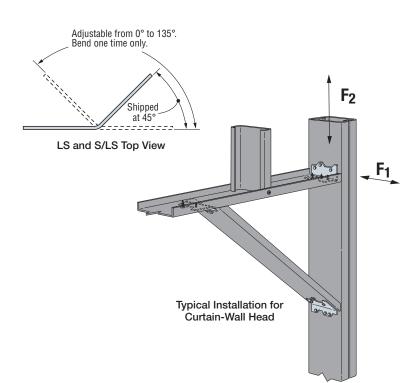
- Use all specified fasteners
- S/LS field-skewable; bend one time only
- CFS framing must be constrained against rotation when using a single S/LS per connection

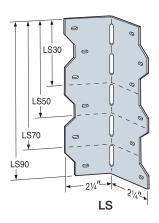
Codes: See p. 11 for Code Reference Key Chart

			Allowable Load (lb.)						
Model No.	Length (in.)	Fasteners	33 mil	(20 ga.)	43 mil	(18 ga.)	54 mil	(16 ga.)	Code Ref.
			F ₁	F ₂	F1	F ₂	F1	F ₂	
L30	3	(4) #10	200	60	315	85	610	—	
L50	5	(6) #10	475	—	675	90	750	110	
L70	7	(8) #10	705	_	760	110	1,100	110	
L90	9	(10) #10	795	_	945	110	1,740	110	100
LS30	3%	(6) #10	200	_	370		500	_	160
S/LS50	41⁄8	(4) #10	200	_	370	—	500	_	
S/LS70	6%	(6) #10	465	_	575	_	715	_	
LS90	71⁄8	(12) #10	465	_	895		915	_	

1. Loads are for one part only.

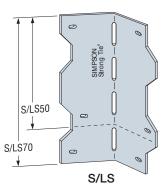
2. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

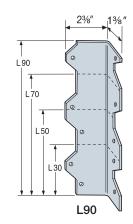


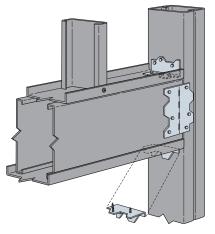


SIMPSON

Strong-







Typical Installation for Gravity Headers



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The Simpson Strong-Tie[®] RCKW rigid connectors have been developed to resist overturning moment at the base of exterior kneewalls and parapets as well as interior partial-height walls. These connectors offer a unique anchor-hole pattern that permits anchorage to both concrete and structural steel, with the larger hole designed to accommodate ½"-diameter concrete screws such as the Simpson Strong-Tie Titen HD[®]. The RCKWS is a heavy 171 mil (7 ga.) stiffener that nests onto the RCKW clip. The screw holes and anchor holes in the stiffener line up with those in the RCKW clip, making fastener and anchor installation a snap. The RCKW clip and RCKWS stiffener are sold separately.

Features:

Rigid Connectors

- Anchorage legs incorporate stiffened flanges, improving overturning moment resistance
- Large-diameter anchor hole accommodates ½"-diameter concrete screws and wedge anchors, such as the Simpson Strong-Tie Titen HD heavy-duty screw and the Strong-Bolt® 2 wedge anchor
- Additional smaller-diameter anchor holes enable attachment to structural steel through use of #12 self-drilling screws
- Attachment to CMU can be achieved with use of Titen[®] concrete and masonry screws
- For the RCKWS: 171 mil (7 ga.) stiffeners are secured to the RCKW clip with screws, optimizing overturning moment resistance and stiffness

Material: RCKW and RCKWS - 171 mil (7 ga.), 33 ksi

Coating: Galvanized (G90)

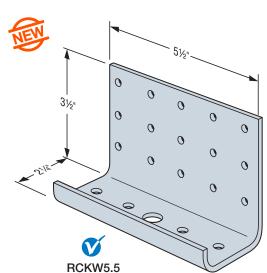
Installation:

- Use all specified screw fasteners. To achieve tabulated load values, use #12–14 screws according to the screw patterns below.
- When using the RCKWS, secure the stiffener to the clip with the specified screw fasteners. Screws must be at least 1" long and extend through the connection with a minimum of three exposed threads.
- Use all specified anchors. To achieve tabulated stiffness values, the installation torque for ½"-diameter anchors shall be at least 17 ft.-lb.
- When using the ½"-diameter Simpson Strong-Tie Titen HD[®] anchor, the bottom track must be pre-drilled or punched with a ¾"-diameter hole.

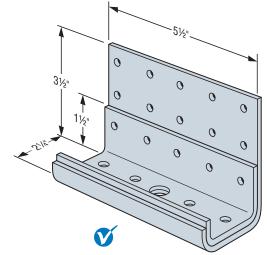
Codes: See p. 11 for Code Reference Key Chart

Ordering Information

-						
Model No.	Ordering SKU	Package Quantity				
RCKW3	RCKW3-R10	10 RCKW3 clips				
RCKW5.5	RCKW5.5-R10	10 RCKW5.5 clips				
RCKW7.5	RCKW7.5-R10	10 RCKW7.5 clips				
RCKW3S	RCKW3S-R10	10 RCKW3S stiffeners				
RCKW5.5S	RCKW5.5S-R10	10 RCKW5.5S stiffeners				



(RCKW3 and RCKW7.5 models also available)



RCKW5.5 shown with RCKW5.5S stiffener (RCKW5.5S can also be used with the RCKW7.5; RCKW3S can be used with RCKW3) U.S. Patent Pending

Ease of Specification

Many cold-formed steel connector manufacturers provide limited technical data for their products. As a result, Designers often rely on detailed and time-consuming hand calculations for CFS connection design. This often involves assumptions regarding connection eccentricity, prying and connection stiffness.

Simpson Strong-Tie strives for ease of specification by providing comprehensive load tables based on tests that simulate real-world conditions. These load tables ensure that tabulated values reflect not only the strength of the connector, but also the strength of the fasteners, the anchorage, the member near the connection, and the overall stiffness. The photo to the right is an example of member failure near the connection. Such failures are reflected in our tabulated loads because of our assembly testing.

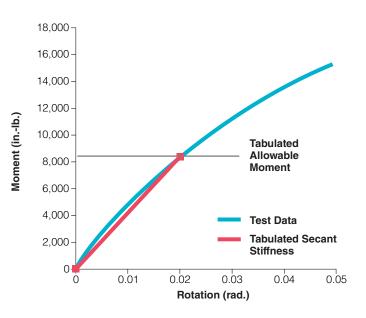


RCKW Assembly Test with Member Failure

Simplified Stiffness Calculations

Some manufacturers tabulate stiffness values only for the connector. It's often unknown or unclear if their stiffness includes the screw fastener slip and how this varies with the thickness of the stud. Additionally, with some manufacturers, the deflection of the stud must be added to the deflection from the rotation of the connector in order to arrive at the final deflection for design.

Because we have tested the entire assembly, Simpson Strong-Tie tabulates stiffness that includes connector deflection, fastener slip and stud deflection for walls up to 38" in height. Our stiffness also takes into account the thickness of the stud, making it simple for the Designer to calculate deflections: Simply divide the required moment by the tabulated stiffness, and then multiply the result by the stud length (Ref. Example #1 on p. 104). For walls over 38", a different approach is required (Ref. Example #2 on pp. 105–106).



SIMPSOI

Strong



RCKW Allowable Loads - Concrete Anchorage

	Fastener Pattern	Fasteners to Stud	Framing Members Thickness mil (ga.)	Allowable Moment M (inlb.)	Assembly Rotational Stiffness β	Connector Rotational Stiffness βc	Anchor Tension, T at Allowable Moment (Ib.)		Allowable Tension Load F2	Anchor Tension, T at Allowable Tension Load F2 (lb.)		Allowable Shear Load F4	Code Ref.
					(inlb./rad.)	(IIIID./ rau.)	f' _c = 3,000 psi	f' _c = 4,000 psi	(lb.)	f' _C = 3,000 psi	f' _c = 4,000 psi	(lb.)	
			33 (20)	2,425	87,000	92,900	1,870	1,790	860	1,080	1,055	620	
RCKW3	1	(4) #10	43 (18)	3,080	112,800	115,000	2,510	2,355	1,340	1,780	1,705	755	IP2
RUKW3		(4) #12	54 (16)	4,330	127,900	136,600	4,120	3,585	1,850	2,645	2,470	1,120	
			68 (14)	5,150	141,400	153,000	6,515	4,570	1,850	2,645	2,470	1,120	160
RCKW3			33 (20)	3,335	177,200	198,000	2,790	2,590	1,310	1,730	1,665	620	160
and		(9) #12	43 (18)	4,215	162,200	179,100	3,935	3,465	1,710	2,390	2,250	795	IP2
RCKW3S	2		54 (16)	5,160	139,600	149,800	6,700	4,585	2,220	3,410	3,085	1,120	
(Stiffener)			68 (14)	5,160	130,900	136,600	6,700	4,585	2,410	3,875	3,425	1,415	
		(6) #12	30 (20 DW)6,7	3,775	240,500	266,600	1,460	1,435	1,030	1,250	1,235	600	- 160 IP2
			33 (20 STR)7	4,670	259,700	281,000	1,830	1,795	1,140	1,395	1,375	665	
RCKW5.5	3		33 (20)	4,670	303,900	328,200	1,830	1,795	1,140	1,395	1,375	665	
NUK W0.0	3		43 (18)	6,245	333,100	355,300	2,525	2,450	1,440	1,790	1,755	1,035	
			54 (16)	8,225	306,000	320,500	3,470	3,320	2,455	3,255	3,125	1,390	
			68 (14)	10,290	422,400	441,400	4,570	4,290	2,455	3,255	3,125	1,390	160
RCKW5.5			33 (20)	4,855	255,500	271,800	1,910	1,870	1,660	2,090	2,040	665	160
and	4	(10) #12	43 (18)	8,445	449,800	489,600	3,580	3,420	2,165	2,815	2,720	1,035	IP2
RCKW5.5S	4		54 (16)	11,575	467,000	502,300	5,340	4,935	2,980	4,115	3,895	1,390	
(Stiffener)			68 (14)	13,935	432,300	456,300	7,020	6,215	2,980	4,115	3,895	1,830	
		(6) #12	33 (20)	6,445	388,500	402,000	1,815	1,790	1,095	1,315	1,300	795	IP2
RCKW7.5	5		43 (18)	8,200	510,400	535,800	2,345	2,305	1,280	1,550	1,530	1,200	
nurw7.0	5		54 (16)	11,400	553,900	571,200	3,370	3,275	2,165	2,715	2,655	1,695	
			68 (14)	13,895	605,200	627,800	4,225	4,065	2,165	2,715	2,655	1,695	160
RCKW7.5			33 (20)	8,705	495,100	516,700	2,505	2,453	1,730	2,130	2,095	795	160
and	6	(10) #12	43 (18)	10,915	590,800	623,400	3,210	3,125	2,255	2,840	2,775	1,200	IP2
RCKW5.5S	0		54 (16)	14,045	762,600	808,400	4,275	4,115	2,625	3,360	3,265	1,695	
(Stiffener)	(Stiffener)		68 (14)	16,670	615,800	631,300	5,245	4,985	2,665	3,420	3,320	2,065	

F2

1. For additional important information, see General Information on pp. 14-16.

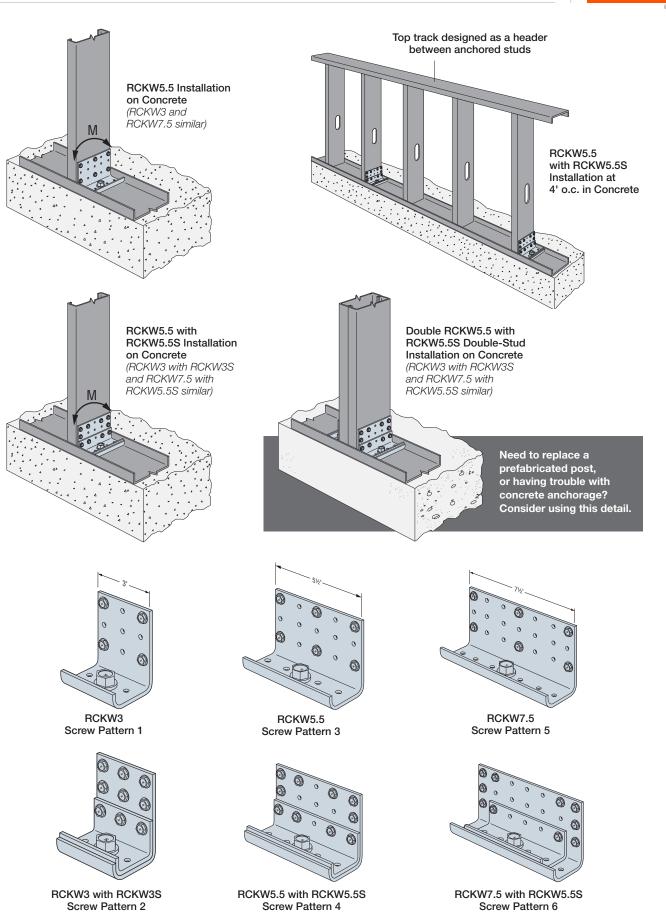
- 2. The Designer is responsible for anchorage design.
- 3. See illustrations for fastener pattern placement.
- 4. Screw length shall be selected based on the total thickness of materials to be joined such that the screws extend through the steel connection a minimum of three exposed threads.
- Tabulated values are based on framing members with track and 5. stud of the same thickness and (1) #10 screw into each stud flange unless otherwise noted.
- 6. Tabulated values may be used for framing members with track and stud of thickness 20 mil, $F_y = 57$ ksi (20 EQ).
- 7. Tabulated values are applicable for framing members with CFS track of thickness 20 mil, Fy = 57 ksi (20 EQ).
- 8. EQ equivalent, DW drywall, STR structural.
- 9. Tabulated moment values correspond to maximum connector strength without consideration of serviceability. Designer must check out-of-plane deflections using tabulated Rotational Stiffness.
- 10. Tabulated Assembly Rotational Stiffness is applicable for walls at 38" tall with corresponding framing member depth and thickness. Reference Example #1 on p. 104.
- 11. Tabulated Connector Rotational Stiffness may be used for wall heights other than 38" tall; the Designer must consider member deflection due to bending in the stud member. Reference Example #2 on pp. 105-106.
- 12. Per IBC 2015 Table 1604.3 footnote f, wind load is permitted to be taken as 0.42 times "component and cladding loads" for deflection checks. For IBC 2009 and earlier, the factor is 0.7 instead of 0.42.
- CFS wall stud ½"-diameter anchor RCKW clip F4 F4 ð Bearing pressure.**q** BY Tension,**t** RCKW5.5 and Anchor holt **RCKW5.5S Anchored** Anchor Bolt Design Into Concrete

- 13. Anchor Tension, (T), is the force in the anchor at maximum allowable moment (M) or maximum allowable vertical tension (F₂).
- 14. Anchor tension is calculated using AISC Steel Design Guide 1. The 'Anchor Bolt Design' illustration given below shows the anchor tension (T) based on an applied moment (M). An illustration for the anchor tension (T) based on a vertical tension load (F2) is not shown. The bearing pressure for F2 load is similar along the length of the clip as opposed to the width of the clip as shown.
- 15. Anchor tension may be interpolated.
- 16. For LRFD loads, multiply the ASD tabulated loads by 1.6.

Rigid Connectors

Connectors for Cold-Formed Steel Construction

RCKW Kneewall Connectors



SIMPSON

Strong-

103

Example #1: Exterior Parapet Stud

Given:

Rigid Connectors

- 2015 IBC (ASCE 7-10 and AISI S100-2012)
- 600S162-33 (33 ksi) studs @ 16" o.c. supported at the base
- Parapet height, L = 38"-tall studs
- Wind design pressure = 49.67 psf (LRFD)
- Deflection Limits, $\Delta_{allow} = L/240$ (Ref. IBC Table 1604.3)

Calculations:

Determine ASD wind pressure:

 $p = (0.6)(49.67 \ psf) = 29.8 \ psf$

Note: 2015 IBC load combinations for ASD include a factor of 0.6 for wind loads.

$$w = (29.8 \ psf) \frac{16 \ in.}{12 \ in.} = 39.7 \ plf$$

Determine Required Moment:

 $M_{req} = \frac{wL^2}{2} = \frac{(39.7\,\rho lf)(38\,in.)^2}{2\left(12\frac{in.}{ft.}\right)} = 2,389\,in.\text{-lb.}$

From Allowable Loads table (p. 102) for 600S162-33, 6"-deep 33-mil stud:

- Select RCKW5.5 connector, screw pattern 3, with ½" anchor diameter and (6) #12 self-drilling screws, attaching to each stud @ 16" o.c.
- Allowable Moment = 4,670 in.-lb. > 2,389 in.-lb. OK
- Assembly Rotational Stiffness, β = 303,900 in.-lb. / rad. for RCKW5.5 connector at 38" wall height

Select Anchorage:

Normal weight concrete with $f_c = 3,000$ psi Table 1 — Cracked Concrete, Wind and Seismic in SDC A&B Titen HD[®] with 31/4" embedment

M_{allow} = 3,010 in.-lb. > 2,389 in.-lb. **OK**

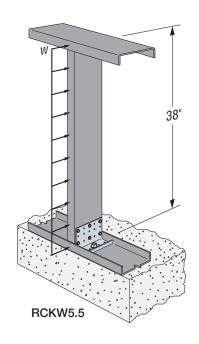
Check Deflection at Required Moment:

$$\Delta_{req} = \left(\frac{(0.7)(M_{req})}{\beta}\right) L = \left(\frac{(0.7)(2,389 \text{ in.-lb.})}{303,900 \frac{\text{in.-lb.}}{\text{rad.}}}\right) 38 \text{ in.} = 0.209 \text{ in.}$$

Note: Per IBC Table 1604.3 footnote f, 0.7 factor can be used to calculate deflections for components and cladding wind load.

Allowable Deflection:

$$\Delta_{allow} = \frac{2L}{240} = \frac{2(38 \text{ in.})}{240} = 0.317 \text{ in.} > 0.209 \text{ in. } \mathbf{OK}$$



Example #2: High Interior Half-Wall

Given:

- 2015 IBC (ASCE 7-10 and AISI S100-2012)
- The top track 600T125-54 (50 ksi) spans between 600S162-54 (50 ksi) studs @ spacing, S = 32" o.c. supported at the base
- 6" drywall studs at 16" o.c. as infill between the bottom and top track
- Wall height, L = 48"-tall studs
- Design Load: w = 50 plf or P = 200 lb. concentrated load for guard or handrail applications in accordance with Section 4.5.1 of ASCE (Ref. IBC 1607.8.1 and 1607.8.1.1)
- Deflection Limit, $\Delta_{\text{allow}} = L/120$ (Ref. IBC Table 1604.3)

Calculations:

Design criteria #1 for linear load of 50 lb./ft.

Determine Required Concentrated Load, Preq:

$$P = (w)(S) = (50 \text{ plf})(32 \text{ in.})\left(\frac{1 \text{ ft.}}{12 \text{ in.}}\right) = 133.3 \text{ lb.}$$

Determine Required Moment, Mreq:

 $M_{req} = (P_{req})(L) = (133.3 \ lb.)(48 \ in.) = 6,400 \ in.-lb.$

Design criteria #2 for concentrated load of 200 lb.

Note: From a 3D structural analysis with the 200 lb. concentrated load at the end stud, a continuous top track distributes some load to adjacent studs so that the worst-case moment is $M_{reg(max)} = 7,513$ in.-lb. as indicated in the illustration.

From Allowable Loads table (p. 102) for 600S162-54, 6"-deep, 54-mil stud:

- Select an RCKW5.5 connector with the RCKW5.5S stiffener, screw pattern 4 with (10) #12 self-drilling screws and a ½"-diameter anchor
- Allowable Moment = 11,575 in.- lb. > 6,400 in.- lb. (for linear load) OK
- Allowable Moment = 11,575 in.- lb. > 7,513 in.- lb. (for concentrated load) OK
- Connector Rotational Stiffness, $\beta_c = 502,300$ in.-lb./rad.

Check Deflection for Design Criteria #1 at Required Load:

Determine Stud Deflection, Δ_s , at P_{req} = 133.3 lb.

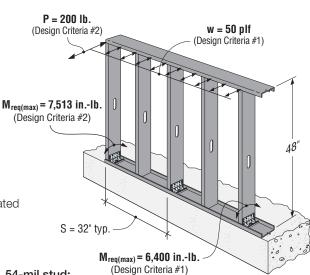
$$\Delta_{S} = \frac{P_{req}L^{3}}{3EI_{xe}} = \left(\frac{(133.3 \text{ lb.})(48 \text{ in.})^{3}}{(3(29,500,000 \text{ psi})(2.86 \text{ in.}^{4})}\right) = 0.058 \text{ in.}$$

Note: Effective moment of inertia for a 600S162-54 stud is $I_{xe} = 2.86$ in.⁴

Determine Connector Deflection, Δ_c , at M_{req} = 6,400 in.-Ib. by utilizing the Connector Rotational Stiffness, β_c = 502,300 in.-Ib. / rad. for RCKW5.5 and RCKW5.5S.

$$\Delta_{C} = \frac{M_{req}}{\beta_{C}} L = \frac{6,400 \text{ in.-lb.}}{502,300 \frac{\text{in.-lb.}}{\text{red}}} (48 \text{ in.}) = 0.612 \text{ in.}$$

Note: The Connector Rotational Stiffness may be used for wall heights other than 38"; the Designer must consider member deflection due to bending in the stud member. See footnote 11 of the Allowable Loads Table (p. 102).





Example #2: High Interior Half-Wall (cont.)

Total Deflection is the sum of the Stud Deflection and the Connector Deflection.

 $\Delta_{total} = \Delta_{s} + \Delta_{c} = 0.058 \text{ in.} + 0.612 \text{ in.} = 0.670 \text{ in.}$

Allowable Deflection:

 $\Delta_{allow} = \frac{2L}{120} = \frac{(2)(48 \text{ in.})}{120} = 0.800 \text{ in.} > 0.670 \text{ in.} \text{ OK}$

Check Deflection for Design Criteria #2 at Required Load:

Determine Stud Deflection, Δ_s , at M_{req} = 7,513 in.-lb. from concentrated load.

 $\Delta_{s} = \frac{M_{req}L^{2}}{3EI_{xe}} = \left(\frac{(7,513 \text{ in.-lb.})(48 \text{ in.})^{2}}{3(29,500,000 \text{ psi})(2.86 \text{ in.}^{4})}\right) = 0.068 \text{ in.}$

Determine Connector Deflection, Δ_c , at M_{req} = 7,513 in.-lb. by utilizing the Connector Rotational Stiffness, β_c = 502,300 in.-lb. / rad. for RCKW5.5 and RCKW5.5S.

 $\Delta_{c} = \frac{M_{req}}{\beta_{c}} L = \frac{7,513 \text{ in.-lb.}}{502,300 \frac{\text{in.-lb.}}{\text{rad.}}} (48 \text{ in.}) = 0.718 \text{ in.}$

Total Deflection is the sum of Stud Deflection and Connector Deflection.

 $\Delta_{total} = \Delta_s + \Delta_c = 0.068 \text{ in.} + 0.718 \text{ in.} = 0.786 \text{ in.}$

Allowable Deflection:

$$\Delta_{allow} = \frac{2L}{120} = \frac{(2)(48 \text{ in.})}{120} = 0.800 \text{ in.} > 0.786 \text{ in.}$$
 OK

Note: Per ASCE Section 4.5.1, for handrail and guardrail systems, there is no need to apply the 50 plf linear load and the 200 lb. concentrated load concurrently. Example #2 demonstrates the design for both loading cases, and the outermost anchored stud governs when using the 200 lb. concentrated load.

Table 1: RCKW Allowable Moments Using Simpson Strong-Tie $^{\rm \tiny 8}$ Anchorage Solutions — Edge of Slab

	Simpson Strong-Tie	Edge of Concrete Slab Allowable Moment, M (inlb.)									
Madal	Anchor Model ½" Diameter	Titen HD®	Titen HD	Strong-Bolt [®] 2	AT-XP®	SET-XP®	SET-XP	AT-XP	SET-XP		
Model No.	Minimum Concrete Thickness, h _{min} (in.)	5	6	6	6	6 3½	6½ 4	9½ 7	91⁄2		
	Nominal Embedment Depth, h _{nom} (in.)	31⁄4	3¾	37⁄8	31⁄2				7		
			Uncracked Con	crete, Wind and S	eismic in SDC	A&B ^{8,10}					
				f'c = 4,000 p	si						
	SLWC	1,020	1,130		630	770	_	_	_		
RCKW3	NWC	1,635	1,795		1,200	1,440	_		_		
DOLUME E	SLWC	2,960	3,150	_	1,755	2,085	_		_		
RCKW5.5	NWC	4,800	5,255	_	3,365	3,980	_		_		
	SLWC	4,280	4,845	4,545	3,225	3,765	_	_	_		
RCKW7.5	NWC	7,120	8,080	7,430	6,200	7,010	_	_	_		
				f'c = 3,000 p	si						
	SLWC	875	970		630	805	_		_		
RCKW3	NWC	1,390	1,530		1,175	1,295					
	SLWC	2,545	2,715		2,185	1,750					
RCKW5.5	NWC	4,110	4,680		3,825	3,330	_	_			
	SLWC	3,680	3,990	3,920	3,945	3,225	_				
RCKW7.5	NWC	6,125	6,945	6,385	6,800	6,155	_	_	_		
	11		Cracked Conc	rete, Wind and Se	eismic in SDC A	& B ^{8,10}	1	,			
				f' _c = 4,000 p							
Davaua	SLWC	740	815		520		470	1,010	805		
RCKW3	NWC	1,200	1,320	_	990	_	895	1,865	1,510		
	SLWC	2,145	2,250	_	1,440	_	1,260	2,745	2,180		
RCKW5.5	NWC	3,500	3,770	_	2,770	_	2,435	4,030	4,030		
00/01/75	SLWC	3,075	3,470	3,630	2,645	_	2,270	4,220	3,935		
RCKW7.5	NWC	5,105	5,785	5,960	5,105	_	4,395	6,220	6,210		
ĺ				f' _c = 3,000 p	si						
	SLWC	640	705		515	_	465	990	790		
RCKW3	NWC	1,030	1,130		970		880	1,610	1,460		
	SLWC	1,845	1,945		1,430		1,255	2,370	2,160		
RCKW5.5	NWC	3,010	3,235		2,735		2,405	3,470	3,360		
	SLWC	2,635	2,995	3,105	2,630	_	2,260	3,640	3,640		
RCKW7.5	NWC	4,390	4,970	5,170	5,050	_	4,355	5,355	5,545		
	•		Cracked Cor	crete, Seismic in	SDC C Through	1 F ^{9,10}					
				f' _C = 4,000 p	si						
RCKW3	SLWC	265	295	<u> </u>	155		165	310	290		
101/10	NWC	440	485		305		325	605	560		
RCKW5.5	SLWC	765	875	—	435	_	445	865	780		
	NWC	1,265	1,445	—	845		870	1,675	1,510		
RCKW7.5	SLWC	1,180	1,470	1,440	795	_	805	1,585	1,400		
	NWC	1,960	2,430	2,385	1,555		1,565	2,900	2,510		
				f' _c = 3,000 p	si						
RCKW3	SLWC	230	255	—	155	—	165	310	290		
100/000	NWC	380	420	—	305	_	325	595	555		
RCKW5.5	SLWC	660	755		435	—	445	860	775		
100.0	NWC	1,090	1,250	—	845	—	865	1,630	1,500		
RCKW7.5	SLWC	1,020	1,270	1,245	795	—	800	1,580	1,395		
100.007.0	NWC	1,695	2,100	2,060	1,550	—	1,560	2,500	2,515		

See footnotes on p. 110.

SIMPSON Strong-Tie

Table 2: RCKW Allowable Moments Using Simpson Strong-Tie® Anchorage Solutions — Center of Concrete Slab

	Simpson Strong-Tie	Edge of Concrete Slab Allowable Moment, M (inlb.)									
Model No.	Anchor Model ½" Diameter Minimum Concrete Thickness, h _{min} (in.)	Titen HD®	Titen HD 6	Strong-Bolt® 2 6	AT-XP®	SET-XP®	SET-XP 6½ 4	AT-XP 9½	SET-XP		
		5							91⁄2		
	Nominal Embedment Depth, h _{nom} (in.)	31⁄4	3¾	37⁄8	31⁄2	31⁄2		7	7		
			Uncracked Co	ncrete, Wind and S	eismic in SDC	A&B ^{8,10}					
				f' _c = 4,000 p	si						
	SLWC	1,785	2,250	2,135	2,030	2,470	2,780	3,705	4,395		
RCKW3	NWC	2,825	3,500	3,340	3,645	4,330	4,435	4,435	4,435		
	SLWC	3,385	4,305	4,080	3,870	4,760	5,395	7,385	7,505		
RCKW5.5	NWC	5,495	6,930	6,580	7,255	7,505	7,505	7,505	7,505		
DOM/WZ F	SLWC	4,665	5,955	5,635	5,340	6,590	7,485	7,565	7,515		
RCKW7.5	NWC	7,625	9,675	9,175	7,500	7,500	7,500	7,565	7,515		
				f' _c = 3,000 p	si						
	SLWC	1,530	1,915	1,825	1,970	2,380	2,660	3,470	4,030		
RCKW3	NWC	2,400	2,950	2,820	3,420	3,810	4,065	4,065	4,065		
DOM/US 5	SLWC	2,915	3,700	3,505	3,810	4,670	5,275	7,150	7,580		
RCKW5.5	NWC	4,705	5,920	5,625	7,025	7,580	7,580	7,580	7,580		
DOWNER	SLWC	4,020	5,125	4,855	5,280	6,500	7,365	7,525	7,525		
RCKW7.5	NWC	6,555	8,295	7,870	7,525	7,500	7,500	7,525	7,525		
			Cracked Con	crete, Wind and Se	ismic in SDC A	&B ^{8,10}					
				f' _c = 4,000 p	si						
RCKW3	SLWC	1,295	1,640	1,560	1,360		1,315	2,575	2,210		
	NWC	2,080	2,605	2,485	2,530	_	2,450	4,435	3,935		
RCKW5.5	SLWC	2,425	3,095	2,940	2,560	_	2,470	4,965	4,235		
	NWC	3,970	5,035	4,790	4,875		4,715	7,505	7,505		
	SLWC	3,335	4,265	4,045	3,515	_	3,395	6,880	5,850		
RCKW7.5	NWC	5,480	6,980	6,630	6,755	_	6,525	7,565	7,515		
				f' _c = 3,000 p	si						
DOLUMO	SLWC	1,110	1,405	1,335	1,335		1,295	2,475	2,140		
RCKW3	NWC	1,775	2,215	2,115	2,435	_	2,360	4,065	3,660		
	SLWC	2,090	2,665	2,535	2,535	_	2,450	4,865	4,160		
RCKW5.5	NWC	3,410	4,320	4,110	4,780	_	4,625	7,580	7,580		
DOWNZE	SLWC	2,875	3,675	3,490	3,490		3,370	6,780	5,780		
RCKW7.5	NWC	4,720	6,005	5,705	6,660		6,435	7,525	7,525		
			Cracked Co	ncrete, Seismic in S	SDC C Through	h F ^{9,10}					
				f' _c = 4,000 p	si						
DOMANO	SLWC	470	600	570	420	_	475	830	825		
RCKW3	NWC	770	985	935	810	_	920	1,575	1,565		
DOM/NE E	SLWC	865	1,110	1,050	775	_	880	1,540	1,530		
RCKW5.5	NWC	1,430	1,835	1,740	1,510	_	1,710	2,970	2,735		
DOWNZE	SLWC	1,180	1,515	1,440	1,060	_	1,205	2,110	2,095		
RCKW7.5	NWC	1,960	2,515	2,385	2,070	_	2,345	2,985	2,730		
				f' _c = 3,000 p	si						
DOLANS	SLWC	405	515	490	420	_	475	820	815		
RCKW3	NWC	665	845	805	805	_	905	1,540	1,530		
	SLWC	745	960	910	775		880	1,530	1,520		
RCKW5.5	NWC	1,235	1,580	1,500	1,500		1,700	2,935	2,765		
	SLWC	1,020	1,310	1,245	1,060	<u> </u>	1,200	2,100	2,085		
RCKW7.5	NWC	1,695	2,170	2,060	2,060	_	2,335	2,860	2,860		

See footnotes on p. 110.

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RCKW Kneewall Connectors

Table 3: RCKW Allowable Tension and Shear Loads Using Simpson Strong-Tie $^{\rm \tiny \$}$ Anchorage Solutions — Edge of Concrete Slab

		Strong-Tie			Edge of Co	ncrete Slab All	owable Moment,	M (inlb.)		
Madal		r Model ameter	Titen HD®	Titen HD	Strong-Bolt® 2	AT-XP [®]	SET-XP®	SET-XP	AT-XP	SET-XP
Model No.		i Concrete 5, h _{min} (in.)	5	6	6	6	6	6½	91⁄2	91⁄2
		mbedment I _{nom} (in.)	31⁄4	3¾	37⁄8	3½	31⁄2	4	7	7
			Uncrack	ed Concrete, W	/ind and Seismic	in SDC A&B ^{8,10}	(f' _c = 4,000 psi)			
	01,040	Tension	720	805	_	435	535	_	_	_
DOV/MO	SLWC	Shear	365	375	_	445	445	_	_	_
RCKW3	NIMO	Tension	1,200	1,340	_	855	1,045	_	_	
	NWC	Shear	605	625	—	655	655	_	_	_
	01,140	Tension	1,120	1,290	_	655	760	_	_	_
DOLUME E	SLWC	Shear	780	810	_	960	960	_	_	
RCKW5.5		Tension	1,865	2,150	_	1,280	1,500	_	_	
	NWC	Shear	1,305	1,350	_	1,410	1,410	_	_	_
	0,1110	Tension	1,280	1,585	1,245	875	1,025	_	_	_
BOUNDE	SLWC	Shear	1,100	1,245	1,295	1,480	1,480	_	_	_
RCKW7.5		Tension	2,130	2,645	2,080	1,720	2,015	_	_	_
	NWC	Shear	1,830	2,075	2,160	1,800	1,800	_	_	_
	1	1	Cracke	d Concrete, Wi	nd and Seismic in	SDC A&B ^{8,10} (1	f' _c = 4,000 psi)		1	
		Tension	515	570		355	_	320	710	560
	SLWC	Shear	260	270		320		325	325	325
RCKW3		Tension	860	950	_	700		630	1,395	1,100
	NWC	Shear	430	445		465		480	480	480
		Tension	800	920		535		465	1,065	815
	SLWC	Shear	560	580		685		705	705	705
RCKW5.5		Tension	1,335	1,530	_	1,045		915	2,090	1,600
	NWC	Shear	930	965		1,010		1,035	1,035	1,035
		Tension	905	1,130	1,105	715		615	1,435	1,075
	SLWC	Shear	785	890	925	1,055		1,085	1,085	1,085
RCKW7.5		Tension	1,510	1,880	1,845	1,405		1,205	2,810	2,110
	NWC	Shear	1,310	1,485	1,540	1,550		1,595	1,595	1,595
		onour			eismic in SDC C T		– 4 000 nsi)	1,000	1,000	1,000
		Tension	180	200		105	; = 4,000 pol)	110	210	195
	SLWC	Shear	120	125	_	105		110	150	195
RCKW3		Tension	300	335	_	210		220	415	385
	NWC	Shear	200	210		210		220	225	225
					_		—		315	285
	SLWC	Tension Shear	280	320		160		165		
RCKW5.5			260	270	-	320		330	330	330
	NWC	Tension	470	535	—	310	—	320	620	560
		Shear	435	450		470	—	485	485	485
	SLWC	Tension	315	395	385	215	—	215	425	375
RCKW7.5		Shear	365	415	430	495	—	505	505	505
	NWC	Tension	530	660	645	420	—	420	835	740
		Shear	610	690	720	725	—	645	745	645

SIMPSON

Strong-Tie

RCKW Kneewall Connectors



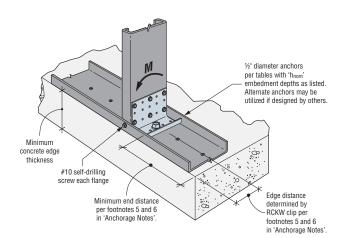
Table 4: RCKW Allowable Tension and Shear Loads Using Simpson Strong-Tie® Anchorage Solutions — Center of Concrete Slab

	Simpson S	trong-Tie®			Allowa	able Tension ar	nd Shear Load	(lb.)				
	Anchor	Model	М	echanical Anc	hors			Epoxy Anchors				
Model	½" Dia	ameter	Titen HD®	Titen HD	Strong-Bolt [®] 2	AT-XP®	SET-XP®	SET-XP	AT-XP	SET-XP		
No.	Minimum Thickness	Concrete , h _{min} (in.)	5	6	6	6	6	6½	91⁄2	91⁄2		
	Nominal Ei Depth, h	mbedment _{nom} (in.)	3¼	3¾	37⁄8	31⁄2	31⁄2	4	7	7		
			Uncra	acked Concret	e, Wind and Seisn	nic in SDC A&E	$B^{8,10}$ (f' _c = 4,000) psi)				
	SLWC	Tension	1,280	1,645	1,555	1,470	1,830	2,090	2,940	3,660		
	SLVVC	Shear	1,380	2,685	2,820	1,925	1,925	1,925	1,925	1,925		
	NWC	Tension	2,130	2,745	2,595	2,885	3,360	3,705	3,705	3,705		
	NWC	Shear	2,295	2,685	2,820	1,925	1,925	1,925	1,925	1,925		
			Cracked Concrete, Wind and Seismic in SDC A&B ^{8,10} (f ¹ c = 4,000 psi)									
	SLWC	Tension	905	1,165	1,105	960	_	925	1,915	1,615		
ACKW all models	SLWG	Shear	975	2,510	2,820	1,925	_	1,925	1,925	1,925		
	NWC	Tension	1,510	1,945	1,845	1,880	_	1,810	3,705	3,170		
	NWG	Shear	1,625	2,685	2,820	1,925	_	1,925	1,925	1,925		
			Cra	acked Concret	e, Seismic in SDC	C Through F ^{9,1}	° (f' _c = 4,000 p	si)				
	01.00	Tension	315	410	385	285	_	325	570	565		
	SLWC	Shear	455	805	1,185	765	_	700	765	700		
	NWC	Tension	530	680	645	560	_	635	1,115	1,110		
	NWC	Shear	760	805	1,185	765		700	765	700		

- Anchor Allowable Loads have been determined using ACI 318-14 Chapter 17 anchorage calculations with the minimum concrete compressive strength, f^r_c, and slab thickness listed. Sand-Lightweight Concrete is abbreviated as 'SLWC', Normal Weight Concrete is abbreviated as 'NWC'.
- 2. Load values are for a single anchor based on ACI 318-14, condition B, load factors from ACI 318-14 Section 5.3, no supplemental edge reinforcement, $\Psi_{\text{C},\text{V}}$ = 1.0 for cracked concrete and periodic special inspection. Reference ICC-ES or IAPMO-UES evaluation reports for further information.
- Load values are based on short-term temperature range of 150°F and 180°F for SET-XP and AT-XP adhesive, respectively. Long term temperature range is assumed to be 110°F for both SET-XP and AT-XP adhesives.
- 4. Allowable Stress Design (ASD) values were determined by multiplying calculated Strength Design values by a conversion factor, Alpha (α), of 0.7 for seismic loads and 0.6 for wind loads. ASD values for other load combinations may be determined using alternate conversion factors.
- 5. At edge of slab, edge distances are assumed to be 1%", 3.0" and 4.0" ($\frac{1}{2}$ of stud width) as determined for 3%", 6" and 8" studs, respectively. 'End distances' are assumed as 1.5 x Min. Edge Distance in one direction and 'N/A' in the other direction. See figure on this page.
- At center of slab, edge and end distances are assumed as 'N/A' in all directions at locations away from edge of slab. See RCKW to concrete illustration below.
- Tabulated anchorage capacities for RCKW models shown are applied to the same model size with stiffener. For example, a value for model RCKW3 is equivalent to model RCKW3 and RCKW3S.
- Tabulated allowable ASD loads for Wind and Seismic in SDC A&B are based on using wind conversion factors and may be increased by 1.17 for seismic SDC A&B only.
- Allowable loads have been divided by an Omega (Ω) seismic factor of 2.5 for brittle failure as required by ACI 318-14 Chapter 17, unless steel failure governs.
- 10. Tabulated capacities are based on maximum allowable anchorage loads only. The capacity of the connection system shall be the minimum of the tabulated value and the RCKW allowable load value listed on p. 102.

11. Tabulated loads in Table 3 and 4 are based on $f_{\rm C}^{\rm c} = 4,000$ psi. For $f_{\rm C}^{\rm c} = 3,000$ psi use an adjustment factor of 0.86 for the blue shaded values and 1.0 for all other values.

- 12. Tabulated values in Table 1 and Table 2 have been checked for combine moment and shear with the following conditions: $N_a / N_{al} + V_a / V_{al} \le 1.2$
 - $N_a = Applied ASD anchor tension load at moment M$
 - N_{a} = Allowable anchor tension load from Tables 3 or 4
 - $V_a = 2M/L$ is based on uniform loading wind application. L is the wall height at 38".
 - $V_a = 200/L$ is based on uniform loading wind application. L is the wall height at 38 • $V_{al} =$ Allowable shear load from Tables 3 or 4
 - Blue shaded values in Tables 1 and 2 exceed 1.2 combine loading limits. Designer is responsible to check.
- For anchor subjected to both tension and shear loads, it shall be designed to satisfy following:
 - For $N_a / N_{al} \le 0.2$, the full allowable load in shear is permitted.
 - For $V_a / V_{al} \le 0.2$, the full allowable load in tension is permitted.
 - For all other cases: $N_a / N_{al} + V_a / V_{al} \le 1.2$ where:
 - $N_a = Applied ASD tension load$
 - $N_{al} = Allowable tension load from Tables 3 or 4$
 - $V_a =$ Applied ASD shear load
 - V_{al} = Allowable shear load from Tables 3 or 4.



RCKW Kneewall Connectors

SIMPSON Strong-Tie

RCKW Allowable Loads - Steel Anchorage

Model No.	Framing Member Depth (in.)	Fastener to Structural Steel ²	Fastener to Stud ³	Framing Member Thickness mil (ga.)	Allowable Moment ^{4,5} M (inIb.)	Assembly Rotational Stiffness ^{6,8} β (inlb./rad)	Connector Rotational Stiffness ^{7,8} β _c (inlb./rad)	Allowable Tension Load F2 (lb.)	Allowable Shear Load F4 (lb.)	Code Ref.	
				33 (20)	2,105	55,500	58,000	850	455		
RCKW3	3.625	(2) #12	(4) #12	43 (18)	2,570	73,300	76,700	1,225	745		
				54 (16)	2,690	87,260	91,200	1,115	1,115		
				33 (20)	5,165	199,200	209,200	1,245	650		
RCKW5.5	6.00	(4) #12	(6) #12	(6) #12	43 (18)	6,370	272,600	287,100	1,900	1,060	170
				54 (16)	6,430	255,900	266,100	2,310	1,295		
				33 (20)	7,030	456,700	483,200	965	655		
RCKW7.5	RCKW7.5 8.00 (6) #12		(6) #12	43 (18)	9,595	571,600	603,600	1,950	1,135		
				54 (16)	11,320	693,600	731,600	2,535	1,710		

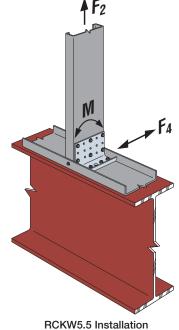
1. For additional important information, see General Information on pp. 14–16.

2. Designer is responsible for structural steel design.

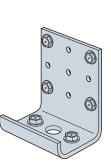
3. See illustrations for fastener patterns.

4. Tabulated values are based on framing members with track and stud of the same thickness and #10 screws into each stud flange.

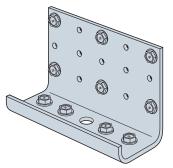
- 5. Tabulated moment values correspond to the maximum connector strength without consideration of serviceability. Designer must check out-of-plane deflections using tabulated Rotational Stiffness.
- Tabulated Assembly Rotational Stiffness is for walls at 38" tall. Reference Example #1 on p. 104.
 The tabulated Connector Rotational Stiffness is for walls other than 38" tall. The Designer must consider member deflection due to bending in the stud. Reference Example #2 on pp. 105–106.
- Per IBC 2015 Table 1604.3 footnote f, wind load is permitted to be taken as 0.42 times "component and cladding loads" for deflection checks. For IBC 2009 and earlier, the factor is 0.7 instead of 0.42.



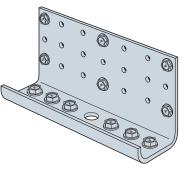
on Structural Steel (RCKW3 and RCKW7.5 similar)



RCKW3 Screw Pattern for Steel Anchorage



RCKW5.5 Screw Pattern for Steel Anchorage



RCKW7.5 Screw Pattern for Steel Anchorage

MSSC4.25KW and MSSC6.25KW Kneewall Connectors

SIMPSON Strong

MSSC connectors are designed to work in tandem with Simpson Strong-Tie® BP1/2-3 bearing plates to provide solutions for moment-resisting kneewall lighter-duty applications.

Features:

- One simple custom hole pattern for each stud size simplifies specification and installation
- 3/8" diameter anchor bolt location enables easy tool access
- Material: MSSC 97 mil (50 ksi); BP 229 mil (33 ksi)

Finish: MSSC - Galvanized (G90); BP - None

Installation:

Rigid Connectors

- Use all specified fasteners/anchors
- Install BP1/2-3 bearing plate over anchor leg of MSSC connectors as shown in the illustrations

Codes: See p. 11 for Code Reference Key Chart

Ordering Information

Model No.	Ordering SKU	Package Quantity
MSSC4.25KW	MSSC4.25KW-KT20	Box of 20 connectors
MSSC6.25KW	MSSC6.25KW-KT20	and 20 BP bearing plates

Allowable Loads

	Connector		Framing	Faste	ners ⁵	0 1 1		Anchor	Rotational	
Model No.	Material Thickness mil (ga.)	L (in.)	Member Depth (in.)	Anchor Diameter (in.)	Stud	Stud Thickness mil (ga.)	Allowable Moment, M (inIb.) ¹	Tension at Allowable Moment (lb.) ²	Stiffness for Wind Deflection (inlb./rad.) ^{3,4}	Code Ref.
						33 (20)	3,135	1,690		
MSSC4.25KW	97 (12)	4¼	6	3⁄8	(8) #10	43 (18)	4,320	2,375	45,360	
						54 (16)	5,830	2,765		IP2
						33 (20)	3,845	1,705		IPZ
MSSC6.25KW	97 (12)	6¼	8	3⁄8	(12) #10	43 (18)	3,845	1,705	77,245	
						54 (16)	8,350	2,885		

Minimum concrete edge thickness.

> # SC

1. Tabulated values correspond to maximum connector strength without consideration of serviceability.

Designer must check out-of-plane deflections using tabulated rotational stiffness.

2. Uplift may be linearly interpolated for design moment less than allowable. Designer is responsible for anchorage design.

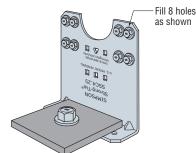
3. Tabulated stiffness is applicable for walls up to 38" tall. For taller walls, the Designer must consider additional

deflection due to bending in the studs.

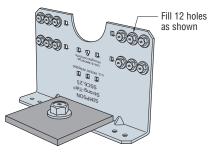
4. Per IBC 2015 Table 1604.3 footnote f, wind load is permitted to be taken as 0.42 times "component and cladding loads" for deflection checks. For IBC 2009 and earlier, the factor is 0.7 instead of 0.42.

5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.





MSSC4.25KW Fastener Pattern



MSSC6.25KW Fastener Pattern

·	M		with 'h _{non} depths as	ors per tables n'embedment listed. Alternate nay be utilized if by others.
			8. 8.	– Min. 33 mil (20 ga.) track.
per footno	end distance tes 3 and 4 .113.	i de	\times \checkmark	Edge distance determined by MSSCKW clip per footnotes 3 and 4

Typical MSSCKW Installation

on p. 113.

MSSC4.25KW and MSSC6.25KW Kneewall Connectors

Kneewall Connector Anchorage Solutions

		Uncracked Cor	ncrete, Wind a	and Seismi	ic in SDC A	& B ^{8,10}			
		%" Diameter	Nominal		Allo	wable Mor	nent, M (in	n-lb.)	
Model No.	Min. Concrete Thickness	Simpson	Embedment Depth	E	dge of Slal	b ³	Ce	enter of Sla	ıb4
	(h _{min})	Strong-Tie® Anchor Type	(h _{nom}) (in.)	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC
	4" or thicker	STB2	21⁄4	_	—	—	1,220	2,040	2,365
	4 OF UTICKET	Titen HD®	21⁄2	1,255	2,090	2,425	1,255	2,090	2,425
		STB2	21/8	—	—	—	1,555	2,590	2,995
MSSC4.25KW	6" or thicker	Titen HD	31⁄4	1,795	2,995	3,450	2,075	3,465	3,995
W15564.25KW	O UI LITICKEI	SET-XP®	4	725	1,425	1,425	1,930	3,705	3,705
		AT-XP®	4	750	1,470	1,470	2,005	3,705	3,705
	Concrete	SET-XP	71⁄2	670	1,320	1,320	3,610	3,705	3,705
	thickness ≥ 9.5 "	AT-XP	71⁄2	695	1,360	1,360	3,690	3,705	3,705
	4" or thicker	STB2	21⁄4	_	—	—	1,515	2,530	2,930
	4 OF UTICKET	Titen HD	21/2	1,555	2,590	3,005	1,555	2,590	3,005
		STB2	21/8	—	_		1,930	3,215	3,715
	C" or this lar	Titen HD	31⁄4	2,570	4,295	4,950	2,570	4,295	4,950
MSSC6.25KW	6" or thicker	SET-XP	4	1,110	2,170	2,170	2,395	4,595	4,595
		AT-XP	4	1,135	2,235	2,235	2,480	4,595	4,595
	Concrete	SET-XP	71⁄2	1,030	2,015	2,015	4,480	4,595	4,595
	thickness ≥ 9.5 "	AT-XP	71⁄2	1,055	2,065	2,065	4,575	4,595	4,595

		Cracked Cond	crete, Wind ar	nd Seismic	in SDC A	& B ^{8, 10}				
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$										
Model No				E	dge of Sla	b³	Ce	enter of Sla	ເb⁴	
			(h _{nom})							
	4" or thickor	STB2	21⁄4	_	—	—	860	1,435	1,660	
	4 OF LITICKEI	Titen HD	21⁄2	575	955	1,100	575	955	1,100	
		STB2	21⁄8	—	—	—	1,295	2,150	2,495	
MOOCA DEKW	6" or thickor	Titen HD			2,095	2,430	1,255	2,095	2,430	
1013304.23KW	0 UI LIIICKEI	SET-XP	4	1,175	2,305	2,305	1,485	2,915	2,915	
		AT-XP	4	1,220	2,395	2,395	1,560	3,065	3,065	
	Concrete	SET-XP	71/2	2,200	3,705	3,705	2,790	3,705	3,705	
	thickness ≥ 9.5 "	AT-XP	71⁄2	2,290	3,705	3,705	2,935	3,705	3,705	
	4" or thickor	STB2	21⁄4	_	—	—	1,070	1,780	2,055	
	4 OF LITICKEI	Titen HD	21⁄2	715	1,185	1,365	715	1,185	1,365	
		STB2	21/8	—	_	_	1,605	2,665	3,090	
MSSC6.25KW	6" or thickor	Titen HD	31⁄4	1,555	2,600	3,010	1,555	2,600	3,010	
1013360.2360	6" or thicker	SET-XP	4	1,795	3,505	3,505	1,840	3,615	3,615	
		AT-XP	4	1,860	3,645	3,645	1,935	3,800	3,800	
	Concrete	SET-XP	71⁄2	3,350	4,595	4,595	3,455	4,595	4,595	
	thickness ≥ 9.5 "	AT-XP	71⁄2	3,490	4,595	4,595	3,640	4,595	4,595	

		Cracked Co	ncrete, Seism	ic in SDC (C through	F ^{9, 10}			
		%" Diameter	Nominal		Allo	wable Mor	nent, M (in	-lb.)	
Model No.	Min. Concrete Thickness	Simpson	Embedment Depth	E	dge of Sla	b ³	Ce	nter of Sla	ເb⁴
	(h _{min})	Strong-Tie® Anchor Type	(h _{nom}) (in.)	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC	3,000 psi SLWC	3,000 psi NWC	4,000 psi NWC
	4" or thicker	STB2	21⁄4	_		—	300	500	580
	4 UI LIIICKEI	Titen HD	21⁄2	200	335	385	200	335	385
		STB2	21/8	_	—	—	450	755	870
MSSC4.25KW	6" or thicker	Titen HD	31⁄4	440	735	850	440	735	850
1013304.25KW	0 UI LIIICKEI	SET-XP	4	410	805	805	520	1,020	1,020
		AT-XP	4	430	840	840	550	1,070	1,070
	Concrete	SET-XP	71⁄2	770	1,495	1,495	975	4,325	4,325
	thickness ≥ 9.5 "	AT-XP	71⁄2	800	1,575	1,575	1,025	4,325	4,325
	4" or thicker	STB2	21⁄4	—	—	—	375	620	720
	4 OF LITICKET	Titen HD	21⁄2	250	415	480	250	415	480
		STB2	21/8	_	—	—	560	935	1,080
MSSC6.25KW	6" or thicker	Titen HD	31⁄4	545	910	1,050	545	910	1,050
1013300.23KW	0 OF LITICKEI	SET-XP	4	625	1,225	1,225	645	1,265	1,265
		AT-XP	4	650	1,275	1,275	680	1,330	1,330
	Concrete	SET-XP	71⁄2	1,180	5,360	5,360	1,210	5,360	5,360
	thickness ≥ 9.5 "	AT-XP	71⁄2	1,220	5,310	5,310	1,270	5,310	5,310

1. Allowable Moments have been determined using ACI 318-14 Chapter 17 anchorage calculations with the minimum concrete compressive strength, f'c and slab thickness listed. Sand-Lightweight Concrete is abbreviated as 'SLWC', Normal Weight Concrete is abbreviated as 'NWC'. 2. Nominal Embedment Depth/Effective Embedment Depth relationships:

- %" Titen HD[®] in 4" concrete: 2.50" (h_{nom}) / 1.77" (h_{ef})
- 3/8" Titen HD in 6" concrete: 3.25" (hnom) / 2.40" (hef)
- 3/8" Carbon Steel STB2 into 4" concrete:
- 2.25"(hnom)/1.875"(hef) 3/8" Carbon Steel STB2 into 6" concrete:
- 2.875"(h_{nom})/2.5"(h_{ef}) SET-XP[®] or AT-XP[®] Adhesive with 3%" F1554 Gr. 36 All-Thread Rod in 6" concrete: 4.0" $(h_{nom}) = 4$ " (h_{ef})
- SET-XP or AT-XP Adhesive with 3/ F1554 Gr. 36 All-Thread Rod in 9.5" concrete: 7.5" (hnom) = 7.5" (hef)
- 3. At edge of slab, edge distances are assumed to be 3.0" and 4.0" (1/2 of stud width) as determined for 6" and 8" studs, respectively. 'End distances' are assumed as 1.5 x Min. Edge Distance in one direction and 'N/A' in the other direction. See figure on p. 112.
- 4. At center of slab, edge and end distances are assumed as 'N/A' in all directions at locations away from edge of slab. See figure on p. 112.
- 5. Load values are for a single anchor based on ACI 318-14, condition B, load factors from ACI 318-14 Section 5.3, no supplemental edge reinforcement, ψ_{C} , v = 1.0 for cracked concrete and periodic special inspection. Reference ICC-ES or IAPMO-UES evaluation reports for further information.
- 6. Load values are based on a short-term temperature range of 150°F and 180°F for SET-XP and AT-XP. Long-term temperature range is assumed to be 110°F for both SET-XP and AT-XP. Dry hole conditions are assumed.Other conditions may be evaluated using Anchor Designer' Software for ACI 318, ETAG and CSA. See strongtie.com/software.
- 7. Allowable Stress Design (ASD) values were determined by multiplying calculated LRFD capacities by a conversion factor, Alpha (α), of 0.7 for seismic loads and 0.6 for wind loads. ASD values for other load combinations may be determined using alternate conversion factors.
- 8 Tabulated allowable ASD loads for Wind and Seismic in SDC A&B are based on using wind conversion factors and may be increased by 1.17 for SDC A&B only.
- 9 Allowable loads have been divided by an Omega (Ω) seismic factor of 2.5 for brittle failure as required by ACI 318-14 Chapter 17, unless steel failure governs.
- 10. Tabulated allowable moments are for MSSC Kneewall Connectors attached to studs with 33 (20) or 43 (18) mil (ga.) thickness. Allowable moment may be increased for MSSC Kneewall Connectors attached to studs with 54 (16) mil (ga.) thickness by multiplying by a factor of 1.16 for MSSC4.25KW and 1.28 for MSSC6.25KW.
- 11. Tabulated capacities assume lateral force applied at height of 38" above concrete. Tabulated capacities are based on maximum allowable anchorage loads only. The capacity of the connection system shall be the minimum of the tabulated value and the allowable load value from the MSSCKW Connectors: Allowable Load Tables.

SIMPSON Strong-I

Bridging and Bracing Connectors

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Simplified Design and Installation Through Innovation

Simpson Strong-Tie® SUBH and MSUBH wall stud bridging connectors for cold-formed steel (CFS) framing offer a compact profile that allows standard 1%" studs to be sistered directly against adjacent studs. The LSUBH connector provides the same installation benefits of the SUBH/MSUBH connectors, and is suitable for many wind- and

load-bearing situations where the load demand is light to moderate. Many applications require only one screw, greatly reducing labor costs and increasing productivity.

Features:

- Tested to include stud-web strength and stiffness in the tabulated design values
- Design values ensure compliance with AISI S100 Sections D3.2.1 and D3.3 for axially and laterally loaded studs
- Flexible design solutions for web thicknesses of 33 mil (20 ga.) through 97 mil (12 ga.) and stud sizes from 35%" to 8"
- MSUBH accommodates back-to-back built-up members ranging from 33 mil (20 ga.) to 54 mil (16 ga.)

 $\mbox{Material:}\ LSUBH3.25-33$ mil (20 ga.); SUBH3.25-43 mil (18 ga.); MSUBH3.25-68 mil (14 ga.)

Finish: Galvanized (G90)

Installation:

• See pp. 116 through 118

Codes: See p. 11 for Code Reference Key Chart

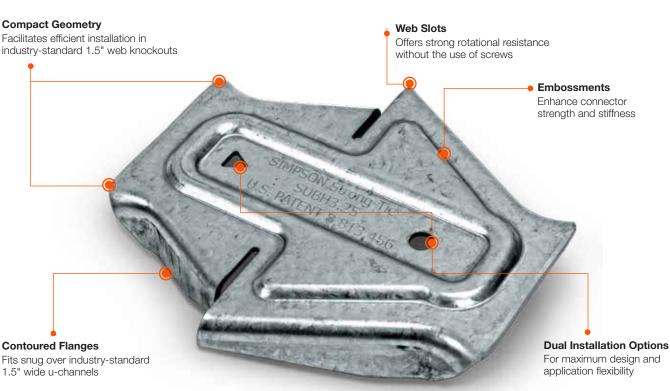
Ordering Information:

LSUBH3.25 and SUBH3.25-R150 (Bucket of 150), MSUBH3.25-R100 (Bucket of 100)



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

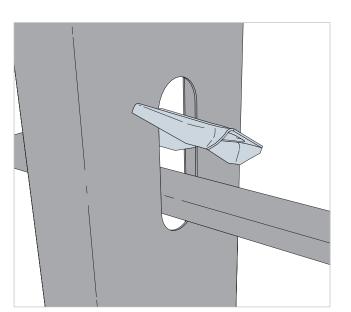




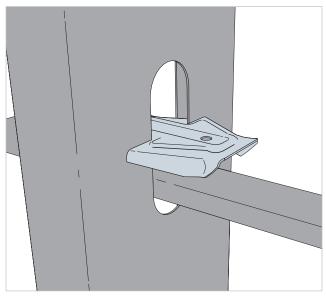
(LSUBH3.25 and MSUBH3.25 similar) U.S. Patent 8,813,456 SIMPSON

Strong

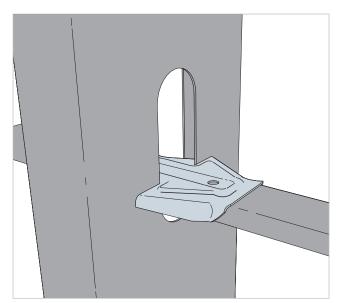
Installation Instructions



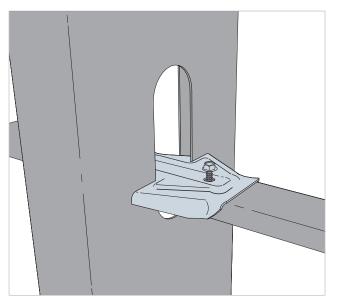
Step 1: With the u-channel in a stable, horizontal position, insert either end of the SUBH into the web knockout at approximately 45°.



Step 2: Rotate the SUBH into a horizontal position aligned with the u-channel so the slots engage the stud web.

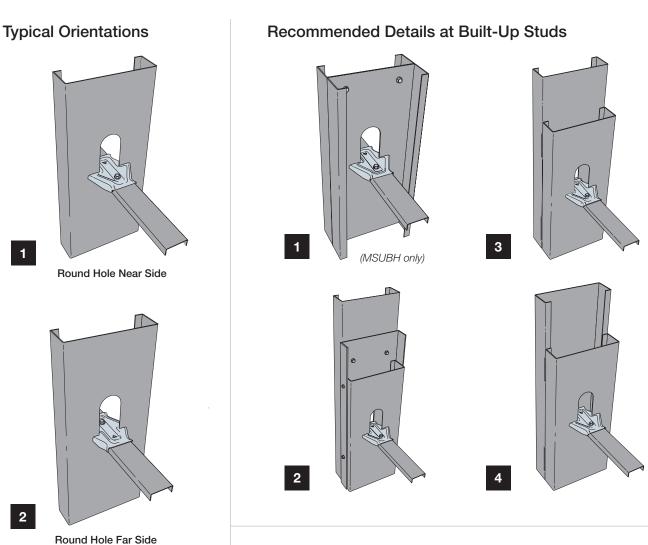


Step 3: Slide the SUBH down over the u-channel flanges, ensuring that the connector and u-channel are fully seated.
(Note: For installations at slip track, the connector may be installed inverted — see p. 117).

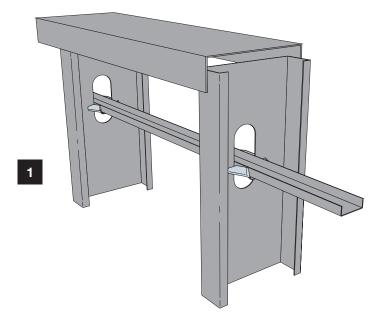


Step 4: Install the specified type and number of screws through the holes of the SUBH into the u-channel.

Installation Details

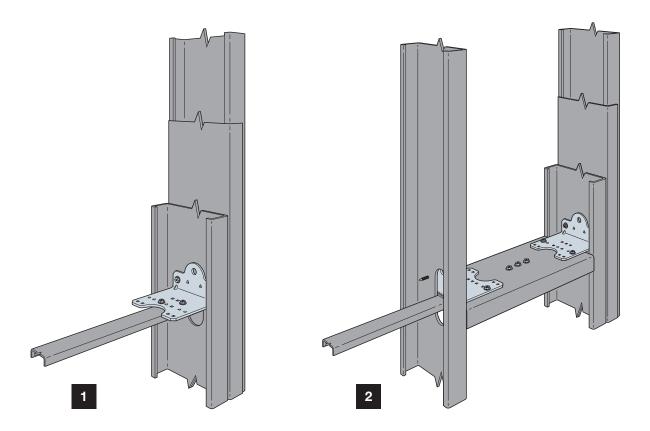


Recommended Detail at Slip Track



Alternate and Optional U-Channel Bridging Installation Details

Recommended details where knockout access is restricted, or where additional U-channel restraint is needed for load path considerations.



SIMPSON

Strong-T

SUBH Bridging Connectors

How to Use Bridging Connector Allowable Load Table

The tabulated strength and stiffness values are for use with Sections D3.2.1 and D3.3 of the 2012 edition of AISI North American Specification for the Design of Cold-Formed Steel Structural Members (AISI S100-2012) as follows:

Bracing Design for Laterally Loaded C-Studs

- Step 1: Calculate required bracing force for each flange using equation D3.2.1-3
- Step 2: Multiply result by stud depth to obtain torsional moment
- Step 3: Select connector with tabulated allowable torsional moment that exceeds torsional moment from Step 2 for the stud depth and gauge required

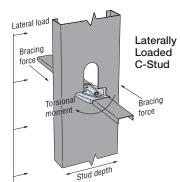
Bracing Design for Axially Loaded C-Studs

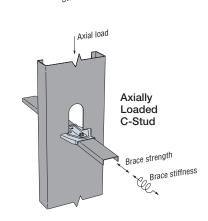
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- Step 1: Calculate required brace strength using equation D3.3-1
- Step 2: Calculate required brace stiffness using equation D3.3-2a
- Step 3: Select connector with tabulated allowable brace strength that exceeds strength from Step 1 and tabulated brace stiffness that exceeds stiffness from Step 2 for the stud depth and gauge required

										1
			Laterally Lo	aded C-Stud		Axially Loa	ded C-Stud			
Model No.	Stud Depth (in.)	Stud Thickness mils (ga.)	Allov Torsional (in.	Moment ¹	Brace St	vable trength ^{1,2} b.)		tiffness³ /in.)	Code Ref.	
			Min.	Max.	Min.	Max.	Min.	Max.		
		33 (20)	215	330	155	275	2,300	2,685		
LSUBH3.25		43 (18)	230	370	175	310	5,075	7,585	IP1	
		54 (16)	225	370	195	345	5,075	8,100		
		33 (20)	320	345	230	370	1,450	1,985		
SUBH3.25	3.625	43 (18)	355	430	255	420	2,780	4,035		
		54 (16)	420	455	290	475	2,925	3,975	IP1, L2	
		54 (16)	550	800	435	630	3,440	4,015	IF I, LZ	
MSUBH3.25		68 (14)	640	860	485	695	4,040	6,145		
		97 (12)	670	860	515	770	6,860	14265		
		33 (20)	225	330	120	140	670	730		
LSUBH3.25		43 (18)	250	395	155	285	1,010	2,075	IP1	
		54 (16)	265	395	180	330	1,025	2,565		
		33 (20)	275	385	110	110	605	605		
SUBH3.25	6.00	43 (18)	295	525	230	250	1,050	1,205		
		54 (16)	350	550	275	415	1,130	1,700	IP1, L2	
		54 (16)	565	895	385	430	1,630	1,695	11 1, LZ	
MSUBH3.25		68 (14)	655	925	455	620	1,860	2,655		
		97 (12)	690	960	505	765	4,070	4,090		
LSUBH3.25		43 (18)	235	375	135	135	815	815	IP1	
L00DH0.20		54 (16)	250	375	180	260	1,130	1,130		
SUBH3.25		43 (18)	255	570	190	190	505	535		
50DH5.25	8.00	54 (16)	325	605	250	300	895	1,025		
		54 (16)	545	890	270	270	1,025	1,045	IP1, L2	
MSUBH3.25		68 (14)	635	925	435	455	1,400	1,400		
		97 (12)	665	955	545	545	2,465	2,465		
MSUBH	10, 12	54 (16)	_	820	—	200	—	510	170	

SUBH Bridge Clip Connector - Strength and Stiffness





- 1. Allowable loads are for use when utilizing Allowable Stress Design methology. For LRFD loads multiply the ASD tabulated values by 1.6.
- 2. Allowable brace strengths are based on ultimate test load divided by a safety factor. Serviceability limit is not considered, as brace stiffness requirements are given in section D3.3 of AISI S100-2012. Contact Simpson Strong-Tie if nominal brace strength is required.
- 3. Tabulated stiffness values apply to both ASD and LRFD designs.
- Allowable loads consider bridging connection only. It is responsibility of the Designer to verify the strength and serviceability of the framing members.
- 5. Min. fastener quantity and tabulated values – fill round hole (one screw total); Max. fastener quantity and tabulated values – fill round and triangle holes (two screws total).



LSUBH - Maximum Vertical Spacing for Rows of U-Channel Bridging (ft.)

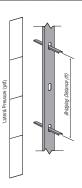
Stud		Stud									Latera	I Stud I	Pressu	re (psf)								
Spacing	Stud Section	Thickness		5	1	0	1	5	2	0	2	25	3	80	3	5	4	0	4	5	5	i0
(in.)		mil (ga.)	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.
		33 (20)	8	8	8	8	8	8	6	8	5	8	4	6	—	5	—	5	—	4	—	4
	362S162	43 (18)	8	8	8	8	8	8	7	8	5	8	4	7	4	6	_	5	—	5		4
		54 (16)	8	8	8	8	8	8	7	8	5	8	4	7	4	6	_	5	—	5	_	4
		33 (20)	8	8	8	8	6	8	5	8	4	6		5	—	4		4				—
	362S200	43 (18)	8	8	8	8	7	8	5	8	4	7		6	—	5		4	—	4		—
		54 (16)	8	8	8	8	7	8	5	8	4	7	—	6	—	5	—	4	—	4	—	—
	362S250	43 (18)	8	8	8	8	6	8	4	7		5		4		4						
		54 (16)	8	8	8	8	5	8	4	7	_	5		4	_	4	_		—	-	—	—
	0000100	33 (20)	8	8	8	8	8	8	8	8	6	8	5	8	4	6	4	6	-	5		4
	600S162	43 (18)	8	8	8	8	8	8	8	8	7	8	6	8	5	8	4	7	4	6		5
16		54 (16)	8	8	8	8	8	8	8	8	7	8	6	8	5	8 5	4	7	4	6 4	—	5
	600S200	33 (20) 43 (18)	8	8	8	8	8	8	6	8	4	8	4	7	_	6		5		4		4
	0003200	54 (16)	8	8	8	8	8	8	7	8	5	8	4	7	4	6		5		4		4
		43 (18)	8	8	8	8	7	8	5	8	4	6	4	5	4	4		4		4		4
	600S250	54 (16)	8	8	8	8	7	8	5	8	4	6		5		4		4		<u> </u>		
		43 (18)	8	8	8	8	8	8	8	8	7	8	6	8	5	8	4	7	4	6		6
	800S162	54 (16)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7	4	7	4	6
		43 (18)	8	8	8	8	8	8	7	8	5	8	4	7	4	6	_	5		5		4
	800S200	54 (16)	8	8	8	8	8	8	7	8	6	8	5	7	4	6		5		5	_	4
		43 (18)	8	8	8	8	7	8	5	8	4	7	_	5	—	5	_	4	—	—	_	
	800S250	54 (16)	8	8	8	8	8	8	6	8	4	7	4	6	—	5	_	4	—	4	—	—
	362S162	33 (20)	8	8	8	8	6	8	4	6	—	5	_	4	—	_	_	_	—	—	_	_
	362S162	43 (18)	8	8	8	8	6	8	4	7	—	6	—	5	—	4	_	_	—	-	_	_
	362S162	54 (16)	8	8	8	8	6	8	4	7	—	6	_	5	—	4	_	—	—	—	_	_
	362S200	33 (20)	8	8	6	8	4	7	—	5	—	4	—		—	—	—	—	—	—	—	—
	362S200	43 (18)	8	8	7	8	4	8		6		4		4	—		_					—
	362S200	54 (16)	8	8	7	8	4	8	—	6	—	4	—	4	—	—	—	—	—			—
	362S250	43 (18)	8	8	6	8	4	6	_	4		—	_	_	—		_		—	-	_	_
	362S250	54 (16)	8	8	5	8	—	6	_	4		—	_	—	—		_		—	_		—
	600S162	33 (20)	8	8	8	8	7	8	5	8	4	6		5	—	4		4				—
	600S162 600S162	43 (18)	8	8	8	8	8	8	6	8	4	7	4	6	—	5		4		4		—
24	0003102	54 (16)	8	8	8	8	8	8	6	8	5	7	4	6	—	5	—	4	—	4	—	—
	600S200	33 (20)	8	8	8	8	5	8	4	6		4		4						<u> </u>		
	600S200 600S200	43 (18)	8	8	8	8	6	8	4	7		5		4	—	4				-		—
		54 (16)	8	8	8	8	6	8	4	7	-	5	_	4	-	4	_		-	-	—	—
	600S250 600S250	43 (18)	8	8	7	8	4	7	—	5	—	4	—		—	—	_	—	—	-	_	—
		54 (16)	8	8	7	8	5	7	-	5		4		-			_		—		—	
	800S162 800S162	43 (18)	8	8	8	8	8	8	6 7	8	5	8	4	6		5		5 5	—	4		4
		54 (16)	8	8	8	8	8	8	4	8	5	8	4	5	4	6		5		4		4
	800S200 800S200	43 (18) 54 (16)	8	8	8	8	6	8	4	7	4	6		5		4						
		43 (18)	8	8	8	8	5	7	о 	5	4	4		<u>р</u>		4						_
	800S250 800S250	43 (16) 54 (16)	0 8	8	8	0 8	5	8	4	6		4		4								_
	5000200	54 (10)	0	0	0	0	0	0	4	0		4		4								

1. See General Notes on pp. 14-16.

2. Tabulated solutions are for ASD lateral pressure. Contact Simpson Strong-Tie for LRFD solutions.

3. Lateral pressure shall be determined based on load combinations of the applicable code. For designs in accordance with the 2009 IBC and earlier, wind pressures are at working stress level and may be used directly. For designs in accordance with the 2012 IBC and later, wind pressures are at strength level and must be multiplied by 0.6 for ASD load combinations.

4. "Min." designates a solution with the minimum number of fasteners ((1) #10 screw installed in round hole). "Max." designates a solution requiring the maximum number of fasteners ((2) #10 screws; fill both round and triangle holes). Blank areas designate conditions where the LSUBH does not offer a solution.



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SUBH — Maximum Vertical Spacing for Rows of U-Channel Bridging (ft.)

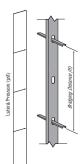
Ctud		Ctud									Latera	l Stud I	Pressui	re (psf)								
Stud Spacing	Stud Section	Stud Thickness	ę	5	1	10	1	5	2	20	2	5	3	0	3	5	4	0	4	5	5	50
(in.)	0000.000	mil (ga.)	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.
		33 (20)	8	8	8	8	8	8	8	8	8	8	6	7	5	6	5	5	4	4	4	4
	362S162	43 (18)	8	8	8	8	8	8	8	8	8	8	7	8	6	7	5	6	5	6	4	5
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	7	6	6	5	5
		33 (20)	8	8	8	8	8	8	7	8	6	6	5	5	4	4		4		—		—
	362S200	43 (18)	8	8	8	8	8	8	8	8	6	8	5	6	4	5	4	5		4		4
		54 (16)	8	8	8	8	8	8	8	8	8	8	6	7	5	6	5	5	4	4	4	4
	362S250	43 (18)	8	8	8	8	8	8	6	8	5	6	4	5		4		4		—		
		54 (16)	8	8	8	8	8	8	8	8	6	7	5	5	4	5	4	4				
	600S162	33 (20) 43 (18)	8 8	8	8	8	8	8	8	8	8	8	6 7	8	5	8 8	5 5	7	4	6 8	4	5
	0003102	54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	5	8
16		33 (20)	8	8	8	8	8	8	7	8	6	8	5	7	4	6	0	5		4		4
	600S200	43 (18)	8	8	8	8	8	8	8	8	6	8	5	8	4	8	4	7		6		5
	0000200	54 (16)	8	8	8	8	8	8	8	8	7	8	6	8	5	8	4	7	4	6		6
		43 (18)	8	8	8	8	8	8	6	8	5	8	4	7	_	6		5	_	5		4
	600S250	54 (16)	8	8	8	8	8	8	7	8	6	8	5	8	4	6		6	_	5		4
		43 (18)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	4	8	4	8
	800S162	54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	6	8	5	8
	0000000	43 (18)	8	8	8	8	8	8	7	8	6	8	5	8	4	8	_	8	—	7	—	7
	800S200	54 (16)	8	8	8	8	8	8	8	8	8	8	6	8	5	8	5	8	4	8	4	7
	800S250	43 (18)	8	8	8	8	8	8	6	8	4	8	4	8		7	_	6		6		5
	0003230	54 (16)	8	8	8	8	8	8	7	8	6	8	5	8	4	8	_	7		6		5
		33 (20)	8	8	8	8	8	8	6	7	5	5	4	4		4	_	_		—		
	362S162	43 (18)	8	8	8	8	8	8	7	8	6	7	5	6	4	5		4		4		
		54 (16)	8	8	8	8	8	8	8	8	7	7	6	6	5	5	4	4	4	4	_	
		33 (20)	8	8	8	8	6	7	5	5	4	4		—								—
	362S200	43 (18)	8	8	8	8	7	8	5	6	4	5		4								
		54 (16)	8	8	8	8	8	8	6	7	5	5	4	4	—	4		—	—	—		—
	362S250	43 (18)	8	8	8	8	6 7	7	4	5 5	-	4								—		
		54 (16) 33 (20)	8	8	8	8	8	8	5	8	4	4	4	6	_	5		4		4		_
	600S162	43 (18)	8	8	8	8	8	8	7	8	5	8	4	8	4	7		6		5		5
	0000102	54 (16)	8	8	8	8	8	8	8	8	7	8	5	8	5	7	4	6		6		5
24		33 (20)	8	8	8	8	6	8	5	7	4	5		4		4						
	600S200	43 (18)	8	8	8	8	7	8	5	8	4	7		6		5		4		4		
		54 (16)	8	8	8	8	8	8	6	8	5	8	4	6	_	5	_	5	_	4	_	4
		43 (18)	8	8	8	8	5	8	4	7		6		5	_	4		_	_			_
	600S250	54 (16)	8	8	8	8	6	8	5	8	4	6		5		4		4	_		_	_
	0000100	43 (18)	8	8	8	8	8	8	7	8	5	8	4	8	4	8		7	—	7		6
	800S162	54 (16)	8	8	8	8	8	8	8	8	7	8	6	8	5	8	4	8	4	7	_	6
	800S200	43 (18)	8	8	8	8	6	8	5	8	4	8		7	—	6	_	5	—	5		4
	0003200	54 (16)	8	8	8	8	8	8	6	8	5	8	4	8	—	7		6	—	5		5
	800S250	43 (18)	8	8	8	8	5	8	4	8	_	7	—	6	—	5	_	4		4		
	0000200	54 (16)	8	8	8	8	6	8	5	8	4	7	—	6	—	5	—	4	—	4		—

1. See General Notes on pp. 14-16.

2. Tabulated solutions are for ASD lateral pressure. Contact Simpson Strong-Tie for LRFD solutions.

3. Lateral pressure shall be determined based on load combinations of the applicable code. For designs in accordance with the 2009 IBC and earlier, wind pressures are at working stress level and may be used directly. For designs in accordance with the 2012 IBC and later, wind pressures are at strength level and must be multiplied by 0.6 for ASD load combinations.

4. "Min." designates a solution with the minimum number of fasteners ((1) #10 screw installed in round hole). "Max." designates a solution requiring the maximum number of fasteners ((2) #10 screws; fill both round and triangle holes). Blank areas designate conditions where the SUBH does not offer a solution.





MSUBH — Maximum Vertical Spacing for Rows of U-Channel Bridging (ft.)

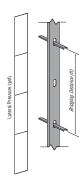
04.14		Ohud									Latera	l Stud F	Pressur	e (psf)								
Stud Spacing	Stud Section	Stud Thickness	5	5	1	0	1	5	2	0	2	5	3	0	3	5	4	0	4	5	5	0
(in.)	0001011	mil (ga.)	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	7	8
	362S162	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	6	8	5	7
	362S200	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	7	8	6	8
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8
		54 (16)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7	4	7	4	6
	362S250	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7	5	6
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	6	7	5	6
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
	600S162	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	7	8	6	8
	600S200	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
	0000050	54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	5	7
	600S250	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8
		97 (12)	8	8 8	8	8	8 8	8	8 8	8 8	8 8	<mark>8</mark>	8 8	8 8	<mark>8</mark> 8	8 8	7	8	6	8	6	8
	800S162	54 (16)	0 8	0 8	0 8	8	0 8	о 8	0 8	0 8	0 8	0 8	0 8	0 8	0 8	0 8	8	8	8 8	8	8 8	0 8
	0003102	68 (14) 97 (12)	0 8	0 8	0 8	8	0 8	0 8	0 8	0 8	0 8	0 8	0 8	0 8	0 8	0 8	8	0 8	0 8	0 8	0 8	8
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8
16	800S200	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8
10	0000200	97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	5	8
	800S250	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	6	8
	0000200	97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8
		54 (16)		8	_	8	_	8	_	8		8	_	8	_	8	_	8	_	8	_	8
	1000S162	68 (14)		8		8		8		8		8		8		8		8		8		8
		97 (12)		8	_	8		8		8		8		8		8		8		8		8
		54 (16)	_	8	_	8	—	8		8		8		8	—	8	_	8		8	_	8
	1000S200	68 (14)	_	8	_	8		8		8		8		8	_	8	_	8		8		8
		97 (12)		8	_	8		8		8		8		8		8	_	8		8		8
		54 (16)	_	8	—	8	—	8		8	_	8	_	8	—	8	—	8	_	8	_	8
	1000S250	68 (14)		8	—	8	_	8		8		8		8		8	_	8		8	_	8
		97 (12)		8	_	8		8		8		8		8		8		8		8		8
		54 (16)		8	—	8	_	8	_	8		8	_	8		8	—	8		8	_	8
	1200S162	68 (14)	_	8	_	8	_	8	_	8	_	8	_	8	_	8	_	8	_	8	_	8
		97 (12)	—	8	—	8	_	8	_	8	_	8	_	8	_	8	—	8	_	8	—	8
		54 (16)	_	8	—	8	_	8	_	8	_	8	_	8	_	8	—	8	_	8	—	8
	1200S200	68 (14)	_	8	—	8		8		8		8		8	_	8	—	8	_	8		8
		97 (12)	—	8	—	8		8		8		8		8	—	8	—	8		8		8
		54 (16)	—	8	—	8	—	8		8		8		8	—	8	—	8		8	—	8
	1200S250	68 (14)	—	8	—	8	—	8		8	_	8	_	8	—	8	_	8		8	—	8
		97 (12)	_	8	—	8	_	8	—	8	_	8	—	8	—	8	_	8	—	8	_	8

1. See General Notes on pp. 14-16.

2. Tabulated solutions are for ASD lateral pressure. Contact Simpson Strong-Tie for LRFD solutions.

3. Lateral pressure shall be determined based on load combinations of the applicable code. For designs in accordance with the 2009 IBC and earlier, wind pressures are at working stress level and may be used directly. For designs in accordance with the 2012 IBC and later, wind pressures are at strength level and must be multiplied by 0.6 for ASD load combinations.

4. "Min." designates a solution with the minimum number of fasteners ((1) #10 screw installed in round hole). "Max." designates a solution requiring the maximum number of fasteners ((2) #10 screws; fill both round and triangle holes). Blank areas designate conditions where the MSUBH does not offer a solution.





MSUBH — Maximum Vertical Spacing for Rows of U-Channel Bridging (ft.) (cont.)

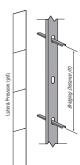
Stud		Stud									Latera	I Stud I	Pressu	re (psf)								
Spacing	Stud Section	Thickness		5	1	0	1	5	2	20	2	25	3	80	3	5	4	0	4	5	5	50
(in.)		mil (ga.)	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.
		54 (16)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	5	7	4	6
	362S162	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	6	8	5	7
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7
		54 (16)	8	8	8	8	8	8	8	8	7	8	6	8	5	7	4	6	4	5		5
	362S200	68 (14)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7	4	6	4	5
		97 (12)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7	5	6	4	5
		54 (16)	8	8	8	8	8	8	7	8	5	8	4	7	4	6		5	—	4		4
	362S250	68 (14)	8	8	8	8	8	8	8	8	6	8	5	7	4	6	4	5	-	5		4
		97 (12)	8	8	8	8	8	8	8	8	7	8	6	7	5	6	4	5	4	5	_	4
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8
	600S162	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8
	0000000	54 (16)	8	8	8	8	8	8	8	8	8	8	7	8	6 7	8 8	5	8	4	7	4	6 7
	600S200	68 (14)	8		8	-					8	8	8	8	7					-	4	7
		97 (12) 54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	4	8	6	8	5	8	5	5
	600S250	68 (14)	0 8	8	8	8	0 8	0 8	0 8	8	7	8	6	8	4 5	7	4	6	4	6		5
	0003230	97 (12)	8	8	8	8	8	8	8	8	8	8	6	8	5	8	5	7	4	6	4	5
		54 (16)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	6	8
	800S162	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8
	0000102	97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8
		54 (16)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	5	8	4	7
24	800S200	68 (14)	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8	5	7
		97 (12)	8	8	8	8	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	8
		54 (16)	8	8	8	8	8	8	8	8	7	8	5	8	5	8	4	7	—	6	—	5
	800S250	68 (14)	8	8	8	8	8	8	8	8	8	8	6	8	5	8	5	7	4	6	4	6
		97 (12)	8	8	8	8	8	8	8	8	8	8	7	8	6	8	5	7	4	7	4	6
		54 (16)	—	8	—	8		8	—	8	—	8	—	8		8	—	8	—	8	—	8
	1000S162	68 (14)	—	8	—	8	—	8	—	8	—	8	—	8		8	—	8	—	8	—	8
		97 (12)		8	—	8		8	—	8	—	8	—	8	—	8	—	8	—	8	_	8
		54 (16)		8		8		8	_	8		8		8		8		8		8		7
	1000S200	68 (14)		8		8		8		8		8		8	—	8		8		8		7
		97 (12)	_	8		8	_	8	—	8	_	8	—	8		8	—	8	—	8	_	7
		54 (16)		8		8		8		8		8		8		8		7		6		5
	1000S250	68 (14)		8	<u> </u>	8		8		8		8		8		8		7		6		5
		97 (12)	—	8	—	8		8	—	8	—	8	—	8		8		7	—	6		5
	10000100	54 (16)		8		8		8	—	8		8		8	—	8		8	—	8		8
	1200S162	68 (14)		8		8	<u> </u>	8	-	8		8		8		8	—	8		8	<u> </u>	8
		97 (12)	—	8		8		8	—	8	—	8		8		8	—	8	—	8		8
	12000200	54 (16)		8		8		8 8		8		8		8		8 8		8	—	8		8
	1200S200	68 (14) 97 (12)		8		8		8		8		8		8		8		8		8		8
		54 (16)		8	-	8		8		8		8		8		8		7		6		6
	1200S250	68 (14)		8		8		8		8		8		8		8		7		6		6
	12000200	97 (12)	_	8	-	8		8	_	8		8	_	8		8		7	_	7		6
		51 (12)			1				1		1					0		'	1	1 '		

1. See General Notes on pp. 14-16.

2. Tabulated solutions are for ASD lateral pressure. Contact Simpson Strong-Tie for LRFD solutions.

3. Lateral pressure shall be determined based on load combinations of the applicable code. For designs in accordance with the 2009 IBC and earlier, wind pressures are at working stress level and may be used directly. For designs in accordance with the 2012 IBC and later, wind pressures are at strength level and must be multiplied by 0.6 for ASD load combinations.

4. "Min." designates a solution with the minimum number of fasteners ((1) #10 screw installed in round hole). "Max." designates a solution requiring the maximum number of fasteners ((2) #10 screws; fill both round and triangle holes). Blank areas designate conditions where the MSUBH does not offer a solution.





Example #1: Curtain-Wall Stud

Given

Bridging and Bracing Connectors

- 2015 IBC (ASCE 7-10 and AISI S100-2012)
- 600S162–43 (33 ksi) studs at 24" o.c.
- 10'-tall studs with mid-point bracing (5' o.c.)
- Wind design pressure = 41 psf

Select Connector Using Design Table (p. 121)

ASD wind pressure:

p = (0.6)(41 psf) = 24.6 psf

Note: 2015 IBC load combinations for ASD include a factor of 0.6 for wind loads.

For 600S162-43 stud with SUBH3.25 connector, and 25 psf wind pressure with 5' bracing distance:

SUBH3.25 with Min. fasteners OK

Notes

- 1. Only lateral load has been included for clarity. Design of curtain-wall studs should consider load combinations with vertical load in accordance with the applicable building code (see Example #2).
- 2. Bridging connector may also be designed using Allowable Loads table on p. 119 (see Example #2).

Example #2: Exterior Bearing-Wall Stud

Given

- 2015 IBC (ASCE 7-10 and AISI S100-2012)
- 600S162-54 (50 ksi) studs at 24" o.c., 10' tall

Mid-point bracing (5' o.c.) Required axial stud strength, P_{ra} = 2,200 lb. Distance from shear center to mid-plane of web, m = 0.663" (2013 AISI Manual, Table I-2)

Wind design pressure = 34 psf

Axially-Loaded Stud Design

Required brace strength (AISI S100 Eq. D3.3-1):

 $P_{br,1} = 0.01P_{ra} = (0.01)(2,200 \text{ lb.}) = 22 \text{ lb.}$

Required brace stiffness (AISI S100 Eq. D3.3-2a):

 $\beta_{rb} = \{2[4 - (2/n)]/Lb\}\Omega P_{ra} = \{2[4 - (2/1)]/60 \text{ in.}\}(2)(2,200) = 294 \text{ lb./in.}\}$

From Allowable Loads table (p. 119) for 6"-deep 54-mil stud:

- Select SUBH3.25 with Min. fasteners
 - Allowable brace strength = 275 lb. > 22 lb. OK
 - Brace stiffness = 1,130 lb./in. > 294 lb./in. OK

Laterally-Loaded Stud Design

Design load tributary to a single connector:

W = (0.6)(34 psf)(2 ft.)(5 ft.) = 204 lb.

Note: 2015 IBC load combinations for ASD include a factor of 0.6 for wind loads.

Required flange force (AISI S100 Eq. D3.2.1-3):

 $P_{L1} = -P_{L2} = 1.5(m/d)W = (1.5)(0.663 \text{ in.}/6 \text{ in.})(204 \text{ lb.}) = 33.8 \text{ lb.}$

Torsional moment:

 $M_Z = P_{L1}d = -P_{L2}d = (33.8 \text{ lb.})(6 \text{ in.}) = 203 \text{ in.-lb.}$

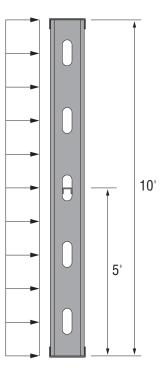
From Allowable Loads table (p. 119) for 6"-deep 54-mil stud:

Select SUBH3.25 with Min. fasteners

Allowable torsional moment = 350 in.-lb. > 203 in.-lb. OK

Combined-Loading Check

 $(P_{br,1}/Allowable brace strength) + (M_z/Allowable torsional moment) = (22 lb./275 lb.) + (203 in.-lb./350 in.-lb.) = 0.66 < 1.0 \text{ OK}$





This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

Simpson Strong-Tie introduces the SBR and DBR spacer bracers for cold-formed steel construction. These spacer bracers reduce the installed cost of cold-formed steel stud walls by enabling faster stud layout while minimizing the need for bridging clips.

The DBR is used for interior walls to eliminate stud bow and allow for quicker drywall attachment, while the SBR is designed for structural exterior walls. Both products provide bracing along the length of the stud, and for head-of-wall slip conditions. The SBR and DBR also come with prepunched slots that eliminate the need to use bridging clips with on-module studs.

The SBR and DBR spacer bracers come with bracing load data based on assembly testing, thus mitigating risk for designers and maximizing confidence in design specs. In fact, the SBR and DBR are the only spacer bracers on the market with tabulated design values based on assembly tests.

Features:

- SBR and DBR have patent-pending precisionengineered prepunched slots strategically located to enable 12", 16" and 24" on-center stud spacing and can be used to space the studs without having to mark the top track for layout
- The SBR will accommodate 3%" and 6" studs in thicknesses of 33 mil (20 ga.) thru 68 mil (14 ga.)
- The DBR will accommodate 2½", 3%" and 6" studs in thicknesses of 15 mil (25 ga. EQ) through 33 mil (20 ga.)
- Prepunched holes in the SBR provide rapid screw installation when spacer-bracer splices are needed for axial load-bearing studs
- In off-layout or end-of run conditions, the hat-section profiles enable clip attachments to the stud with Simpson Strong-Tie[®] LSSC or RCA connectors

Installation:

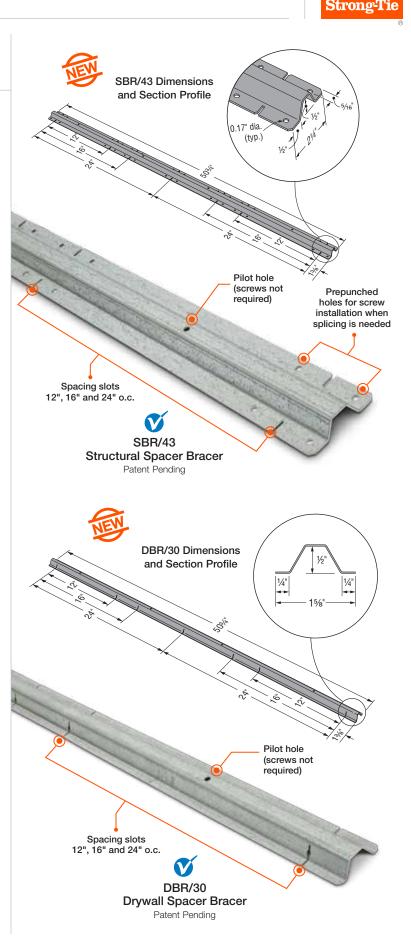
- Spacer bracers are fed through the stud knockout at a 90° angle until studs align with spacer-bracer slots. With the slots engaging the stud web, the spacer-bracer is then rotated back to the flat position so that the slotted flanges are on the bottom.
- For off-layout or end-of-run studs where a spacerbracer slot does not engage a stud, manually snip the spacer-bracer flanges with a ½"-deep slot and secure the spacer bracer to the stud with Simpson Strong-Tie LSSC or RCA connectors. Use all specified fasteners.
- Wear gloves while handling and installing spacer bracers.

Material: SBR/43 — 43 mil (40 ksi); DBR/30 — 27 mil (33 ksi)

Finish: Galvanized (G90)

Codes: See p. 11 for Code Reference Key Chart

Ordering Information: SBR/43-R20 (Box of 20) DBR/30-R20 (Box of 20)



SIMPSON

SBR and DBR Spacer Bracer — Connection Strength and Stiffness

Model No.	Stud Depth (in.)	Stud Thickness mil (ga.)	Allowable Torsional Moment (in./lb.)	Allowable Brace Strength (lb.)	Brace Stiffness (lb./in.)	Code Ref.
		33 (20)	235	390	845	
	05/	43 (18)	310	435	1,390	
	35%	54 (16)	400	435	1,390	
SBR/43		68 (14)	400	435	1,390	
3BH/43		33 (20)	215	160	495	
	6	43 (18)	310	330	765	
	0	54 (16)	365	450	840	
		68 (14)	365	450	840	
		15 (25 EQ)	55	—	_	170
		18 (25)	55	—	_	170
	21⁄2	19 (20 EQ)	60	—	_	
		30 (20 DW)	85	—	—	
DBR/30		33 (20 STR)	90	—	_	
DDU/30		15 (25 EQ)	55	—	—	
		18 (25)	55	_	_	
	6	19 (20 EQ)	60	—	—	
		30 (20 DW)	85	_	—	
		33 (20 STR)	90	—	—	

1. Allowable loads are for use when utilizing the traditional Allowable Stress Design methodology. For LRFD loads multiply the ASD tabulated values by 1.6.

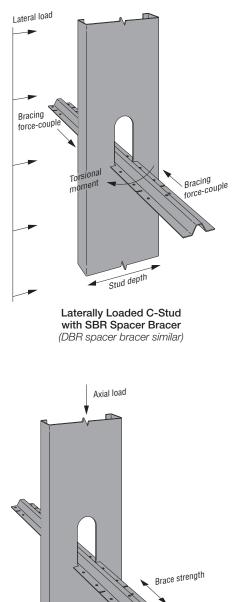
 Tabulated Allowable Brace Strengths are based on ultimate test load divided by a safety factor. Serviceability limit is not considered, as brace stiffness requirements are given in section D3.3 of AISI S100-2012.

3. Tabulated Brace Stiffness values apply to both ASD and LRFD designs.

4. Allowable loads consider bridging connection only. It is the responsibility of the

Designer to verify the strength and serviceability of the framing members.

5. EQ - equivalent, DW - drywall, STR - structural.



Brace stiffness

Axially Loaded C-Stud with SBR Spacer Bracer

Bridging and Bracing Connectors

SIMPSON Strong-Tie

SBR and DBR Gross Properties

Model	Design	Fv	Area ²	I _X ⁴	S _x ³	Rx	lv⁴	S _v ³	Rv		Tors				
No.	Thickness (in.)	(ksi)	(in.)	(in.)	(in.)	(in.)	(in.)	(in.)	(in.)	Jx1,000⁴ (in.)	Cw ⁶ (in.)			R _o (in.)	β
SBR/43	0.0468	40	0.126	0.0047	0.1458	0.1936	0.0436	0.0400	0.5891	0.0916	5.56E-04	0.283	0.017	0.681	0.828
DBR/30	0.0289	33	0.060	0.0023	0.0082	0.1936	0.0109	0.0141	0.4259	0.0167	7.05E-05	0.346	0.087	0.582	0.647

SBR and DBR Net Properties

Model	Area ²	lx ⁴	Sx ³	Rx	lv ⁴	Sv ³	Rv			Torsional			
No.	(in.)	(in.)	(in.)	(in.)	(in.)	(in.)	(in.)	Jx1,000 ⁴ (in.)	Cw ⁶ (in.)				β
SBR/43	0.085	0.0028	0.0097	0.1816	0.0120	0.0184	0.3765	0.0617	3.43E-05	0.355	0.141	0.548	0.581
DBR/30	0.022	0.0001	0.0004	0.0479	0.0008	0.0027	0.1944	0.0061	1.09E-06	0.086	0.051	0.218	0.844

SBR and DBR Allowable Member Strengths

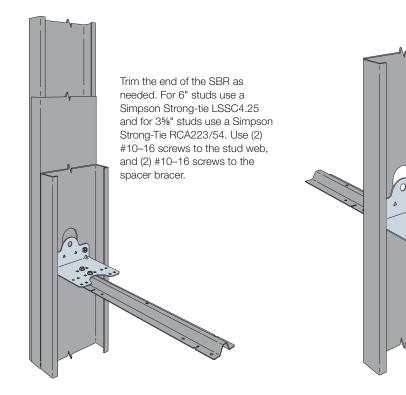
Model No.	M _a (F _y) (inIb.)	M _a (12" o.c.) (inlb.)	Ma (16" o.c.) (inlb.)	Ma (24" o.c.) (inlb.)	Pa (12" o.c.) (lb.)	Pa (16" o.c.) (lb.)	Pa (24" o.c.) (lb.)
SBR/43	369	369	369	360	945	904	618
DBR/30	44	40	38	32			_

1. Net section properties are based a section that excludes all material that is interrupted by the slots.

2. Member strengths are based on DSM Analysis (non-prequalified section, $\Omega = 2.0$).

 $3. C_b = 1.67$ has been applied to M_a to account for a triangular moment diagram with zero end moment.

Bridging and Bracing Connectors

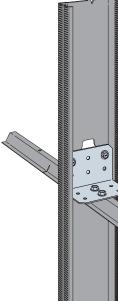


SBR End-of-Run for 6" Studs with LSSC4.25 (3%" studs with RCA 223/54 similar) When the SBR slots do not line-up with a stud, field-snip a ½"-deep slot in each flange of the SBR. For 6" studs use a Simpson Strong-Tie LSSC4.25. For 3%" studs use a Simpson Strong-Tie RCA223/54. Use (2) #10–16 screws to the stud web, and (2) #10–16 screws to the spacer bracer.

Note: Field-snipped slots shall be at least 4" away from other prepunched slots, and shall be spaced at least 12" o.c. from other field-snipped slots.

SBR Off-Module for 6" Studs with LSSC4.25 (3%" studs with RCA 223/54 similar)

Trim the end of the DBR as needed. For 6" studs use a Simpson Strong-Tie LSSC4.25. For 35%" studs use a Simpson Strong-Tie RCA223/54. Use (2) #10–16 screws to the stud web and (2) #10–16 screws to the spacer bracer.

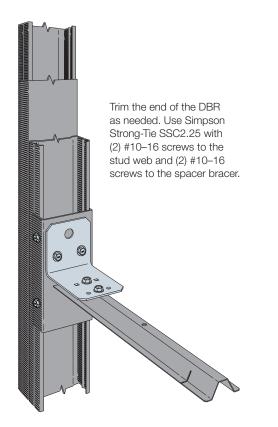


When the DBR slots do not line up with a stud, field-snip a ½"-deep slot in each side of the DBR. For 6" studs, use a Simpson Strong-Tie LSSC4.25. For 3%" studs, use a Simpson Strong-Tie RCA223/54 with (2) #10–16 screws to the stud web and (2) #10–16 screws to the spacer bracer.

Note: Field-snipped slots shall be at least 4" away from other prepunched slots, and shall be spaced at least 12" o.c. from other field-snipped slots. C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

DBR End-of-Run for 3%" Studs with RCA223/54 (6" studs with LSSC4.25 similar) DBR Off-Module for 3⁵/⁸ Studs with RCA223/54 (6" studs with LSSC4.25 similar)

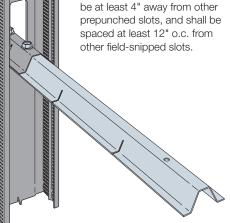




DBR End-of-Run for 2.5" Studs with SSC2.25

Note: Field-snipped slots shall

When the DBR slots do not line up with a stud web, field-snip a 1/2"-deep slot in each DBR flange, and use a 3" long section of DBR reinforcement as shown. Secure the DBR reinforcement with (2) #10-16 screws so that the screw spacing is 11/2".



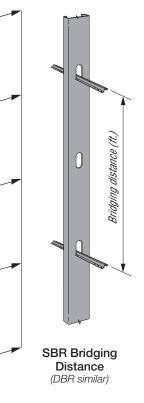
DBR Off-Module for 21/2" Studs with DBR Reinforcement (DBR and SBR with 3%" studs and 6" studs similar)



Typical SBR Splice for Axially Loaded Studs

SBR/43 Maximum Bridging Distance (ft.)

Stud	.	Stud		-		Latera		Pressur	e (psf)			
Spacing (in.)	Stud Section	Thickness mil (ga.)	5	10	15	20	25	30	35	40	45	50
		33 (20)	8	8	8	8	7	6	5	4	4	_
		43 (18)	8	8	8	8	8	8	7	6	5	5
	362S162	54 (16)	8	8	8	8	8	8	8	7	7	6
		68 (14)	8	8	8	8	8	8	8	8	7	6
		33 (20)	8	8	8	7	6	5	4	—	—	_
		43 (18)	8	8	8	8	8	6	5	5	4	4
	362S200	54 (16)	8	8	8	8	8	8	6	6	5	4
10		68 (14)	8	8	8	8	8	8	6	6	5	4
12		33 (20)	8	8	8	8	8	7	6	5	4	4
		43 (18)	8	8	8	8	8	8	8	7	6	6
	600S162	54 (16)	8	8	8	8	8	8	8	8	8	7
		68 (14)	8	8	8	8	8	8	8	8	8	7
		33 (20)	8	8	8	7	6	5	4	—	_	_
	0000000	43 (18)	8	8	8	8	8	7	6	5	5	4
	600S200	54 (16)	8	8	8	8	8	8	7	6	6	5
		68 (14)	8	8	8	8	8	8	7	6	6	5
		33 (20)	8	8	8	7	5	4	4	—	—	—
	0000100	43 (18)	8	8	8	8	7	6	5	4	4	—
	362S162	54 (16)	8	8	8	8	8	7	6	5	5	4
		68 (14)	8	8	8	8	8	8	6	6	5	4
		33 (20)	8	8	7	5	4	_	_	—	_	_
		43 (18)	8	8	8	7	6	5	4	—	—	_
	362S200	54 (16)	8	8	8	8	7	6	5	4	4	_
10		68 (14)	8	8	8	8	7	6	5	4	4	_
16		33 (20)	8	8	8	7	6	5	4	—	_	_
	0000100	43 (18)	8	8	8	8	8	7	6	5	5	4
	600S162	54 (16)	8	8	8	8	8	8	7	6	6	5
		68 (14)	8	8	8	8	8	8	7	6	6	5
		33 (20)	8	8	7	5	4	—	_	—	—	—
	0000000	43 (18)	8	8	8	8	6	5	4	4	_	_
	600S200	54 (16)	8	8	8	8	8	6	5	5	4	4
		68 (14)	8	8	8	8	8	6	5	5	4	4
		33 (20)	8	8	6	4	_	_	_	_	—	_
	2600100	43 (18)	8	8	8	6	5	4	_	_	—	
	362S162	54 (16)	8	8	8	7	6	5	4	—	_	—
		68 (14)	8	8	8	7	6	5	4	_		_
		33 (20)	8	7	5	—	_	_	—	—	—	—
	2620200	43 (18)	8	8	6	5	4	_		—	_	—
	362S200	54 (16)	8	8	7	5	4	_		—	_	_
24		68 (14)	8	8	7	5	4	—	—	—	—	—
24		33 (20)	8	8	7	5	4		_	—		—
	6000100	43 (18)	8	8	8	7	6	5	4	—	—	—
	600S162	54 (16)	8	8	8	8	7	6	5	4	4	—
		68 (14)	8	8	8	8	7	6	5	4	4	_
		33 (20)	8	7	5	—	_	—	—	—	—	_
	6000000	43 (18)	8	8	7	5	4			_		_
	600S200	54 (16)	8	8	8	6	5	4	—	—	-	_
		68 (14)	8	8	8	6	5	4	_	—	_	_



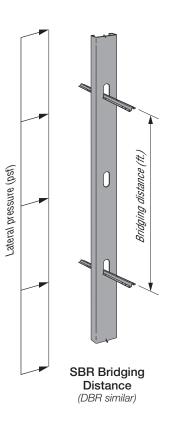
Lateral pressure (psf)

- 1. Tabulated solutions are for ASD lateral pressure. Contact Simpson Strong-Tie for LRFD solutions.
- 2. Lateral pressures shall be determined based on the load combinations of the applicable building code. For designs in accordance with the 2009 IBC and earlier, wind pressures are at the working stress level and may be used directly. For designs in accordance with the 2012 and 2015 IBC, wind pressures are at the strength level and must be multiplied by 0.6 for ASD load combinations.
- Tabulated values are based on the minimum of the tested connection strength and the calculated SBR/DBR member strength. Studs must be checked separately for unbraced length.



DBR/30 Maximum Bridging Distance (ft.)

Stud Spacing (in.)	Stud Section	Stud Thickness mils (ga.)		ld Pressure sf)
			5	10
		15 (25 EQ)	8	5
		18 (25)	8	5
	362S125	19 (20 EQ)	8	5
		30 (20 DW)	8	5
12		33 (20 STR)	8	5
12		15 (25 EQ)	8	6
		18 (25)	8	6
	600S125	19 (20 EQ)	8	6
		30 (20 DW)	8	6
		33 (20 STR)	8	6
		15 (25 EQ)	7	_
		18 (25)	7	_
	362S125	19 (20 EQ)	7	_
		30 (20 DW)	7	_
16		33 (20 STR)	7	—
10		15 (25 EQ)	8	4
		18 (25)	8	4
	600S125	19 (20 EQ)	8	4
		30 (20 DW)	8	4
		33 (20 STR)	8	4
		15 (25 EQ)	4	_
		18 (25)	4	_
	362S125	19 (20 EQ)	4	_
		30 (20 DW)	4	—
24		33 (20 STR)	4	—
24		15 (25 EQ)	4	_
		18 (25)	4	_
	600S125	19 (20 EQ)	4	
		30 (20 DW)	5	—
		33 (20 STR)	5	_



1. Tabulated solutions are for ASD lateral pressure. Contact Simpson Strong-Tie for LRFD solutions.

- 2. Lateral pressures shall be determined based on the load combinations of the applicable building code. For designs in accordance with the 2009 IBC and earlier, wind pressures are at the working stress level and may be used directly. For designs in accordance with the 2012 and 2015 IBC, wind pressures are at the strength level and must be multiplied by 0.6 for ASD load combinations.
- Tabulated values are based on the minimum of the tested connection strength and the calculated SBR/DBR member strength.
 Studs must be checked separately for unbraced length.

SBR Spacer Bracer

Given

- 2015 IBC (ASCE 7-10 and AISI S100-2012)
- 600S162-54 (50 ksi) studs at 24" o.c., 10'-stud height
 - Mid-point bracing (5' o.c.)
 - Distance from shear center to mid-plane of web, m = 0.663". (2013 AISI Manual, Table I-2)
- Wind design pressure = 34 psf
- Pra = Required ASD axial load = 3,000 lb.

Axially Loaded Stud

Required brace strength (AISI S100, Eq. D3.3-1) $P_{rb} = 0.01P_{ra} = (0.01)(3,000 \text{ lb.}) = 30 \text{ lb.}$

Required brace stiffness (AISI S100, Eq. D3.3-2a) $\beta_{rb} = (2[4-(2/n)]/L_b)(\Omega P_{ra}) = (2[4-(2/n)]/60)(2)(3,000) = 400 \text{ lb./in.}$

Check connection strength and stiffness from Strength and Stiffness table (page 4) for the SBR/43 for 6"-deep, 54-mil studs

- Allowable brace strength = 450 lb. > 30 lb. OK
- Allowable brace stiffness = 840 lb./in. > 400 lb./in. OK

Check member strength from Allowable Strengths table (page 4) for the SBR/43 for 24" o.c.

> Pa (24" o.c.) = Allowable member strength = 618 lb. > 30 lb. OK

Note: Member stiffness and the effects of accumulated load for multiple axially loaded studs have not been accounted for in the above calculations. Reference CFSEI Tech Note W400-16 for additional guidance on these topics.

Laterally Loaded Stud

ASD Design load tributary to brace: W = (0.6)(34 psf)(2 ft.)(5 ft.) = 204 lb.

> Note: 2015 IBC load combinations for ASD include a factor of 0.6

Required flange force (AISI S100 Eq. D3.2.1-3) $P_{L1} = -P_{L2} = 1.5(m/d)W = (1.5)(0.663 \text{ in.}/6 \text{ in.})(204 \text{ lb.}) = 33.8 \text{ lb.}$

Torsional moment

 $M_Z = P_{L1}d = -P_{L2}d = (33.8)(6) = 202.8$ in.-lb.

Moment applied to bridging member

 $M_m = 0.64M_z = (0.64)(202.8) = 129.8$ in.-lb.

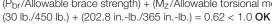
Note: The 0.64 factor is from an analysis of a five-span continuous beam that is loaded with equal support moments (Reference AIS) Design Guide D110-07, pp. 2-9, Figure 2-6)

Check connection strength from Strength and Stiffness table (p. 126) for the SBR/43 for 6"-deep, 54-mil studs

Allowable torsional moment = 365 in.-lb. > 202.8 in.-lb. OK

Check member strength from Allowable Strengths table (p. 126) for the SBR/43 for 24" o.c.

> Ma (24" o.c.) = Allowable moment = 360 in.-lb. > 129.8 in.-lb. OK



Reference AISI Eqs. C5.2.1-1, C5.2.1-2, or Eq. C52.1-3 as applicable. For this condition, Eq. C5.2.1-3 applies.

$$\frac{\Omega_c P}{P_n} + \frac{\Omega_b M}{M_n} \le 1.0$$

$$P_n = 2P_a$$
 $M_n = 2M_a$

$$\frac{1.8 (30)}{2 (618)} + \frac{1.67 (129.8)}{2 (360)} = 0.34 < 1.0 \text{ OK}$$

Note: The allowable strengths given in the Allowable Strengths table (p. 126) have been converted to nominal strengths by multiplying by $\Omega = 2.0$.

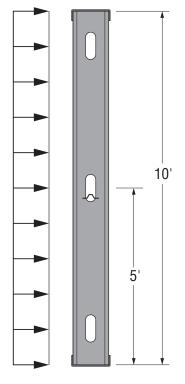
Combined-Loading Check of Connection (P_{br}/Allowable brace strength) + (M_z/Allowable torsional moment) \leq 1.0

Strong

Combined-Loading Check of Bridging Member

$$+$$
 $+$ $\frac{\Omega_b M}{M_n} \le 1.0$

$$P_n = 2P_a \qquad M_n = 2M_a$$



Bridging and Bracing Connectors

SFC Steel Framing Connectors / SSC Steel-Stud Connectors

SFC/SSC Connectors – U-Channel Bridging Allowable Loads

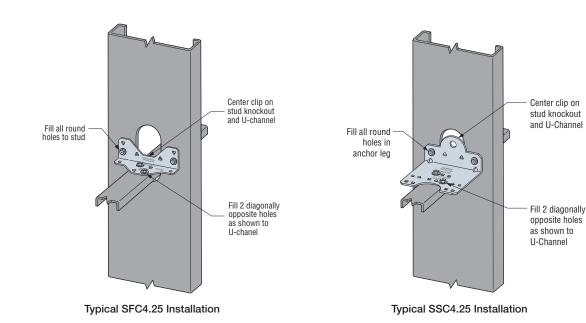
	Connector		Churd	Churd	Stud Bridging Allowable Torsional Moment ² (inlb.) Allowable Brac Strength ^{2.3} (lb.) (2) #10 (2) #10 275 125 (2) #10 (2) #10 510 190 (2) #10 (2) #10 510 190 (2) #10 (2) #10 645 280 (2) #10 (2) #10 1,085 180 (2) #10 (2) #10 655 280 (2) #10 (2) #10 805 335 (2) #10 (2) #10 920 660	Axially Loa	ded C-Stud			
Model No.	Material Thickness mil (ga.)	L (in.)	Stud Depth (in.)	Stud Thickness mil (ga.)	Stud	Bridging	Torsional Moment ²		Brace Stiffness⁴ (Ib./in.)	Code Ref.
				33 (20)	(2) #10	(2) #10	275	125	860	
SFC4.25	54 (16)	4¼	6	43 (18)	(2) #10	(2) #10	510	190	1,220	
				54 (16)	(2) #10	(2) #10	645	280	2,045	
LSSC4	54 (16)	41⁄4	6	54 (16)	(2) #10	(2) #10	1,085	180	165	IP2
				54 (16)	(2) #10	(2) #10	655	280	2,045	IF2
SSC4.25	68 (14)	4¼	6	68 (14)	(2) #10	(2) #10	805	335	2,305	
				97 (12)	(2) #10	(2) #10	920	660	4,230	
LSSC6.25	54 (16)	6¼	8, 10, 12	54 (16)	(2) #10	(2) #10	1,085	180	685	

1. See illustrations for fastener placement.

Allowable loads are for use when utilizing Allowable Stress Design methodology. For LRFD loads, multiply the tabulated ASD values by 1.6.
 Allowable brace strengths are based on ultimate test load divided by a safety factor. Serviceability limit is not considered, as brace stiffness requirements are given in Section D3.3 of AISI S100. Contact Simpson Strong-Tie if nominal brace strength is required.

4. Tabulated stiffness values apply to both ASD and LRFD designs.

5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



SIMPSON

Strong-I

DBC Drywall Bridging Connector



Work smarter, not harder

Patented design allows for one- or two-screw installation of the DBC, significantly reducing labor and material cost. The first and only connector load rated for ¾" u-channel, the DBC joins the SUBH line of bridging connectors tested as a system, ensuring that published design capacities capture the influence of stud web depth and thickness.

Features:

- Most applications require only a single screw
- Designed for ¾" u-channel to fit smaller web knockouts common to drywall studs
- Compatible with drywall stud depths of 3%" and 6" with 11/2" wide knockouts



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.





U.S. Patents 8,813,456 and 8,590,255 Material: 33 mil (20 ga.) carbon steel Finish: Galvanized (G90)

Installation:

- With ¾" x 54 mil (16 ga.) u-channel installed through the stud web knockouts, insert the DBC2.5 through the knockout so that the DBC slots engage the stud web and the DBC flanges engage the u-channel as shown in the illustration
- Use the specified number of #8 screws to fasten the DBC to the u-channel

Codes: See p. 11 for Code Reference Key Chart

Ordering Information: DBC2.5-R200 (Bucket of 200)

DBC Drywall Bridging Connector

DBC — Bridging Connector Strength Allowable Loads

Model	Stud		d Thickness 'ield Strengt		Faste	ners	Laterally Loaded C-Stud Allowable Torsional	Code
No.	Depth	Mil	Gauge ³	Fy (ksi)	Tust		Moment (in lb.)	Ref.
		15	25 EQ.	50				
		18	25	33	Min.	(1) #8	65	
		19	20 EQ.	65	IVIIII.	(1)#0	00	
	3%	20	20 EQ.	57				
	578	30	20 DW	33	Min.	(1) #8	85	
			20.011		Max.	(2) #8	125	
		33	20 STR	33	Min.	(1) #8	85	
DBC2.5			20.0111	- 55	Max.	(2) #8	125	170
DD02.0		15	25 EQ.	50				170
		18	25	33	Min.	(1) #8	65	
		19	20 EQ.	65	IVIIII.	(1) #0	00	
	6	21	20 EQ.	57				
	0	30	20 DW	33	Min.	(1) #8	85	
			20 DW		Max.	(2) #8	125	
		33	20 STR	33	Min.	(1) #8	85	
			20316	33	Max.	(2) #8	125	

1. Allowable loads are for use when utilizing Allowable Stress Design methodology. For LRFD loads, multiply the ASD tabulated values by 1.6.

2. Min. fastener quantity and tabulated values — fill round hole (one screw total);

Max. fastener quantity and tabulated values - fill round and triangle holes (two screws total).

3. EQ - equivalent, DW - drywall, STR - structural.

Design Example

Given

- 600S125–18 (33 ksi) studs at 24" o.c., 10' tall Mid-point bracing (5' o.c.) Distance from shear center to mid-plane of web, m = 0.408 in. (SFIA Technical Guide Version 2012.101)
- Lateral load = 5 psf

Laterally-Loaded Stud Design

ASD Design load tributary to brace:

Required bracing force (AISI S100 Eq. D3.2.1-3):

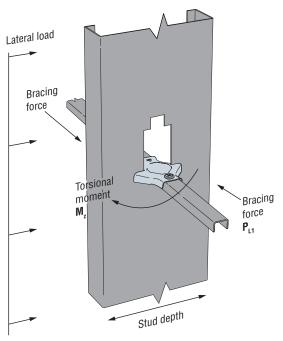
 $P_{L1} = -P_{L2} = 1.5(m/d)W = (1.5)(0.408 \text{ in.}/6 \text{ in.})(50 \text{ lb.}) = 5.1 \text{ lb.}$

Torsional moment:

 $M_Z = P_{L1}d = -P_{L2}d = (5.1 \text{ lb.})(6 \text{ in.}) = 30.6 \text{ in.- lb.}$

From Allowable Loads table above, for 6"-18 mil stud:

Select DBC2.5 with Min. fasteners ((1) #8) Allowable torsional moment = 65 in.- lb. > 30.6 in.- lb. OK



Laterally Loaded C-Stud

SIMPSON

Strong-I

CS Coiled Strap



CS coiled utility straps are an ideal solution when it is desired to brace wall studs via the flanges with strap. These products are packaged in lightweight (about 40 pounds) cartons and can be cut to length on the job site.

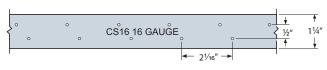
Materials: See table.

Finish: Galvanized (G90); ZMAX®

Installation:

- Use all specified fasteners. See General Notes.
- Refer to the applicable code for minimum edge and end distance.
- The table shows the maximum allowable loads and the screws required to obtain them. See footnote #1. Fewer screws may be used as given by footnote #3.

Codes: See p. 11 for Code Reference Key Chart



CS16 Hole Pattern (all other CS straps similar)

Model No.	Total Length	Connector Material Thickness	Width (in.)		eners (At Block ng Thickness m		Allowable Tension Load	Code Ref.
	(ft.)	mil (ga.)		33 (20)	43 (18)	54 (16)	(lb.)	
CS16	150	54 (16)	11⁄4	(9) #10	(6) #10	(4) #10	1,550	
CS18	200	43 (18)	11⁄4	(7) #10	(5) #10	(3) #10	1,235	
CS20	250	33 (20)	1¼	(6) #10	(4) #10	(3) #10	945	IP1, L2, FL
CS22	300	27 (22)	11⁄4	(5) #10	(3) #10	(3) #10	775	

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

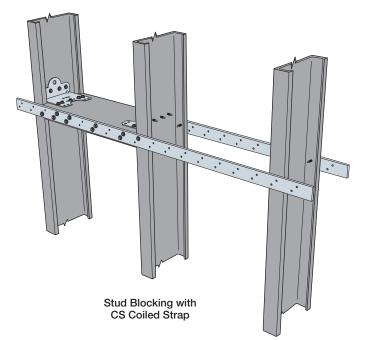
1. In order to achieve the tabulated loads in the strap, attach each strap to the blocking with the tabulated number of screws.

2. Strap length at blocking to achieve tabulated load = number of tabulated screws + 1".

3. Calculate the strap value for a reduced number of screws to the blocking as follows:

Allowable Load = $\frac{\text{No. of Screws Used}}{\text{No. of Screws in Table}} \times \text{Table Load.}$

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



LTB Bridging

SIMPSON Strong-Tie

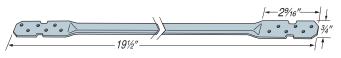
LTB bridging connectors are a cost-effective solution for bracing between non-load-bearing wall studs when compared with field fabricated blocking and clip angles.

Material: 27 mil (22 ga.)

Finish: Galvanized (G90)

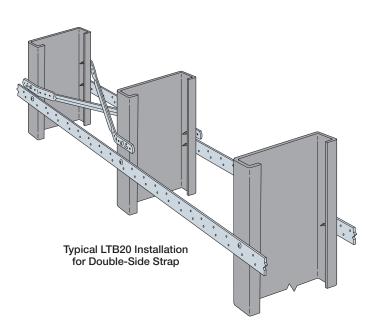
Installation:

- Use (2) #10 screws at each end
- The LTB can be utilized with 3%", 6", 8", and 10" studs at 16" o.c.
- LTB works only in tension, so must be used in cross pairs
- Install bridging tightly; loose installation may allow stud movement





Bridging and Bracing Connectors



Fasteners and Quik Drive°



Simpson Strong-Tie® Fasteners



In the Fastener Marketplace, Simpson Strong-Tie Stands Apart from the Rest.

Quality and reliability are our top priorities. That's why we hire Ph.D.s, metallurgists, materials engineers and structural engineers to create the best possible fasteners. And why each production run goes through rigorous testing to ensure our products can handle higher loads, resist corrosion and make installation more efficient.



Applications: Fastening to Metal



Screws



5/16" hex head, Quik Guard® coating, p. 149

Strong-Drive® FPHSD FRAMING-TO-CFS Screw



Fasteners and Quik Drive®

Flat-pan head, clear zinc coating, p. 143

Self-Drilling **Fiber-Cement Screw**

Type 410 stainless steel

Strong-Drive® XM MEDIUM-HEAD METAL Screw



5/16" hex head. Quik Guard® coating. p. 147

Strong-Drive® PPSD SHEATHING-TO-CFS Screw



Subfloor to CFS screw Quik Guard® and yellow zinc coating, p. 152

Strong-Drive® TB WOOD-TO-STEEL Screw



#4 drill point, black phosphate, mechanically galvanized coating, p. 156

Strong-Drive® XE EXTERIOR STRUCTURAL METAL Screw



5/16" hex head, Quik Guard® coating, p. 141

Strong-Drive® SELF-DRILLING X METAL Screw



5/16" hex head, Quik Guard® and clear zinc coating, p. 145

Self-Drilling E Metal Screw



#3 drill point, clear zinc coating, p. 142

Collated Screws for the Quik Drive® System

Strong-Drive® XL LARGE-HEAD METAL Screw



5/16" hex head, Quik Guard® coating, p. 149





5/16" hex head, Quik Guard® coating, p. 147

Strong-Drive® SELF-DRILLING X METAL Screw



5/16" hex head, Quik Guard® and clear zinc coating, p. 145

DWFSD Drywall-to-CFS Screw

Drywall to steel, #2 point, yellow zinc

coating (54, 43 mil / 16, 18 ga.), p. 155

Strong-Drive® TB WOOD-TO-STEEL Screw

#4 drill point, black phosphate, yellow zinc, mechanically galvanized coating, p. 156

CBSDQ Sheathing-to-CFS Screw



Sheathing to steel, #2 drill point, Quik Guard® coating, p. 150

Strong-Drive® PPSD SHEATHING-TO-CFS Screw

#3 drill point, Quik Guard®, yellow zinc coating, p. 152

DWF Drywall-to-CFS Screw



Drywall to steel, gray phosphate coating (33, 27, 18 mil / 20, 22, 25 ga.), p. 155

PCSD Standing-Seam Roofing Panel Clip Screw



#3 drill point, clear zinc or Quik Guard® coating and Type 410 stainless steel, p. 154

Strong-Drive® XE EXTERIOR STRUCTURAL METAL Screw

Common Applications:

Cold-formed steel framing or connectors to cold-formed steel.

Features:

- 5/16" hex head
- 16 threads per inch
- Dual hardened heat treatment improves drilling efficiency, maximizes ductility and reduces the potential for hydrogen embrittlement

Codes/Standards: ASTM C1513 compliant

- Quik Guard[®] coated for corrosion protection
- Only fastener load rated for Simpson Strong-Tie® L70Z and LS70Z connectors for use with Trex[®] Elevations[™] steel deck framing

Quik Guard[®] Coating

			0								
Length (in.)	Screw Size/ Nail Gauge	Shank Dia. (in.)	Head Dia. (in.)	Drive	Head Type	Threads	Drill Point Size	Point Size	Material/ Coating	Package Size	Model No.
3⁄4	#10	0.19	0.4	5⁄16" hex	Hex washer head	Machine threads	#2	2	Quik Guard coating	100	XEQ34B1016C
3⁄4	#10	0.19	0.4	%ı6" hex	Hex washer head	Machine threads	#2	2	Quik Guard coating	1,000	XEQ34B1016M

Single-Fastener Cold-Formed Steel Member Connection Load^{1, 2, 3}

	Nominal	Weeher	r Load Description	Shear (lb.) Steel Thickness: mil (ga.)						Pullover (lb.)						Pullout (lb.)					
Screw	Dia. d (in.)	Dia. d _w (in.)								Steel Thickness: mil (ga.)						Steel Thickness: mil (ga.)					
Size				27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)
			Allowable strength (ASD)	182	235	365	465	465	465	330	425	605	785	785	785	64	95	128	226	306	501
#10 x ¾"	0.19	0.4	Design strength (LRFD)	292	375	585	695	695	695	525	675	970	1,175	1,175	1,175	103	152	205	361	490	801
			Nominal strength	423	535	830	1,290	1,290	1,290	805	1,035	1,485	2,065	2,065	2,065	167	234	348	555	750	1,225

1. Screws and their connections have been tested per AISI Standard Test Method S904-08 and S905-08.

2. Loads are based on cold-formed steel members with a minimum yield strength, F_y = 33 ksi and tensile strength, F_u = 45 ksi for 43 mil (18 ga.)

and thinner, and a minimum yield strength, $F_y = 50$ ksi and tensile strength, $F_u = 65$ ksi for 54 mil (16 ga.) and thicker.

3. Screws shall extend through the connection with a minimum of three exposed threads per AISI General Provisions Standard Section D1.3.

Screw Strength (lb.)

Screw Size		ninal ngth	(LR	Strength FD) 0.5	Allowable Strength (ASD) $\Omega = 3.0$				
	P _{ss}	P _{ts}	ϕP_{ss}	ϕP_{ts}	P_{ss}/Ω	P _{ts} /Ω			
#10 x ¾"	4" 1,390 2,3		695	1,175	465	785			

Pss - Shear strength.

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

Pts - Tensile strength.



Self-Drilling E Metal Screw

Common Application:

Cold-formed steel framing or connectors to cold-formed steel.

Features:

Fasteners and Quik Drive $^{\otimes}$

- Hex-washer head
- Clear zinc finish

- Recommended for use with certain Simpson Strong-Tie®
 connectors that require large diameter #14 screws
- Bit included in each box

Codes/Standards: ASTM C1513 compliant

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Clear Zinc Coating

	0 00utin	9					
Size	Length (in.)	Hex Head Size (in.)	Threads per Inch	Head Diameter (in.)	Drill Point Size	Carton Quantity	Model No.
#14	1	3⁄8	14	0.5	#3	100	E1B1414R100
#14	1	3⁄8	14	0.5	#3	2,500	E1B1414B

Screw Strength

Screw Size	Model No.	Stre	ninal ngth o.)	(LRFE	Strength)) (lb.) 0.5	Allowable Strength (ASD) (lb.) $\Omega = 3.0$			
		P _{ss}	Pts	φP _{ss}	ϕP_{ts}	P _{SS} /Ω	P_{ts}/Ω		
#14 x 1"	E1B1414	3,130	5,395	1,565	2,700	1,045	1,800		

Cold-Formed Steel Member Connection Loads

	Model No.	Nominal Dia. (in.)	Washer Dia. (in.)	Load	Shear (lb.) Steel Thickness: mil (ga.)					Pullover (lb.)					Pullout (lb.)				
Screw Size										Steel Thickness: mil (ga.)					Steel Thickness: mil (ga.)				
0120					33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)
				ASD	200	295	605	850	1,045	390	505	920	1,160	1,655	105	140	250	320	455
#14 x 1"	E1B1414	0.242	0.5	LRFD	300	445	905	1,280	1,565	585	760	1,380	1,740	2,480	160	210	380	480	680
				Nominal strength	600	890	1,810	2,555	3,130	1,170	1,520	2,760	3,475	4,960	320	415	755	955	1,360

1. Screws shall extend through the connection with a minimum of three exposed threads per AISI S200 General Provisions Standard Section D1.3.

2. Tabulated loads are based on calculations per AISI S100 using the thinner steel member in the connection.

A safety factor of Ω = 3.0 and resistance factor ϕ = 0.5 were used to determine the ASD and LRFD strength values.

3. Loads are based on cold-formed steel members with a minimum yield strength, Fy, of 33 ksi and tensile strength, Fu, of 45 ksi for

43 mil (18 ga.) and thinner, and a minimum yield strength of 50 ksi and tensile strength of 65 ksi for 54 mil (16 ga.) and thicker.

4. For other pertinent information, please refer to the Important Information and General Notes on pp. 14–16 of this catalog.

Strong-Drive® FPHSD FRAMING-TO-CFS Screw

Common Applications:

Cold-formed steel sheet, framing or connectors to cold-formed steel.

Features:

- Flat pan head
- #3 square drive (Driver bit in each box; replacement bit model BIT3S-2)
- Self-drilling
- This screw is also available collated for the Quik Drive® system.

Clear zinc finish

Codes/Standards: ASTM C1513 compliant; ICC-ES ESR-3006; City of L.A. RR 25670

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Clear Zinc Coating

	Length	Thread	Head	Drill	Approx.	1 lb.		Bulk
Size	(in.)	per Inch	Diameter (in.)	Point Size	Count per lb.	Model No.	Carton Quantity	Model No.
#10	3⁄4	16	0.365	#3	165	FPHSD34B1016	5,000	FPHSD34B1016-5K
#12	3⁄4	14	0.365	#3	147	FPHSD34B1214	5,000	FPHSD34B1214-5K

Refer to Fastening Systems catalog C-F-2017 for collated fastener information.

Cold-Formed Steel Member Connection Loads

		Nominal Dia. (in.)	a. Load Description	Shear (lb.)						Pullover (lb.)						Pullout (lb.)					
Model	Size			Steel Thickness: mil (ga.)						Steel Thickness: mil (ga.)						Steel Thickness: mil (ga.)					
No.				27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)
		0.190	ASD load	175	235	380	570	570	570	280	365	485	695	740	740	76	95	156	240	340	505
FPHSD34S1016	#10–16 x ¾"		LRFD load	280	375	605	855	855	855	445	585	775	1,110	1,110	1,110	123	151	250	380	545	805
			Nominal strength	395	535	860	1,305	1,305	1,305	685	895	1,190	1,705	2,215	2,215	190	230	385	585	840	1,235
			ASD load	205	260	410	610	610	610	240	330	430	630	840	1,125	76	95	159	240	345	530
FPHSD34S1214	#12–14 x ¾"	0.216	LRFD load	330	420	650	975	975	975	390	530	685	1,005	1,340	1,690	123	151	255	385	550	855
			Nominal strength	485	610	930	1,385	1,385	1,385	595	815	1,050	1,540	2,060	2,065	190	230	390	590	845	1,295

1. Screws and connections have been tested per AISI Standard Test Method S904-08 and S905-08.

2. The tabulated ASD and LRFD allowable loads for cold-formed steel (CFS) members are based on the lower of the screw strength or the strength of the screw in the connected members per AISI S100 Section E4. Values are based on CFS members with a minimum yield strength of $F_y = 33$ ksi and tensile strength of $F_u = 45$ ksi for 43 mil (18 ga) to 27 mil (22 ga), minimum yield strength of $F_y = 50$ ksi and $F_u = 65$ ksi for 54 mil (16 ga) to 97 mil (12 ga).

3. For design purposes, steel sheet thicknesses are 0.0283" for 27 mil, 0.0346" for 33 mil, 0.0451" for 43 mil, 0.0566" for 54 mil, 0.0713" for 68 mil, and 0.1017" for 97 mil. The actual sheet thickness shall not be less than 95% of these design thicknesses as specified in AISI S100 Section A2.4.

4. Screw diameters per AISI S200 General Provision Commentary Table D1.1.

5. Minimum required screw length is the lesser of ¾" or the minimum length required for the screw to extend through the steel connection a minimum of 3 exposed threads per AISI S200 General Provisions Standard Section D1.3.

The allowable load (ASD) values shown are not permitted to be increased for short-duration loads such as wind or earthquake loads.

- 7. The lower of the Pullover and Pullout allowable loads should be used for tension design.
- The tabulated shear values are based on the thinner steel member in connection. Steel thickness for both members must be in the range of 12–22 gauge.

Strong-Drive® FPHSD FRAMING-TO-CFS Screw



FPHSD (#10) Screw — (Sheet Steel Sheathing to CFS) Nominal Shear Strength (R_n) for Wind (W) and Seismic (S) for Shear Walls¹ (Ib./ft.)

Assembly Description	Max. Aspect Ratio	Fa	astener Spacing (ir	Designation Thickness ⁵ of Stud, Track and Blocking ⁷		
	(h/w)	6	4	3	2	(mil)
0.018" sheet steel, one side	2:1	485 (W) 390 (S)	—	—	—	33 (min.)
0.027" sheet steel, one side	4:1		1,000	1,085	1,170	43 (min.)
	2:1 ³	647	710	778	845	33 (min.)
0.018" sheet steel, both sides	2:1	970 (W) 780 (S)	—	—	_	33 (min.)
0.007" aboot staal, both sides	4:1	—	2,000	2,170	2,340	43 (min.)
0.027" sheet steel, both sides	2:1 ³	1,294	1,420	1,556	1,690	33 (min.)

1. Nominal strength shall be multiplied by the resistance factor (ϕ = 0.6, LRFD Seismic, ϕ = 0.65, LRFD Wind) to determine design strength or divided by the safety factor (Ω = 2.5, ASD Seismic, Ω = 2.0, ASD Wind) to determine allowable strength.

5. In lieu of blocking, panel edges shall be permitted to be overlapped and attached to each other with screw spacing as required for panel edges. Where such a connection is used, tabulated design values shall be reduced 30%.
6. Maximum stud spacing 24" o.c.

2. Screws in the field of the panel shall be installed 12" (305 mm) on center (o.c.). 7.

3. Shear wall height to width aspect ratio (h/w) greater than 2:1, but not exceeding 4:1, shall be permitted provided the nominal strength values are multiplied by 2w/h.

4. Wall studs and track shall be of ASTM A1003 Stuctural Grade 33 Type H steel for members with a designation thickness of 33 and 43 mil.

 Blocking, if applicable, shall be a minimum 33 mil 11/2" width.
 Table based on Table C2.1-1 AISI Standard "North American Standard for Cold-Form Steel Framing-Lateral Design 2007 Edition with Supplement No. 1 and Commentary".

Strong-Drive® SELF-DRILLING X METAL Screw

Common Applications:

- 1. Steel decking or other cold-formed steel framing or connectors to structural steel, up to 3/8" thick.
- 2. Side-lap (stitching) for steel decking.
- 3. Cold-formed steel framing or connectors to cold-formed steel.

Features:

- 5/16" hex head
- Drill point
- Hex washer head

Codes/Standards: ICC-ES ESR-3006, City of LA RR 25670, RR25917 and RR26009, ASTM C1513 compliant, FM Approval #3045651 and #3050714, SDI DDM03, Appendix VII; SDI DDM04; IAPMO UES ER-326

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Refer to Fastening Systems catalog C-F-2017 for collated fastener information.

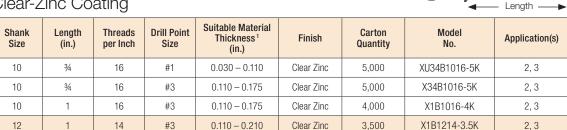


• This screw is also available collated for the Quik Drive® system

Quik Guard® Coating

Shank Size	Length (in.)	Threads per Inch	Drill Point Size	Suitable Material Thickness (in.)	Finish	Carton Quantity	Model No.	Application(s)
10	1	16	#3	0.110 - 0.175	Quik Guard	4,000	XQ1B1016-4K	2, 3
12	1	14	#3	0.110 - 0.210	Quik Guard	3,500	XQ1B1214-3.5K	2, 3
12	1 1⁄4	24	#5	0.125 – 0.500	Quik Guard	2,500	XQ114B1224-2.5K	1, 3
12	1 1/2	24	#5	0.125 – 0.500	Quik Guard	2,000	XQ112B1224-2K	1, 3

Clear-Zinc Coating



1. Suitable material thickness includes all material layers and gaps between layers.

Screw Strength

					Nom	ninal
Model	Size	Model Numbers	Nominal Diameter (in.)	Washer Diameter (in.)	Shear (lb.)	Tension (lb.)
			()	(,	P _{ss}	P _{ts}
Self-Drilling X Metal Screw	#12 x 11⁄4"	XQ114S1224, XQ114B1224-2.5K, X114S1224	0.216	0.415	0.110	4.005
Self-Drilling X Metal Screw	#12 x 1 ½"	XQ112S1224, XQ112B1224-2K	0.216	0.415	3,110	4,985
Self-Drilling X Metal Screw	#10 x ¾"	X34B1016-5K	0.190	0.415	1,625	_
Self-Drilling X Metal Screw	#10 x 1"	X1S1016, XQ1S1016, X1B1016-4K, XQ1B1016-4K	0.190	0.415	1,625	_
Self-Drilling X Metal Screw (undersized drillpoint)	#10 x ¾"	XU34B1016-5K	0.190	0.475	1,735	_

1. Pss and Pts are nominal shear strength and nominal tension strength for the screw itself, respectively, and are the average (ultimate) value of all tests determined by independent laboratory testing.

2. The ASD and LRFD loads for tension are calculated using a safety factor Ω = 3.0 and the resistance factor ϕ = 0.5 respectively.

Strong

Strong-Drive® SELF-DRILLING X METAL Screw



Cold-Formed Steel Connection Loads

						Shea	r (lb.)					I	Pullov	er (lb.))					Pu	llout (l	lb.)			
Model	Nominal	Load		S	iteel T	hickn	ess: n	nil (ga	.)		S	teel T	hickn	ess: m	nil (ga	.)			Stee	el Thic	kness	s: mil ((ga.)		
No.	Dia. (in.) ⁸	Description	27	33	43	54	68	97	1/11/2	1/4"12	27	33	43	54	68	97	27	33	43	54	68	97	3/16"	1/4"	1/2"12
			(22)	(20)	(18)	(16)	(14)	(12)	1/8	1/42	(22)	(20)	(18)	(16)	(14)	(12)	(22)	(20)	(18)	(16)	(14)	(12)	9/16	74.	1/2
		ASD	175	235	360	540	540	540	-	—	330	400	475	645	925	975	71	87	129	200	270	445	_	_	_
X34B1016-5K XQ1S1016 X1S1016	0.190	LRFD	280	375	570	810	810	810	_	_	525	640	755	1,035	1,475	1,465	114	139	205	320	430	715	_	_	_
(#10–16 x 1")		Nominal strength	400	535	815	1,290	1,290	1,290	_	_	805	990	1,160	1,585	2,260	2,695	174	215	315	490	660	1,095	_	_	_
		ASD	176	235	385	595	840	840	_	_	295	375	525	785	1,045	1,210	74	96	147	215	325	500	_	_	_
XQ1S1214 X1S1214	0.216	LRFD	280	375	610	950	1,265	1,265	-	—	470	600	835	1,255	1,675	1,930	117	154	235	340	520	795	_	—	—
(#12–14 x 1")		Nominal strength	400	535	870	1,350	2,135	2,135	_		720	920	1,285	1,925	2,565	2,965	180	235	360	520	800	1,220	_	_	_
		ASD	140	230	350	640	740	935	935	935	265	290	400	720	790	1,390	78	80	115	200	260	460	730	1,375	1,420
XQ78S1224		LRFD	210	365	560	1,025	1,175	1,355	1,355	1,355	395	440	640	1,155	1,260	2,160	117	125	185	320	415	735	1,170	2,135	2,160
(#12–24 x 7%") XQ112S1224 (#12–24 x 11⁄2")	0.216	Nominal strength	420	550	920	1,455	1,675	2,675	2,675	2,675	795	875	985	1,770	1,930	3,400	235	205	280	505	640	1,130	1,990	3,370	4,260

1. Screws and screw connections have been tested per AISI Standard Test Method S904-08 and S905-08 with the exception of 22-gauge values which are based on calculations of the AISI S100 Section E4.

 The tabulated ASD and LRFD allowable loads for cold-formed steel (CFS) members are based on the lower of the screw strength or the strength of the screw in the connected members per AISI S100.

- 3. The safety factor Ω and resistance factor ϕ used to determine the ASD and LRFD strength are based on AISI S100 Section F.
- 4. The nominal strength values listed are achieved under laboratory conditions and should not be used for design loads.
- 5. Values are based on CFS members with a minimum yield strength of $F_y = 33$ ksi and tensile strength of $F_u = 45$ ksi for 43 mil (18 ga) to 27 mil (22 ga), minimum yield strength of $F_y = 50$ ksi and $F_u = 65$ ksi for 54 mil (16 ga) to 97 mil (12 ga), and a minimum yield strength of $F_y = 36$ ksi and $F_u = 58$ ksi for 1% and thicker.
- 6. For design purposes, steel sheet thicknesses are 0.0283" for 27 mil, 0.0346" for 33 mil, 0.0451" for 43 mil, 0.0566" for 54 mil, 0.0713" for 68 mil, and 0.1017" for 97 mil. The actual sheet thickness shall not be less than 95% of these design thickness as specified in AISI S100 Section A2.4.

7. Screw diameters per AISI S200 General Provisions Commentary Table D1-1.

- Minimum required screw length is the lesser of %" or the minimum length required for the screw to extend through the steel connection a minimum of 3 exposed threads per AISI General Provisions Standard Section D1.3.
- 9. Screw head or washer diameter, d_w for #10 and #12 screws is 0.398".
- 10. The allowable load (ASD) values showing are not permitted to be increased for short duration loads such as wind or earthquake loads.
- 11. The lower of the Pullover and Pullout allowable loads should be used for tension design.
- 12. Not applicable for XQ78S1224.
- 13. The tabulated shear values are based on the thinner steel member in connection. Steel thickness for both members must be in the range of 1/2" 22 gauge.
- 14. The XQ-S1224 screws are recommended for 16-gauge and thicker steel.

Strong-Drive® XM MEDIUM-HEAD METAL Screw

Common Applications:

Steel decking to structural members involving wide or narrow valley; nestable or interlocking steel decking

Strong-Drive® metal screws are load-tested and code-listed, allowing you to get the maximum load values for installation. Comparison testing shows Strong-Drive® XM Medium-Head Metal screws are stronger than many alternative fastener types in 33 ksi and 50 ksi steel decking.

Features:

- 5/16" hex drive
- ½"-diameter hex washer head is ideal for narrow-channel steel decking
- Available only in 1 1/4" length with #5 drill point
- Available in Quik Guard® coating
- Available in bulk for hand-drive installation and collated for the Quik Drive[®] system

Codes/Standards: IAPMO UES ER-326, FM Approval 3050714, State of Florida FL16937, City of Los Angeles RR26009, SDI DDM04

U.S. Patent Pending

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.



Quik Guard® Coating

	Length	Hex Head	Washer	Threads	Point	Suitable Material		Bulk		Collated	
Size	(in.)	Size (in.)	Dia. (in.)	per Inch	Size	Thickness (in.)	Carton Qty.	Model No.	Carton Qty.	Model No.	PROSDX150
#12	11⁄4	5⁄16	0.483	24	#5	0.125 – 0.500	2,000	XMQ114B1224-2K	1,500	XMQ114S1224	\checkmark



SIMPSO

Strong-Drive® XL LARGE-HEAD METAL Screw

High-Performance Screw Alternative to Welds and Pins

Strong-Drive[®] metal screws are load-tested and code-listed, allowing you to get the maximum load values for installation. Strong-Drive[®] XL Large-Head Metal screws are the perfect choice when high shear or uplift resistance is required. Strong-Drive[®] XM Medium-Head Metal screws, with their 1/2" washer head, are designed for narrow flutes commonly found on interlocking deck profiles. In high-strength decks ($F_y = 50$ ksi), these screws are excellent 1-for-1 replacements for pins. The Self-Drilling X Metal screw is your go-to screw for lighter-duty support fastening and stitching applications. These screws are available in bulk or collated for Quik Drive[®] steel-decking systems.

Simpson Strong-Tie provides a full offering of code-listed fasteners for your next steel-decking job.



DDM04 Approved

Fasteners and Quik Drive®





FM APPROVAL #3050714 #3045651



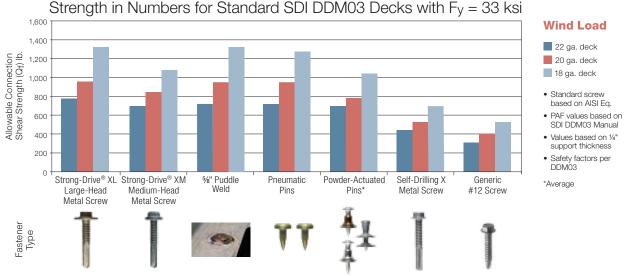


Strong

Strength in Numbers

Comparison testing shows that Strong-Drive XL Large-Head Metal screws and Strong-Drive XM Medium-Head Metal screws are stronger than many alternative fastener types in 33 ksi and 50 ksi steel decking.

e stronger than many alternative fastener types in 33 ksi and 50 ksi steel decking.



Steel Deck Diaphragm Calculator



Steel Deck Design Made Easier

The Steel Deck Diaphragm Calculator web app offers optimized steel deck design solutions based on fastener and labor costs for a given shear and uplift. It can provide calculations for any solution generated. Generate diaphragm tables for various roof and floor decks using Simpson Strong-Tie[®] fasteners. The app can also generate a submittal package that includes fastener information, code reports, Factory Mutual reports, Appendix VII of DDM03, coating information and tools for installation. The app is accessible from any web browser and does not require downloading or installing special software. Users can:

- Design for multiple zones and develop solutions in either ASD or LRFD
- Modify deck properties from the standard properties listed in SDI DDM03 and DDM04
- Generate multiple cost- and labor-optimized solutions with calculations included
- Generate tables in Nominal, ASD Wind, LRFD Wind, ASD Seismic or LRFD Seismic
- Design for loads using the new Strong-Drive[®] XL Large-Head Metal screw (included in the optimization calculator)
- Design for additional structural patterns not covered in SDI literature
- Access proprietary deck tables with the new Strong-Drive® XM Medium-Head Metal screw



Strong-Drive® XL LARGE-HEAD METAL Screw

Common Applications:

Steel decking or other cold-formed steel framing or connectors to structural steel supports up to %" in thickness where high shear and pullover values are needed.

Features:

- 5/16" hex drive (driver part #BITHEXLB516)
- 5%" diameter hex washer head

• #5 drill point

• Available collated; see strongtie.com/steeldeck

• Quik Guard® coating

Codes/Standards: IAPMO UES ER-326, FM Approval #3050714, SDI DDM03 Appendix IX; SDI DDM04, Patent Pending

Quik Guard® Coating

			-					
Size	Length (in.)	Hex Head Size (in.)	Washer (in.)	Threads per Inch	Drill Point Size	Suitable Material Thickness (in.)*	Carton Qty.	Model No.
#12	11⁄4	5⁄16	0.625	24	#5	0.125 – 0.500	2,000	XLQ114B1224-2K

*Maximum material thickness includes all material layers and gaps between layers

Refer to Fastening Systems catalog C-F-2017 for collated fastener information.

Cold-Formed Steel Connection Loads

							Sh (II						Pull (It	over o.)			Pullout (lb.)
Model No.	Size	Nominal Dia. (in.)	Washer Dia. (in.)	Load Description		5	Steel Th mil		S			5		iicknes: (ga.)	S		Support Thickness (in.)
					27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)	3⁄16"
				ASD	605	725	1,015	1,035	1,035	1,035	525	690	990	1,220	1,220	1,220	575
XLQ114T1224, XLQ114B1224-2K	#12-24 x 11⁄4"	0.216	0.625	LRFD	970	1,160	1,555	1,555	1,555	1,555	840	1,100	1,585	1,950	1,950	1,950	860
				Nominal strength	1,400	1,670	2,340	3,110	3,110	3,110	1,295	1,705	2,430	2,995	2,995	2,995	1,720

1. Screws and screw connections have been tested per AISI Standard Test Method S904-08 and S905-08.

2. The tabulated ASD and LRFD allowable loads for cold-formed steel (CFS) members are based on the lower of the screw strength

or the strength of the screw in connected members per AISI S100.

3. The safety factor Ω and resistance factor ϕ used to determine the ASD and LRFD strengths are based on AISI S100, Section F.

4. The nominal strength values listed are achieved under laboratory conditions and should not be used for design loads.

5. Values are based on CFS members with a minimum yield strength of $F_y = 33$ ksi and tensile strength of $F_u = 45$ ksi for 43 mil (18 ga.) to 27 mil (22 ga.), minimum yield strength of $F_y = 50$ ksi and $F_u = 65$ ksi for 54 mil (16 ga.) to 97 mil (12 ga.) and a support member of $\%^{\circ}$ " thickness with a minimum yield strength of $F_y = 36$ ksi and tensile strength of $F_u = 50$ ksi.

6. For design purposes, steel sheet thickness are 0.0283" for 27 mil, 0.0346" for 33 mil, 0.0451" for 43 mil, 0.0566" for 54 mil, 0.0713" for 68 mil, and 0.1017" for 97 mil. The actual sheet thickness shall not be less than 95% of these design thicknesses as specified in AISI S100 Section A2.4.

7. Screw diameters are per AISI S200 General Provisions Commentary Table D1-1.

8. Minimum required screw length is the lesser of ³/₄" or the minimum length required for the screw to extend through the steel connection a minimum of three exposed threads per AISI General Provisions Standard Section D1.3.

9. The tabulated values are based on a minimum support thickness of $\mathfrak{Y}_{16}".$

Strong

CBSDQ SHEATHING-TO-CFS Screw

Common Applications:

replacement bit model BIT2SU)

Sheathing to cold-formed steel. (Recommended thicknesses: 16 and 18 ga.)

Features:

- Ribbed flat head with nibs for easy countersinking
- #2 undersized square drive (driver bit in each box;
- #2 drill point with wings
 - Quik Guard[®] coating
 - Curved collation

Codes/Standards: ASTM C1513 compliant, #8 screws meet minimum head diameter requirement per AISI S213, Lateral Design Standard; City of L.A. RR 25670

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Quik Guard® Coating

Guilt Gu		ung						/o <u> </u>	
Length (in.)	Shank Size	Threads per Inch	Drill Point Size	Carton Quantity	Min. Head Dia. (in.)	Model No.	PR0 200S	PR0 250	PR0 300S
1 5⁄8	#8	18	#2	1,500	0.322	CBSDQ158S	\checkmark	\checkmark	\checkmark
21⁄4	#10	16	#2	1,000	0.322	CBSDQ214S		\checkmark	\checkmark

These coated fasteners possess a level of corrosion resistance that makes them suitable for use in some exterior and corrosive environments and with some preservative-treated woods. For applications in higher-exposure applications, consider Type-300 series stainless-steel fasteners for superior corrosion resistance. See pp. 18–21 for additional important information before selecting a fastener for a specific application.

CBSDQ/PPSD Screws¹⁰ — Nominal Shear Strength (R_n) for Wind and Other In-Plane Loads for Shear Walls^{1,4,6} (Ib./ft.)

Accomble Description	Maximum Aspect Ratio		Fastener Spacing	at Panel Edges (in.)	
Assemble Description	(h/w)	6	4	3	2
15/32" structural 1 sheathing (4-ply), one side	2:1	1,065³			
$7_{\rm 16}$ " rated sheathing (OSB), one side	2:1	910 ³	1,410	1,735	1,910
746" rated sheathing (OSB), one side oriented perpendicular to framing	2:1	1,020	_	_	_
7⁄ю" rated sheathing (OSB), one side	2:15	_	1,025	1,425	1,825

- 1. Nominal strength shall be multiplied by the resistance factor ($\phi = 0.65$) to determine the design strength or divided by the safety factor ($\Omega = 2.0$) to determine the allowable strength.
- 2. Screws in the field of the panel shall be installed 12" (305 mm) on center (o.c.).
- 3. Where fully blocked gypsum board is applied to the opposite side of this assembly, per Table C2.1-2 AISI S-213 Lateral Standard, with screw spacing at 7" (178 mm) o.c. edge and 7" (178 mm) o.c. field, these nominal strengths are permitted to be increased by 30%.
- 4. For walls with material of the same type and nominal strength applied to opposite faces of the same wall, the available strength of material of same capacity is cumulative. Where the material nominal strengths are not equal, the available strength shall be either two times the available strength of the material with the smaller value or shall be taken as the value of the stronger side, whichever is greater. Summing the available strengths of dissimilar material applied to opposite faces or to the same wall line is not allowed.

 Shear wall height to width aspect ratio (h/w) greater than 2:1, but not exceeding 4:1, shall be permitted provided the nominal shear strength is multiplied by 2w/h.

156" _ 014

6. For wood structural panel sheathed shear walls, tabulated Rn values shall be applicable for short-term load duration (wind loads). For other in-plane lateral loads of normal or permanent load duration as defined by the AWC NDS, the values in the table above for wood structural panel sheathed shear walls shall be multiplied by 0.63 (normal) or 0.56 (permanent).

- 7. Maximum stud spacing 24" o.c.
- 8. All sheathing edges shall be attached to framing or $1\,\ensuremath{\ensure$
- 9. Table based on Table C2.1-1 AISI S-213 Lateral Standard.
- 10. #8 screws CBSDQ and PPSD.
- 11. Stud, track and blocking (if applicable) shall be a minimum of 33 mil.

CBSDQ SHEATHING-TO-CFS Screw

CBSDQ/PPSD Screws 11 — Nominal Shear Strength (Rn) for Seismic and Other In-Plane Loads for Shear Walls 1,4,7 (lb./ft.)

Assemble Description	Maximum Aspect Ratio	Fast	ener Spacing a	it Panel Edges ^a	² (in.)	Designation Thickness ^{5,6} of Stud, track and Blocking	Required Sheathing
Assemble Description	(h/w)	6	4	3	2	(mil)	Screw Size
	2:1 ³	780	990		—	33 or 43	8
¹⁵ / ₃₂ " structural 1 sheathing (4-ply), one side	2:1	890	1,330	1.775	2,190	43 or 54	8
	2.1	090	1,330	1,775	2,190	68	10
	2:1 ³	700	915	—	—	33	8
7⁄16" rated sheathing (OSB),	2:1 ³	825	1,235	1,545	2,060	43 or 54	8
one side	2:1	940	1,410	1,760	2,350	54	8
	2:1	1,232	1,848	2,310	3,080	68	10

1. Nominal strength shall be multiplied by the resistance factor ($\phi = 0.60$) to determine the design strength or divided by the safety factor ($\Omega = 2.5$) to determine the allowable strength.

2. Screws in the field of the panel shall be installed 12" (305 mm) on center (o.c.).

- 3. Shear wall height to width aspect ratio (h/w) greater than 2:1, but not exceeding 4:1, shall be permitted provided the nominal shear strength is multiplied by 2w/h.
- 4. For walls with material of the same type and nominal strength applied to opposite faces of the same wall, the available strength of material of same capacity is cumulative. Where the material nominal strengths are not equal, the available strength shall be either two times the available strength of the material with the smaller value or shall be taken as the value of the stronger side, whichever is greater. Summing the available strengths of dissimilar material applied to opposite faces or to the same wall line is not allowed.
- Substitution of a stud or track of a different designation thickness is not permitted.

 Wall studs and track shall be of ASTM A1003 Structural Grade 33 Type H steel for members with a designation thickness of 33 and 43 mil, and A1003 Structural Grade 50 Type H steel for members with a designation thickness equal to greater than 54 mil.

7. For wood structural panel sheathed shear walls, tabulated R_n values shall be applicable for short-term load duration (seismic loads). For other in-plane lateral loads of normal or permanent load duration as defined by the AF&PA NDS, the values in the table above for wood structural panel sheathed shear walls shall be multiplied by 0.63 (normal) or 0.56 (permanent).

- 8. Maximum stud spacing 24" o.c.
- 9. All sheathing edges shall be attached to framing or 11/2" width 33 mil blocking.
- 10. Table based on Table C2.1-3 AISI S-213 Lateral Standard.
- 11. #8 screws CBSDQ and PPSD.

151



Strong-Drive® PPSD SHEATHING-TO-CFS Screw

Common Application:

- Subfloor/sheathing to cold-formed steel. (#8 maximum thickness: 54 mil / 16 ga.; #10 and #12 - maximum thickness: 97 mil / 12 ga.)
- Shearwall applications (see load tables on pp. 150 and 151)

Features:

- · Flat head with nibs for easier countersinking
- #3 square drive (replacement bit BIT3SU-2 for Quik Guard[®] and BIT3S-2 for yellow-zinc coating)
- Head diameter meets AISI S-213 Lateral Standard
- · This screw is also available collated for the Quik Drive® system

• Fine threads Pilot point

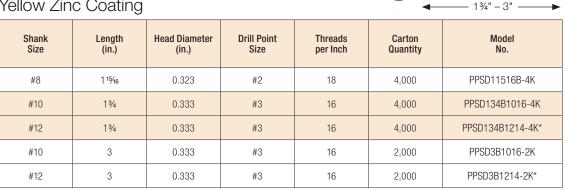
Codes/Standards: ASTM C1513 compliant; ICC-ES ESR-3006; City of L.A. RR 25670

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Refer to Fastening Systems catalog C-F-2017 for collated fastener information.

Quik Guar	rd® Coating)				1¾" – 3"►
Shank Size	Length (in.)	Head Diameter (in.)	Drill Point Size	Threads per Inch	Carton Quantity	Model No.
#8	1 15/16	0.323	#2	18	4,000	PPSDQ11516B-4K
#10	1¾	0.333	#3	16	4,000	PPSDQ134B1016-4K
#12	1¾	0.333	#3	16	4,000	PPSDQ134B1214-4K*
#10	3	0.333	#3	16	2,000	PPSDQ3B1016-2K
#12	3	0.333	#3	16	2,000	PPSDQ3B1214-2K*

Yellow Zinc Coating



* Has underhead nibs.



Strong-Drive® PPSD SHEATHING-TO-CFS Screw

PPSD - Pullout Loads - Steel Connections

					Pullout L	.oad (lb.)		
Model No.	Screw Size	Load Description			Steel Thickn	ess: mil (ga.)		
	0120	Decemption	27 (22)	33 (20)	43 (18)	54 (16)	68 (14)	97 (12)
		ASD	63	87	119	183	_	—
PPSD11516S0818 PPSDQ11516S0818	#8	LRFD	100	139	190	295	_	_
		Nominal strength	154	215	290	450	_	_
		ASD	80	128	194	315	425	480
PPSD134S1016 PPSDQ134S1016	#10	LRFD	128	205	310	500	680	765
		Nominal strength	225	325	480	765	1,045	1,205
		ASD	80	128	194	315	425	480
PPSD3S1016 PPSDQ3S1016	#10	LRFD	128	205	310	500	680	765
		Nominal strength	225	325	480	765	1,045	1,205

1. Screws and connections have been tested per AISI Standard Methods S904-08 and S905-08.

2. Values are based on cold-formed steel (CFS) members with a minimum yield strength, F_y of 33 ksi and minimum tensile strength, F_u of 45 ksi for 43 mil (18 ga.) to 27 mil (22 ga.), and a minimum yield strength, F_y of 50 ksi and minimum tensile strength, F_u of 65 ksi for 54 mil (16 ga.) to 97 mil (12 ga.).

3. For design purposes, steel sheet thicknesses are 0.0283" for 27 mil (22 ga.), 0.0346" for 33 mil (20 ga.), 0.0451" for 43 mil (18 ga.), 0.0566" for 54 mil (16 ga.), 0.0713" for 68 mil (14 ga.) and 0.1017" for 97 mil (12 ga.). The actual sheet thickness shall not be less than 95% of these design thicknesses as specified in AISI S100, Section A2.4.

4. A minimum of three exposed screw threads is required to achieve the loads in the Table.

PPSD — Pull-Through Loads — Rated Sheathing Panels

				Re	ference Pull-T	hrough Load (
Model No.	Screw	Load		Minin	num Nominal F	anel Thicknes	s (in.)			
Model No.	Size	Description		Plywood			OSB			
			15/32	¹⁹ / ₃₂	²³ / ₃₂	15/32	¹⁹ / ₃₂	²³ / ₃₂		
		ASD	83	84	116	49	109	117		
PPSD11516S0818 PPSDQ11516S0818	#8	LRFD	179	181	250	106	235	255		
		Nominal strength	415	420	580	245	545	585		
		ASD	75	85	118	52	111	114		
PPSD134S1016 PPSDQ134S1016	#10	LRFD	162	184	255	112	240	245		
		Nominal strength	375	425	590	260	555	570		
		ASD	75	85	118	52	111	114		
PPSD3S1016 PPSDQ3S1016	#10	LRFD	162	184	255	112	240	245		
		Nominal strength	375	425	590	260	555	570		

1. The tabulated values are based on testing per AC233.

2. ASD pull-through loads based on a factor of safety of 5 applied to the nominal strength value

($C_D = 1.0$, increases to $C_D = 1.6$ allowed where applicable).

3. LRFD load based on adjustment of ASD load per NDS 2015, Appendix N using K_f = 3.32, ϕ = 0.65, and λ =1.0.

PCSD Standing-Seam Roofing Panel Clip Screw

Common Applications:

Cold-formed steel framing or connectors to cold-formed steel.

Features:

- Pancake head
- #2 square drive (replacement bit BIT2SU-2 for Quik Guard[®]; BIT2S-2 for Type 410 stainless steel and clear-zinc coating)

Codes/Standards: ASTM C1513 compliant

- Type 410 stainless steel is coated for additional corrosion protection
- This screw is also available collated for the Quik Drive® system

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Refer to Fastening Systems catalog C-F-2017 for collated fastener information.

Type 410 Stainless Steel

51						
Size	Length (in.)	Thread per Inch	Drill Point Size	Head Diameter (in.)	Carton Quantity	Model No.
#10	1	16	#3	0.405	5,000	SSPCSD1B1016-5K

Quik Guard® Coating

	0					
Size	Length (in.)	Thread per Inch	Drill Point Size	Head Diameter (in.)	Carton Quantity	Model No.
#10	1	16	#3	0.405	5,000	PCSDQ1B1016-5K
#12	1	14	#3	0.405	4,000	PCSDQ1B1214-4K

Clear Zinc Coating

	0					
Size	Length (in.)	Thread per Inch	Drill Point Size	Head Diameter (in.)	Carton Quantity	Model No.
#10	1	16	#3	0.405	5,000	PCSD1B1016-5K
#12	1	14	#3	0.405	4,000	PCSD1B1214-4K

Cold-Formed Steel (CFS) Member Connection Loads

			Nominal			Shear (lb.)			Pullover (lb.)				Pullout (lb.)											
Model	Model			Load	Steel Thickness: mil (ga.) ⁶				Steel Thickness: mil (ga.)6				Steel Thickness: mil (ga.)6											
wouer	No.	Size	Dia. ⁷ (in.)	Description	27	33	43	54	68	97	27	33	43	54	68	97	22	27	33	43	54	68	97	
					(22)	(20)	(18)	(16)	(14)	(12)	(22)	(20)	(18)	(16)	(14)	(12)	(24)	(22)	(20)	(18)	(16)	(14)	(12)	
DCCD		#10_10/1	0.10		ASD ²	168	250	385	570	570	570	172	255	430				67	68	95	138	255	310	
PCSD	PCSD PCSD1S1016 #10-16x1"	PCSD1S1016 #10–16x1" 0.190	0.190	Nominal load ⁴	420	570	875	1,475	1,645	1,690	420	735	1,220		_	-	171	166	235	340	630	760		
DOOD	D00D101014	#10 14,11	0.010	ASD ²	156	295	420	585	585	585	210	320	505				66	66	88	129	255	320		
PCSD PCSD1S1214	PCSD1S1214	#12–14x1" 0.2	#12–14x1" 0.216		Nominal load ⁴	400	695	955	1,640	1,890	2,290	520	780	1,245	_	_	-	170	162	240	315	625	785	_

 Screws and screw connections have been tested per AISI Standard Test Method S904-08 and S905-08.

The tabulated ASD loads for cold-formed steel (CFS) members are based on the lower of the screw strength or the strength of the screw in the connected members per AISI S100 Section E4.

3. The safety factor is based on AISI S100 Chapter F for tested connections.

 The nominal load values listed are achieved under laboratory conditions and should not be used for design loads.

- 5. Values are based on CFS members with a minimum yield strength of F_y = 33 ksi and tensile strength of F_u = 45 ksi for 43 mil (18 ga) to 27 mil (22 ga.), minimum yield strength of F_y = 50 ksi and tensile strength of F_u = 65 ksi for 22 mil (24 ga.), and a minimum yield strength of F_y = 50 ksi and F_u = 65 ksi for 54 mil (16 ga.) and thicker.
- For design purposes, steel sheet thicknesses are 0.0227" for 22 mil, 0.0283" for 27 mil, 0.0346" for 33 mil, 0.0451" for 43 mil, 0.0566" for 54 mil, 0.0713"

for 68 mil, and 0.1017" for 97 mil. The actual sheet thickness shall not be less than 95% of these design thickness as specified in AISI S100 Section A2.4.

Screw diameters per AISI S200 General Provision Commentary Table D1.1.
 Minimum required screw length is the lesser of %" or the minimum length

 Minimum required screw length is the lesser of ³/⁴ or the minimum length required for the screw to extend through the steel connection a minimum of 3 exposed threads per AISI S200 General Provisions Standard Section D1.3.

9. Larger of screw head or washer diameter, d_w, for #10 and #12 screws is 0.375".

- 10. The allowable load (ASD) values shown are not permitted to be increased for short-duration loads such as wind or earthquake loads.
- 11. The lower of the Pullover and Pullout allowable loads should be used for tension design.
- 12. The tabulated shear values are based on the thinner steel member in connection. Steel thickness for both members must be in the range of 12–22 gauge.

DWFSD Drywall-to-CFS Screw

Common Applications:

Drywall to cold-formed steel. (Recommended max. steel thicknesses: 54 and 43 mil / 16 and 18 ga.)

Features:

- Bugle head
- #2 Phillips (driver bit in each box; replacement bit model BIT2P)

Codes/Standards: ASTM C954 compliant; City of L.A. RR 25670

Yellow-Zinc Coating

	0				
Length (in.)	Shank Size	Threads per Inch	Drill Point Size	Box Quantity	Model No.
1 1⁄4	#6	20	#2	2,500	DWFSD114PS
1 %	#6	20	#2	2,500	DWFSD158PS
1 7⁄8	#6	20	#2	2,000	DWFSD178PS
2¾	#8	20	#2	1,500	DWFSD238PS

• Fine threads

• Sharp point

Curved collation

• Gray-phosphate coating

- 1 5/2'

• Curved collation

11/4" - 23/8'

DWF Drywall-to-CFS Screw

Common Applications:

Drywall to cold-formed steel. (Recommended thicknesses: 33, 27 and 18 mil; 20, 22 and 25 ga.)

Features:

- Bugle head
- #2 Phillips (driver bit in each box; replacement bit model BIT2P)
- · Fine threads

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Codes/Standards: ASTM C1002-04 Type S compliant; City of L.A. RR 25670

Gray-Phosphate Coating

Length (in.)	Shank Size	Carton Quantity	Model No.
1 1⁄4	#6	2,500	DWF114PS
1%	#6	2,500	DWF158PS



Strong-

Strong-Drive® TB WOOD-TO-STEEL Screw

Common Applications:

Wood to hot-rolled or heavy-gauge cold-formed steel (Maximum recommended thicknesses: 5/16")

Features:

- Flat head with nibs for easy countersinking
- #3 square drive (driver bit in each box; replacement bit model BIT3S; use BIT3SU for Mechanically Galvanized – N2000[®])
- #4 drill point with wings
- This screw is also available collated in straight collation for the Quik Drive[®] system; see p. 171 for details

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, use this product in dry, interior and noncorrosive environments only.

Mechanically Galvanized - N2000

Length (in.)	Maximum Grip Length (in.)	Shank Size	Head Diameter (in.)	Threads per Inch	Carton Quantity	Model No.
23⁄8"	1.65	#12	0.39	14	1,500	TBG1260R1500



Black Phosphate Coating

Black i hoopinato coating											
Length (in.)	Maximum Grip Length (in.)	Shank Size	Head Diameter (in.)	Threads per Inch	Carton Quantity	Model No.					
13⁄4"	1.06	#12	0.39	14	2,000	TBP1245R2000					
13⁄4"	1.06	#12	0.39	14	50	TBP1245R50					
2%"	1.65	#14	0.46	14	1,000	TBP1460R1000					
2%"	1.65	#14	0.46	14	50	TBP1460R50					
3"	2.24	#14	0.46	14	1,000	TBP1475R1000					
3"	2.24	#14	0.46	14	50	TBP1475R50					

* Grip length includes side member, steel thickness, air gap (if any) and allowance for three threads protuding through the steel.

Strong-Drive® TB WOOD-TO-STEEL Screw

Mechanically Galvanized — N2000°										
Max. Grip Length (in.)*	Shank Size	Threads Per Inch	Carton Quantity	Model No.	PRO HSD60	PRO HSD75				
1.055	#12	14	1,000	TBG1245S	\checkmark					
1.645	#12	14	1,000	TBG1260S	\checkmark	\checkmark				
1.055	#14	14	1,000	TBG1445S	\checkmark					
1.645	#14	14	750	TBG1460S	\checkmark	\checkmark				
2.236	#14	14	750	TBG1475S		\checkmark				
	Max. Grip Length (in.)* 1.055 1.645 1.055 1.645	Max. Grip Length (in.)* Shank Size 1.055 #12 1.645 #12 1.055 #14 1.645 #14	Max. Grip Length (in.)* Shank Size Threads Per Inch 1.055 #12 14 1.645 #12 14 1.055 #14 14 1.055 #14 14 1.645 #14 14	Max. Grip Length (in.)* Shank Size Threads Per Inch Carton Quantity 1.055 #12 14 1,000 1.645 #12 14 1,000 1.055 #14 14 1,000 1.055 #14 14 1,000 1.055 #14 14 1,000 1.645 #14 14 750	Max. Grip Length (in.)* Shank Size Threads Per Inch Carton Quantity Model No. 1.055 #12 14 1,000 TBG1245S 1.645 #12 14 1,000 TBG1245S 1.055 #12 14 1,000 TBG1260S 1.055 #14 14 1,000 TBG1445S 1.645 #14 14 750 TBG1460S	Max. Grip Length (in.)* Shank Size Threads Per Inch Carton Quantity Model No. PRO HSD60 1.055 #12 14 1,000 TBG1245S ✓ 1.645 #12 14 1,000 TBG1260S ✓ 1.055 #14 14 1,000 TBG1260S ✓ 1.055 #14 14 750 TBG146SS ✓				

	Max. grip length
	10000000000000000000000000000000000000
-	1¾" – 3"

- Max. grip length -

1¾" – 3"

🗲 Max. grip length 🔸

Yellow Zin	ic Coating				— 1¾" – 3"		
Length in. (mm)	Max. Grip Length (in.)*	Shank Size	Threads Per Inch	Carton Quantity	Model No.	PRO HSD60	PRO HSD75
1 ½ (40)	0.826	#14	14	1,000	TB1440S	\checkmark	
1 ¾ (45)	1.055	#14	14	1,000	TB1445S	\checkmark	
23⁄8 (60)	1.645	#14	14	750	TB1460S	\checkmark	\checkmark
3 (75)	2.236	#14	14	750	TB1475S		\checkmark

Black Phosphate Coating

DIGONTITIC		Jating			-		-
Length in. (mm)	Max. Grip Length (in.)*	Shank Size	Threads Per Inch	Carton Quantity	Model No.	PRO HSD60	PRO HSD75
1 ¾ (45)	1.055	#12	14	1,000	TBP1245S	\checkmark	
23% (60)	1.645	#12	14	1,000	TBP1260S	\checkmark	\checkmark
23% (60)	1.645	#14	14	750	TBP1460S	\checkmark	\checkmark
3 (75)	2.236	#14	14	750	TBP1475S		\checkmark

* Grip length includes side member, steel thickness, air gaps (if any) and allowance for three threads protruding through the steel.

Strong-Drive® TB WOOD-TO-STEEL Screw

TB — Allowable Loads — DF and SP Lumber Attachment to Steel (Steel Members 16 ga. – 5/16" Thick)



	Length	Nominal Wood Thickness (in.)	Steel Thickness mil (ga.)	DF/SP Allowable Load (lb.)			
Model No.	in. (mm)			Uplift		Shear	
-				C _D = 1.0	C _D = 1.6	C _D = 1.0	C _D = 1.6
			54 (16)	195	195	210	335
TB1460S 23% (60)			68 (14)	225	225	210	335
			97–312 (12 – 5/16'')	245	390	215	345
	S 3 (75)	2x	54 (16)	195	195	210	335
TB1475S		68 (14) 97–312 (12 – ⁵⁄₁6")	68 (14)	225	225	210	335
				245	390	215	345

1. For use with structural steel members up to 5%" thick or cold-formed steel members 54 mil (16 ga.) or thicker.

2. Standard product available in a black phosphate, yellow zinc or N2000 coating for additional corrosion protection

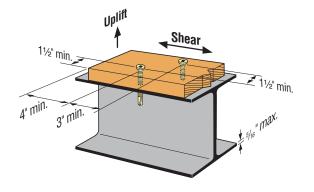
(TBG1460S or TBG1475S).

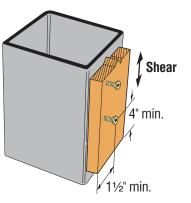
Fasteners and Quik Drive®

3. For use with 2x (11/2") DF/SP only.

4. For use with QD HSD60 or HSD75 tool.

5. Use increased allowable loads ($C_D = 1.6$) only when resisting wind or seismic forces.





	PROLDH Underlayment/Backerboard System
	Applications: Cement board and fiber-cement board underlayment/ backerboard to wood or steel
	PRO200 Drywall System
7	Applications: Drywall to wood or steel
	PRO250 Subfloor System
	Applications: Subfloor to wood or steel
-2/-	PROPP150 Metal Roofing System
R	Applications: Panel clips for standing-seam roofing, steel decking to structural steel members, panel flanges for snap-and-seam metal roofing
ball Direct State	BSD200 Structural Steel-Decking System
TIM	Applications: Steel decking to structural steel members and stitching of steel panels at the edges ("side lap" stitching)
	PROSDX150 Steel-Decking Attachment
Quit Drive	Applications: Steel decking to structural steel members
	PROPH Cold-Formed Steel Framing Attachment
Quik Drive	Applications: Fasten cold-formed steel framing
	PROHX516 Steel-to-Steel Fastening Attachment
Quik Drive	Applications: Cold-formed steel framing, steel decking to structural steel members
	PRO200S Multi-Purpose Attachment
Out Drive	Applications: Subfloor to wood or steel, wall plates, stair treads, sheathing, fiber cement siding to steel, gypsum panel to wood or steel
Commence of the	PROHSD60 Wood-to-Steel Attachment
deit Diver	Applications: Truck beds and trailer flooring to steel, wood nailer to structural steel
- 61	PROHSD75 Wood-to-Steel Attachment
0.0 Phy	Applications: Truck beds and trailer flooring to steel, wood nailer to structural steel

Connectors for Cold-Formed Steel Construction

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159

Quik Drive® Applications: Drywall

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Quik Drive[®] auto-feed screw driving systems are ideal for fastening drywall. They provide a fast, efficient solution with a precision countersink adjustment that produces consistent dimples.

PRO200 System



- Compact body for reduced weight and easy handling
- Smooth nose will not mar drywall surface
- Slim profile allows driving in corners



PRO250DW Attachment



- Compact body for reduced weight and easy handling
- Smooth nose will not mar drywall surface
- Slim profile allows driving in corners



Quik Drive® Applications: Drywall



Collated Screws for the Quik Drive® System

Fastener Model	PR0200	PR0250DW
DWF Drywall-to-CFS Screw Drywall to steel, gray phosphate coating (33, 27, 18 mil / 20, 22, 25 ga.)	114", 15%"	N/A
DWFSD Drywall-to-CFS Screw Drywall to steel, #2 point, yellow zinc coating (54, 43 mil / 16, 18 ga.)	11⁄4", 15⁄8", 17⁄8"	17%", 2%"
DWFSD Drywall-to-CFS Screw Drywall to steel, #2 point, Quik Guard® coating (54, 43 mil / 16, 18 ga.)	1 1/4"	11⁄4"

Connectors for Cold-Formed Steel Construction

Quik Drive® Applications: Underlayment/Backerboard



Quik Drive[®] auto-feed screw driving systems are ideal for underlayment. The variety of screws solves challenges such as driving over radiant heat panels and the extension enables stand-up-and-drive installation.

PROCGB Combo System



• Slim profile allows driving in corners

- Drives fasteners that meet ANSI standards
- · Compact body for reduced weight and easy handling
- Includes both PROLDH Attachment and PRO200S Attachment for added versatility

↓1" – 2"→

PROLDH System



- Slim profile allows driving in corners
- Compact body for reduced weight and easy handling



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Quik Drive® Applications: Underlayment/Backerboard

PRO200S System



- Slim profile allows driving in corners
- Compact body for reduced weight and easy handling



Collated Screws for the Quik Drive® System

Fastener Model	PROCGB	PROLDH	PR0200S
DWF Drywall-to-CFS Screw	11⁄4", 15⁄8"		1 1/4", 1 5%"
DWFSD Drywall-to-CFS Screw Gypsum panel to steel, yellow zinc or Quik Guard® coating	11⁄4", 15⁄8"		1 ¼", 15%"

Connectors for Cold-Formed Steel Construction

Quik Drive® Applications: Steel Decking/Stitching

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The Quik Drive[®] PROSDX150 auto-feed screw driving system is the right choice for steel decking because it provides an efficient fastening solution that is safer and easier than welding or P.A.T.

PROSDX150 System



- Features an extended nosepiece for easy access to valley
- · Stand-up driving increases comfort and productivity
- One system for fastening steel decking to structural members and steel stitching
- No special inspection or certification required as with welding or P.A.T. fastening

BSD200 System



- Innovative Precision Placement[™] nosepiece allows for easy location of the standing seam metal roofing clip holes
- · Stand-up driving increases comfort and productivity
- · Patented belt collation enables auto-feed fastening of screws
- Depth control prevents over driving

Quik Drive® Applications: Steel Decking/Stitching



Collated Screws for the Quik Drive® System

Fastener Model	PROSDX150	BSD200
Strong-Drive® SELF-DRILLING X METAL Screw	1", 11⁄4", 11⁄2"	_
Strong-Drive® SELF-DRILLING X METAL Screw	3⁄4", 1", 1 <i>1</i> ⁄4"	_
Strong-Drive® XL LARGE-HEAD METAL Screw	_	1 1/4"
Strong-Drive® XM MEDIUM-HEAD METAL Screw	1 ¼ª	_

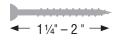
Quik Drive® Applications: Fiber-Cement



PROLDH System



- Slim profile allows driving in corners
- Drives fasteners that meet ANSI standards
- Compact body for reduced weight and easy handling

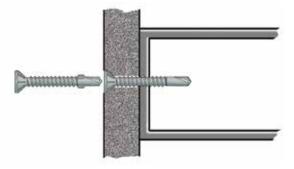


Collated Screws for the Quik Drive® System

Fastener Model	PR0200S	PROCGB	PROLDH	PR0250
CBSDQ Sheathing Screw*	1%"	15%"	_	21⁄4"

* #2 Drill Point with Wings

Wings cut a path, protecting the integrity of the threads and break away before penetrating the steel.



Quik Drive® Applications: Standing-Seam Metal



The Quik Drive[®] PROPP150 auto-feed screw driving system is ideal for standing-seam metal roofing. The Precision Placement[™] nosepiece cuts installation time and the collated fasteners eliminate handling of individual screws.

PROPP150 System



- Innovative Precision Placement nosepiece allows for easy location of the hole in the clip
- Hands-free screw advancement speeds installation
- Collated fastener strips reduce waste and prevent damage to roof panels
- Suitable for panel clips up to 21/2" tall
- Also ideal for fastening panel flanges for snap-and-seam metal roofing and installing trim and drip edge

- 3¼" - 1½" →

Collated Screws for the Quik Drive® System

Fastener Model	PROPP150
PC Standing-Seam-Roofing Panel Clip Screw	1", 1½"
PCSD Standing-Seam-Roofing Panel Clip Screw	1"
Strong-Drive [®] SELF-DRILLING X METAL Screw	1"

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Connectors for Cold-Formed Steel Construction

Quik Drive® Applications: Steel Framing/Stitching



Quik Drive® auto-feed screw driving systems save time when fastening cold-formed steel framing. The collated fastening strips eliminate the handling of individual screws and the attachments hold the screw in place while it drills through the material.

PROPH Attachment



- Compact body for reduced weight and easy handling
- · Slim profile allows driving in corners
- · Long-lasting reliability for targeted applications



PROHX516 Attachment



- Engineered to drive screws for steel fastening
- · Precise depth adjustment prevents over and under driving
- · Long-lasting reliability for targeted applications



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Fasteners and Quik Drive®

Quik Drive® Applications: Steel Framing/Stitching



Collated Screws for the Quik Drive® System

Fastener Model	PROPH	PROHX516
Strong-Drive [®] FPHSD FRAMING-TO-CFS Screw	3⁄4"	
Strong-Drive [®] PHSD FRAMING-TO-CFS Screw	3/4"	_
Strong-Drive® SELF-DRILLING X METAL Screw	_	7⁄8"—1"

Connectors for Cold-Formed Steel Construction

Quik Drive® Applications: Nailers/Ledgers-to-Steel



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Quik Drive[®] PROHSD auto-feed fastening systems are ideal for wood-to-steel fastening. They hold the screw in place while drilling and our self-drilling screws eliminate the need for pre-drilling.

PROHSD60 System



- Specifically engineered for fastening wood-to-steel
- Expanded depth settings for various material thickness
- Broad nose increases stability and protects surfaces



PROHSD75 System



- Specifically engineered for fastening wood-to-steel
- Expanded depth settings for various material thickness
- Broad nose increases stability and protects surfaces

Quik Drive® Applications: Nailers/Ledgers-to-Steel

SIMPSON Strong-Tie

Collated Screws for the Quik Drive® System

Fastener Model	PROHSD60	PROHSD75
Strong-Drive® TB WOOD-TO-STEEL Screw Black phosphate coating	1¾", 2¾" (45, 60 mm)	2¾", 3" (60, 75 mm)
Strong-Drive® TB WOOD-TO-STEEL Screw	1¾", 2¾" (45, 60 mm)	2¾", 3" (60, 75 mm)
Strong-Drive® TB WOOD-TO-STEEL Screw	1¾", 2¾" (45, 60 mm)	2¾", 3" (60, 75 mm)
Strong-Drive® PPSD SHEATHING-TO-CFS Screw	#12 x 1%"	#12 x 1¾", #12 x 3"
Strong-Drive® PPSD SHEATHING-TO-CFS Screw	#12 x 1¾"	#12 x 1¾", #12 x 3"

Concrete Connectors



2

AnchorMate® Anchor Bolt Holders

The reusable AnchorMate[®] anchor bolt holder is designed to hold the anchor in place before the concrete pour, as required in some jurisdictions. The gripping section secures the bolt in place without a nut for quicker setup and teardown. It also protects the threads from wet concrete and simplifies trowel finishing.

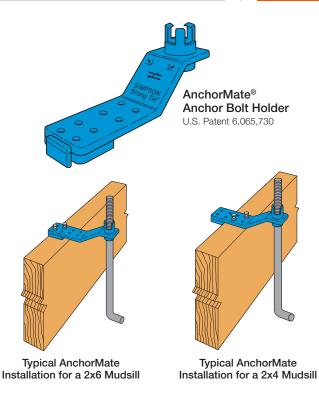
Features:

- Built-in 2x4 and 2x6 stops eliminate measuring.
- Color coded for easy size identification.
- Use the 5%" and 7%" AnchorMate to secure the SSTB to the formboard before the concrete pour. Alignment arrows (left or right) match the SSTB bolt head arrow.

Material: Nylon

Codes: See p. 11 for Code Reference Key Chart

Model No.	Diameter (in.)	Color	Code Ref.
AM1/2	1/2	Yellow	
AM%	5⁄8	Blue	
AM¾	3⁄4	Red	180
AM7⁄8	7⁄8	Green	
AM1	1	Black	



ABS Anchor Bolt Stabilizer

The ABS stabilizes the anchor bolt to prevent it from being pushed against the form during the concrete pour.

Features:

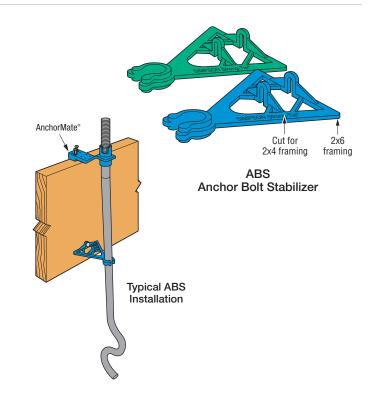
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- Supports the bolt approximately 8" below the top of the concrete
- Model ABS⁵/₈ is for the ⁵/₈" SSTB and ABS⁷/₈ is for the ⁷/₈" SSTB
- Thin section limits the effect of a cold joint
- Sized for 2x4 and 2x6 mudsills

Material: Engineered Composite Plastic

Codes: See p. 11 for Code Reference Key Chart

Model No.	Diameter (in.)	Color	Code Ref.
ABS%	5⁄8	Blue	180
ABS7/8	7⁄8	Green	100



StrapMate® Strap Holder

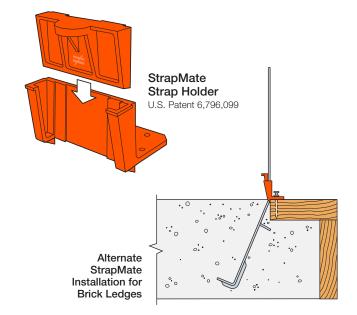
The StrapMate® is designed to keep the STHD and LSTHD straps vertically aligned during the concrete pour to minimize possibility of spalling. The friction fit allows for quick and easy installation.

Features:

- The StrapMate is reusable
- Works with STHD, LSTHD
- Material: Engineered Composite Plastic
- Designed to fit $\ensuremath{\mathscr{Y}}$ " plywood forms up to 1 $\ensuremath{\mathscr{Y}}$ " LVL forms and larger
- The strap is positioned off the front edge of the form board

Codes: See p. 11 for Code Reference Key Chart

Model No.	Nails	Code Ref.
SM1	(2) 8d Duplex	180



ABL Anchor Bolt Locator

The ABL enables the accurate and secure placement of anchor bolts on concrete-deck forms prior to concrete placement. The structural heavy-hex nut is attached to a pre-formed steel "chair," which eliminates the need for an additional nut on the bottom of the anchor bolt. Electro-galvanized versions available for HDG anchor bolts. Order ABL-OST when using HDG anchor bolts.

Features:

Concrete Connectors

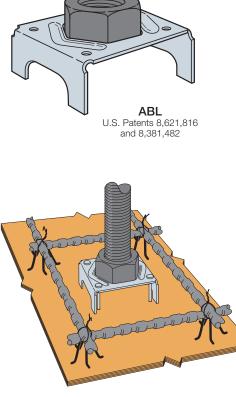
- Designed for optimum concrete flow.
- Installed with nails or screws.
- Meets code requirement for 1" stand off. Also available with 11/2" standoff. Order ABLXX-1.5.
- PAB anchors are not designed for use with the ABL. Contact Simpson Strong-Tie for pre-assembled anchor solutions to be used with ABL.

Material: Nut - Heavy hex; Chair - Steel

Finish: Nut - None or Electro-galvanized; Chair - G90; ABL-OST - HDG

Codes: See p. 11 for Code Reference Key Chart

Model No.	Anchor Bolt Diameter (in.)	Code Ref.
ABL4-1	1/2	
ABL5-1	5⁄8	
ABL6-1	3⁄4	
ABL7-1	7⁄8	180
ABL8-1	1	
ABL9-1	11⁄8	
ABL10-1	11⁄4	



Typical ABL Installation

BP/LBP Bearing Plates

Bearing plates give greater bearing surface than standard cut washers, and help distribute the load at these critical connections.

The BP½-3 and BP5⁄-3 are 3" x 3" bearing plates that meet the latest requirements of the IRC and IBC. These plate washers are available uncoated or with a hot-dip galvanized (HDG) coating.

The BPS and LBPS are bearing plates that offer increased flexibility. The slotted hole allows for adjustability to account for bolts that are not in the middle of the track-bottom plate.

Material: See table

Finish: LBP, LBPS and BP%S — Galvanized;
BP%-2 and BP%S — Zinc Plated; BPS, BP — None.
BPs and BPSs may be ordered HDG;
LBP and LBPS products may be ordered ZMAX®; contact
Simpson Strong-Tie. Refer to pp. 18–21 for Corrosion Information.

Installation:

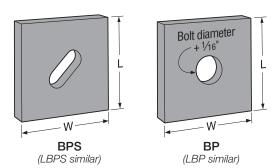
- See General Notes on pp. 14–16.
- BP/BPS For shearwall applications, position edge of plate washer within ½" of sheathed edge of track-bottom plate.

Codes: See p. 11 for Code Reference Key Chart

Bolt Dia.	Model	Thickness	Dimensi	Code		
(in.)	No.	THICKNESS	W	L	Ref.	
3⁄8	BP%-2	3⁄16"	2	2	l1, FL	
	LBP1/2	9⁄64''	2	2		
	LBPS1/2	9⁄64''	3	3		
1/2	BPS1/2-3	3 ga.	3	3	190	
72	BPS1/2-6	3 ga.	3	41⁄2		
	BP1/2	3⁄16"	2	2		
	BP1⁄2-3	3 ga.	3	3	l1, FL	
	LBP%	9⁄64"	2	2	190	
	LBPS%	9⁄64"	3	3		
	BPS%-3	3 ga.	3	3		
5⁄8	BPS%-6	3 ga.	3	41⁄2		
	BP%-2	3⁄16"	2	2	11, FL	
	BP%	1⁄4"	21⁄2	21⁄2	180	
	BP%-3	3 ga.	3	3		
	BP¾	5⁄16"	2¾	2¾	11, FL	
3/4	BP¾-3	3 ga.	3	3		
94	BPS¾-3	3 ga.	3	3		
	BPS¾-6	3 ga.	3	41⁄2		
7/8	BP%-2	3⁄8"	1 ¹⁵ ⁄16	21⁄4	190	
'/8	BP7∕s	5⁄16"	3	3		
1	BP1	3⁄8"	31⁄2	31⁄2		

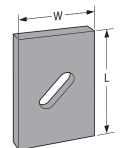
These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. Standard cut washer required with BPS½-3, BPS¾-3, BPS½-6, BPS¾-6 and BPS¾-6 (not provided) per the 2015 IRC and 2015 SPDWS.

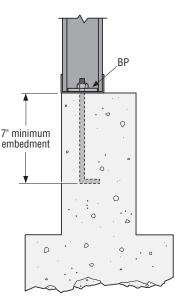


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BPS1/2-6 (other models similar)



Typical BP Installed with a Bottom Track Anchor Bolt

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CNW Coupler Nuts

Simpson Strong-Tie® coupler nuts are a tested and load-rated method to join threaded rod and anchor bolts. "Witness" holes in the nut provide a means to verify when rods are properly installed. The positive stop feature helps ensure even threading into each end of the nut. CNWs meet and exceed the specified minimum tensile capacity of corresponding ASTM A36 bolts and threaded rod. HSCNWs meet and exceed the specified minimum tensile capacity of corresponding ASTM A449 bolts and threaded rod. Contact Simpson Strong-Tie for other coupler nut sizes.

Finish: Zinc Plated

Installation:

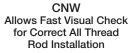
- Tighten the two rods until each all-thread rod is visible in the witness hole. Any portion of the thread visible in the witness is a correct installation.
- Standard CNW for use with non-hot-dip galvanized all-thread rod only.
- 5%" and 7%" diameter couplers available with oversized threads for installation to hot-dip galvanized bolts (order CNW%-%OST and CNW%-%OST). Note that only one side is oversized to accommodate HDG rods and bolts.
- · Some OST couplers are typically oversized on one end of the coupler nut only and will be marked with an "O" on the oversized side. Couplers may be oversized on both ends. Contact Simpson Strong-Tie.

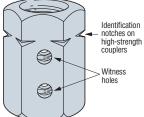
Codes: See p. 11 for Code Reference Key Chart

Model No.	Rod Diameter	H Min.	Allowable Tension Capacity (lb.)	Code Ref.			
NO.	(in.)	(in.)	(100)				
CNW1/2	0.500	1 1/2	4,265				
CNW5/8	0.625	1%	6,675	11, FL			
CNW¾	0.750	21⁄4	9,610				
CNW7/8	0.875	21⁄2	13,080				
CNW1	1.000	2¾	17,080				
CNW11/4	1.250	3	26,690	170			
HSCNW3/4	0.750	21⁄4	19,880				
HSCNW1	1.000 234 35,345						
Transition Couplers							
CNW5/8 -1/2	0.625 to 0.500	1½	4,265				
CNW3⁄4 -5⁄8	0.750 to 0.625	1¾	6,675	11, FL			
CNW% -%	0.875 to 0.625	2	6,675	170			
CNW1-7/8	1.000 to 0.875	21⁄4	13,080	170			

1. Allowable loads shown are based on AISC 14th Edition A36 and A449 (HS) threaded rod capacities.



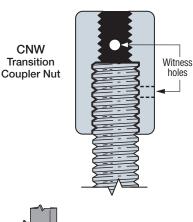


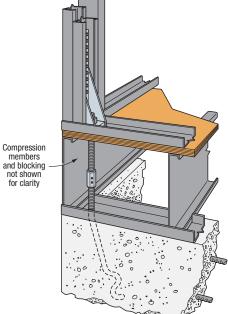


HSCNW

Coupler Nut

High-Strength





Typical CNW Rim Joist Installation

LCB Column Base

SIMPSON Strong-Tie

LCB column bases use screws, which allows for fast installation, reduced reveal and high capacity, while maintaining the net section of the column.

Material: Strap - 97 (12 ga.) x2; Base - 54 (16 ga.)

Finish: LCB - galvanized.

Installation:

- Use all specified fasteners; see General Notes
- For full loads, minimum side cover required is 2" for LCB
- Install all models with bottom of base plate flush with concrete
- Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports)

Codes: See p. 11 for Code Reference Key Chart

Installation for cold-formed steel built-up column.

2" min.

side

cover

Ď

Note: The engineer of record is responsible for design of column.

LCB44 (LCB46 and LCB66 similar)

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71/2

Allowable Loads for LCB Column Base

Nominal Model Column	Dimensions		Column	Allowable Load (lb.)							
	(in.)			Wind and SDC A&B		SDC C-F		Code			
No.	Size (in.)	-	D	Fasteners ⁷	Non-Cracked Cracked		Non-Cracked Cracked		Ref.		
	()		D		Uplift	Uplift	Uplift	Uplift			
LCB44	3.5 x 3.5	3%16	31⁄2	6½	(12) #10						
LCB46	3.5 x 5.5	3%16	5½	6½		1,170	820	985	690	170	
LCB66	5.5 x 5.5	5½	5½	5½							

1. Loads may not be increaed for short-term loading.

2. Concrete shall have a minimum compressive strength, $f'_{c} = 2,500$ psi.

3. Multiply Seismic and Wind ASD load values by 1.4 or 1.6 respectively to obtain LRFD capacities.

 In accordance with IBC Section 1613.1, detached one- and two- family dwellings in Seismic Design Category (SDC) C may use "Wind and SDC A&B" allowable loads.

Post bases do not provide adequate resistance to prevent members from rotating about the base and therefore are not recommended for non top-supported installations (such as fences or unbraced carports).

6. Designer is responsible for concrete foundation design.

7. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

2" min.

side

cover

3" min. cover

MASA/MASAP Mudsill Anchors

Mudsill anchors have always been a time-saving alternative to anchor bolts, and the MASA anchors provide even greater load-carrying capacity alternative. For %" and ½" mudsill anchor bolts on bottom tracks, the MASA has load capacities that meet or exceed the parallel and perpendicular to plate shear capacity of other cast-in-place anchors. Two versions of the MASA are available — the standard MASA for installation on standard forms, and the MASAP for panelized forms.

The MASA and MASAP are code listed by ICC-ES under the 2006, 2009, 2012 and 2015 IBC[®] and IRC[®] and have been tested to meet the requirements of ICC-ES acceptance criteria AC-398 for cracked and uncracked concrete.

Material: 16 gauge

Finish: Galvanized, all available in ZMAX[®] coating. See Corrosion Information, pp. 18–21.

Installation:

Concrete Connectors

• Use all specified fasteners; see General Notes

MASA/MASAP

- Concrete shall have a minimum f'_c = 2,500 psi.
- Spalling Full loads apply for spalls up to a maximum height of 11⁴" and a maximum depth of 7⁶". Any exposed portion of the mudsill anchor must be protected against possible corrosion.
- Minimum MASA end distance is 4" and minimum center-to-center spacing is 8" for a full load.
- For continuous load path, MASA should be installed on the same side of the wall as uplift connectors.

Codes: See p. 11 for Code Reference Key Chart

Embedment line (top of concrete) U.S. Patent D656,391S

Strong

U.S. Patent 8,484,917

Allowable Loads for MASA/MASAP Cast-in-Place Mudsill Anchor on CFS Track

Model No.	Fasteners ⁸		Allowable Load (lb.) ^{1,2,3,4,5} 43 mil (18 ga.) CFS						
		Top of Track	Standard Installation						Cada
	Sides		Wind and SDC A&B ⁶			SDC C-F			Code Ref.
			Uplift	Parallel to Track ⁷	Perpendicular to Track	Uplift	Parallel to Track ⁷	Perpendicular to Track	
MASA or (S MASAP (S			Non-Cracked						
	(3) #10	(6) #10	675	1,205	895	565	1,010	750	14, L7, FL
	(3) #10		Cracked						14, L/, FL
				510	1,205	655	425	1,010	550

1. Allowable loads are governed by tests and may not be increased ($C_D = 1.0$).

2. The tabulated allowable (ASD) loads may be multiplied by 1.6 for designs for wind and in SDC A&B, and

by 1.4 for designs in SDC C through F to obtain the LRFD loads.

3. Minimum concrete compression strength, $\rm f^{\prime}_{\rm C}$ is 2,500 psi.

4. Allowable loads are based on a minimum stemwall width of 6".

5. For simultaneous loads in more than one direction, the connector must be evaluated using the Unity Equation.

6. Per Section 1613 of the 2009, 2012 and 2015 IBC, detached one- and two-family dwellings in SDC C may

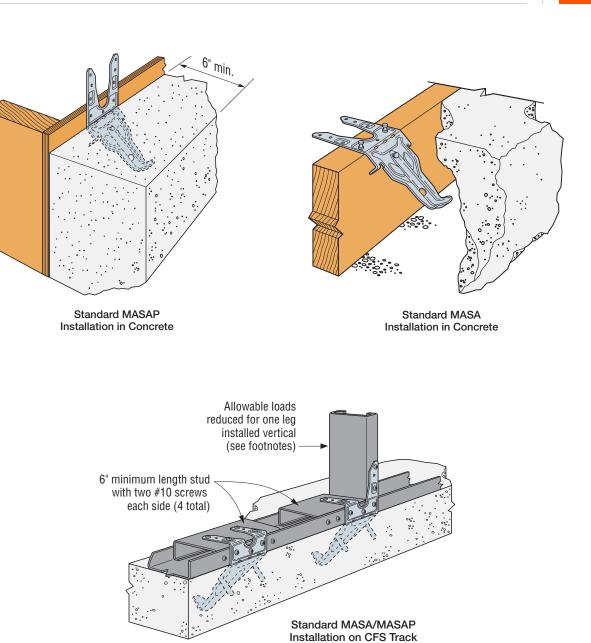
use the "Wind and SDC A&B" allowable loads.

7. Parallel-to-Track loads for One-Leg-Up Installation: SDC A-C = 1,025 lb., SDC C-F = 860 lb.

8. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

Connectors for Cold-Formed Steel Construction

MASA/MASAP Mudsill Anchors

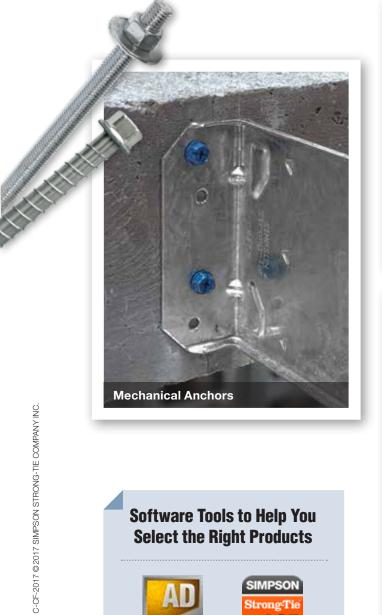




Connectors for Cold-Formed Steel Construction

Anchor Systems

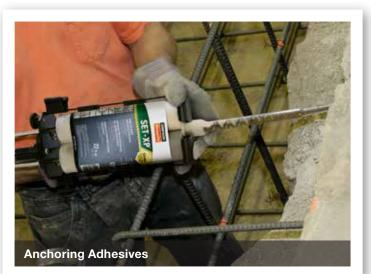
Simpson Strong-Tie® Anchor Systems manufactures a full array of anchoring and fastening products for concrete and masonry — including adhesives for anchoring threaded rod and rebar, mechanical anchors, powder- and gas-actuated fastening systems and accessories. These products offer unique solutions to applications in light-framed construction when used with and without Simpson Strong-Tie connectors.



Software Tools to Help You Select the Right Products



For more information, visit strongtie.com/software.





SET-XP[®] epoxy anchoring adhesive is a high-strength formula for anchoring and doweling in cracked and uncracked concrete and masonry applications. It is a two-part system with the resin and hardener being simultaneously dispensed and mixed through the mixing nozzle. When properly mixed, adhesive will be a uniform teal color for easy post-installation identification.

Features:

- 1:1 two-component, high-solids, epoxy-based anchoring adhesive formula
- Passed the demanding ICC-ES AC308 adverse-condition tests pertaining to elevated temperatures and long-term sustained loads
- Code listed under the IBC/IRC for cracked and uncracked concrete per ICC-ES ESR-2508
- Code listed under the IBC/IRC for masonry per IAPMO UES ER-265
- Suitable for use under static and seismic loading conditions in cracked and uncracked concrete and masonry
- Cure times: 24 hours at 70°F, 72 hours at 50°F
- Easy hole-cleaning no power-brushing required
- Suitable for use in dry or water-saturated concrete
- For best results, store between 45°F and 90°F
- Available in 8.5 oz., 22 oz. and 56 oz. cartridges for application versatility
- Manufactured in the USA using global materials

Applications:

Anchors

- Threaded rod anchoring and rebar doweling into concrete and masonry
- Suitable for horizontal, vertical and overhead applications
- Multiple DOT listings refer to strongtie.com/DOT for current approvals

Codes: ICC-ES ESR-2508 (concrete); IAPMO UES ER-265 (masonry); City of L.A. RR25744 (concrete), RR25965 (masonry); Florida FL-17449.2 (concrete), FL-16230.3 (masonry); AASHTO M-235 and ASTM C881 (Type I and IV, Grade 3, Class C); NSF/ ANSI Standard 61 (216 in.²/1,000 gal.)

Installation and Application Instructions:

- Surfaces to receive epoxy must be clean.
- Base material temperature must be 50°F or above at the time of installation. For best results, material should be between 70°F and 80°F at time of application.
- To warm cold material, store cartridges in a warm, uniformly heated area or storage container. Do not immerse cartridges in water to facilitate warming.
- Mixed material in nozzle can harden in 30 minutes at temperatures of 70°F and above.

Suggested Specifications:

See **strongtie.com** for more information.



Strong

SET-XP[®] Adhesive

Test Criteria

Anchors installed with SET-XP[®] adhesive have been tested in accordance with ICC-ES Acceptance Criteria for Post-Installed Adhesive Anchors in Masonry Elements (AC58) and Adhesive Anchors in Concrete Elements (AC308).

Property	Test Method	Result*
Consistency	ASTM C881	Passed, non-sag
Glass transition temperature	ASTM E1356	155°F
Bond strength (moist cure)	ASTM C882	3,742 psi at 2 days
Water absorption	ASTM D570	0.10%
Compressive yield strength	ASTM D695	14,830 psi
Compressive modulus	ASTM D695	644,000 psi
Shore D Durometer	ASTM D2240	84
Gel time	ASTM C881	49 minutes
Volatile Organic Compound (VOC)	—	3 g/L

*Material and curing conditions: 73 \pm 2°F, unless otherwise noted.

SET-XP[®] Cartridge System

Model No.	Capacity (ounces)	Cartridge Type	Carton Quantity	Dispensing Tool(s)	Mixing Nozzle
SET-XP10 ⁴	8.5	Single	12	CDT10S	
SET-XP22-N⁵	22	Side-by-side	10	EDT22S, EDTA22P, EDTA22CKT	EMN22i
SET-XP56	56	Side-by-side	6	EDTA56P	

1. Cartridge estimation guidelines are available at strongtie.com/apps.

2. Detailed information on dispensing tools, mixing nozzles and other adhesive accessories is available at strongtie.com.

3. Use only Simpson Strong-Tie® mixing nozzles in accordance with Simpson Strong-Tie instructions.

Modification or improper use of mixing nozzle may impair SET-XP adhesive performance.

4. Two EMN22i mixing nozzles and two nozzle extensions are supplied with each cartridge.

5. One EMN22i mixing nozzle and one nozzle extension are provided with each cartridge.

Cure Schedule

Base N Tempe	Cure Time (hrs.)			
°F	°C	(113.)		
50	10	72		
60	16	48		
70	21	24		
90	32	24		
110	43	24		

For water-saturated concrete, the cure times must be doubled.



$\mathsf{SET-XP}^\circ-\mathsf{Tension}$ Loads for Threaded Rod in Normal-Weight Concrete

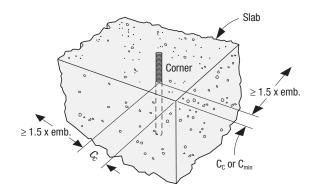
				Tension Load ^{6,13} (lb.)										Tension Loads ¹⁰	
Anchor Dia.	Drill Bit	Emb. Depth	Critical Edge (C _C) + Critical End (C _C)					М	inimum Edg	e (C _{min}) + C	ritical End (C	;c)	F _u = 58 ksi Steel (lb.)		
(in.)	(in.)	(in.)	Cc	LRFD ¹ (Seismic)	LRFD ¹¹ (Wind)	ASD ^{1,2} (Seismic)	ASD ^{2,11} (Wind)	Cmin	LRFD ¹ (Seismic)	LRFD ¹¹ (Wind)	ASD ^{1,2} (Seismic)	ASD ^{2,11} (Wind)	LRFD	ASD	
		4	6	2,085	2,780	1,460	1,670	1¾	1,080	1,445	755	865			
1⁄2	5⁄8	7	8	4,700	6,175 ⁹	3,290	3,705	1¾	2,305	3,075	1,615	1,845	6,175	4,265	
		10	8	6,175 ⁹	6,175 ⁹	4,325	3,705	1¾	3,295	4,395	2,305	2,635			
		5	8	3,555	4,740	2,490	2,845	1¾	1,665	2,220	1,165	1,330			
5⁄8	3⁄4	81⁄2	8	6,040	8,055	4,230	4,835	1¾	2,830	3,770	1,980	2,260	9,830	6,675	
		12	9	8,235	9,830 ⁹	5,765	5,900	1¾	4,005	5,340	2,805	3,205			
		6	9	4,415	5,885	3,090	3,530	1¾	2,030	2,705	1,420	1,625			
3⁄4	7⁄8	101⁄2	12	7,985	10,650	5,590	6,390	1¾	3,595	4,795	2,515	2,875	14,530	9,615	
		15	15	11,410	14,530 ⁹	7,985	8,720	1¾	5,135	6,845	3,595	4,105			
		8	10	5,045	8,410	3,530	5,045	1¾	2,270	3,780	1,590	2,270			
7⁄8	1	12	12	7,855	13,090	5,500	7,855	1¾	3,450	5,750	2,415	3,450	20,095	13,070	
	17½	14	11,455	19,095	8,020	11,455	1¾	5,035	8,390	3,525	5,035				
		8	12	6,690	9,695	4,685	5,815	1¾	2,895	4,190	2,025	2,515			
1	1 1⁄8	14	18	11,705	16,965	8,195	10,180	1¾	5,060	7,335	3,540	4,400	26,360	17,075	
		20	23	16,720	24,235	11,705	14,540	1¾	7,230	10,480	5,060	6,290		,	

Note: Reference p. 185 for footnotes.

SET-XP[®] — Shear Loads for Threaded Rod in Normal-Weight Concrete

									Sh	ear Load ⁶ (lb./	ft.)		
Anchor Dia.	Drill Bit	Edge Distance	End Distance	Emb. Depth	Concrete Thickness	Spacing		Conci	rete ^{4,13}		Cold-	Formed Steel	(ASD)⁵
(in.)	(in.)	(in.)	(in.)	(in.)	(in.)	5,555,5	LRFD ¹ (Seismic)	LRFD ¹¹ (Wind)	ASD ^{1,2} (Seismic)	ASD ^{2,11} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)
						0' - 8"	1,825	1,825	1,280	1,095	895	1,320	2,485
						1'- 0"	1,215	1,215	850	730	595	880	1,655
						1'- 4"	910	910	635	545	445	660	1,240
1⁄2	5⁄8	1¾	5	4	6½	2'-0"	610	610	425	365	300	440	830
						2' - 8"	455	455	320	275	225	330	620
						4' - 0"	305	305	215	185	150	220	415
						6' - 0"	205	205	145	125	100	145	275
						0' - 8"	2,035	2,035	1,425	1,220	960	1,490	2,995
						1' 0"	1,355	1,355	950	815	640	995	2,000
					5 8½	1'- 4"	1,015	1,015	710	610	480	745	1,500
5⁄8	3⁄4	1¾	5	5		2' – 0"	680	680	475	410	320	495	1,000
						2'-8"	510	510	355	305	240	375	750
						4' - 0"	340	340	240	205	160	250	500
						6' - 0"	225	225	160	135	105	165	335
						0' - 8"	2,190	2,190	1,535	1,315	965	1,600	3,320
						1'- 0"	1,460	1,460	1,020	875	640	1,065	2,215
	% % 1% 5				1'- 4"	1,095	1,095	765	655	480	800	1,660	
3⁄4		5	6	10	2' – 0"	730	730	510	440	320	535	1,110	
					2' - 8"	550	550	385	330	240	400	830	
				4' - 0"	365	365	255	220	160	265	555		
						6' – 0"	245	245	170	145	105	180	370

- 1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.
- Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.
- Load values based on the following: minimum concrete strength of 2,500 psi, dry hole, maximum short-term temperature of 150°F, maximum long-term temperature of 110°F, and continuous special inspection. Reference ICC-ES ESR-2508 for further information.
- 4. Shear load is applied parallel to the edge of concrete. Anchor is considered as an individual anchor without influence from other anchors.
- 5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD, multiply value by 1.5.
- 6. Governing shear load is the lesser of concrete and CFS. Governing tension load is the lesser of concrete and steel.
- For conditons not covered by this table, use the Simpson Strong-Tie[®] Anchor Designer[™] software available at strongtie.com.
- 8. Third- and fourth-edge distances must be \geq 1.5 x Embedment Depth.
- 9. Failure mode governed by ductile steel rod (F1554 Grade 36).
- 10. LRFD steel strength based on ACI 318-14 Chapter 17. ASD steel strength based on AISC Steel Construction Manual, 13th Edition, $F_u = 58$ ksi.
- 11. Wind design includes SDC A&B.
- 12. Minimum concrete thickness must be \geq embedment depth + 5 x anchor diameter.
- 13. Designer shall consider the design requirements of ACI 318-14 Sections 17.2.3.4.3 and 17.2.3.5.3, where applicable.



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Strong-Tie

AT-XP® High-Strength Acrylic Adhesive

Formulated for high-strength anchorage of threaded rod and rebar into cracked and uncracked concrete and masonry under a wide range of conditions, AT-XP[®] adhesive dispenses easily in cold or warm environments and in below-freezing temperatures with no need to warm the cartridge. When mixed properly, this low-odor formula is a dark teal color for easy post-installation identification.

Features:

- Passed the demanding ICC-ES AC308 adverse-condition tests pertaining to reduced and elevated temperatures and long-term sustained loads
- Code listed under the IBC/IRC for cracked and uncracked concrete per IAPMO UES ER-263 and City of L.A. RR25960
- Code listed under the IBC/IRC for masonry per IAPMO UES ER-281 and City of L.A. RR25966
- 10:1 two-component high-strength, acrylic-based anchoring adhesive
- Suitable for use under static and seismic loading conditions in cracked and uncracked concrete as well as masonry
- Easy hole-cleaning procedure no power-brushing required
- Suitable for use in dry or water-saturated concrete
- For best results, store between 14°F and 80°F
- Cures in substrate temperatures as low as 14°F (-10°C) in 24 hours or less
- Available in 9.4 oz., 12.5 oz. and 30 oz. cartridges for application versatility
- Volatile Orgainic Compound (VOC) 30 g/L
- Manufactured in the USA using global materials

Applications:

Anchors

- Threaded rod anchoring and rebar doweling into concrete and masonry
- · Suitable for horizontal, vertical and overhead applications

Codes: IAPMO UES ER-263 (concrete); IAPMO UES ER-281 (masonry); City of L.A. RR25960 (concrete), RR25966 (masonry); FL-16230.1; NSF/ANSI Standard 61 (43.2 in.²/1,000 gal.)

Installation and Application Instructions:

- Surfaces to receive adhesive must be clean.
- Base material temperature must be 14°F or above at the time of installation. For best results, material should be 14–80°F at time of application.
- To warm cold material, store cartridges in a warm, uniformly heated area or storage container. Do not immerse cartridges in water to facilitate warming.
- Mixed material in nozzle can harden in 3–4 minutes at temperatures of 70°F and above.

Suggested Specifications

See **strongtie.com** for more information.



Cracked Concrete Strong



AT-XP[®] Adhesive

AT-XP® High-Strength Acrylic Adhesive

AT-XP® Adhesive Cartridge System

Model No.	Capacity ounces (cubic in.)	Cartridge Type	Carton Qty.	Dispensing Tool	Mixing Nozzle
AT-XP10	9.4 (16.9)	Coaxial	6	CDT10S	
AT-XP13	12.5 (22.5)	Side-by-side	10	ADT813S	AMN19Q
AT-XP30	30 (54)	Side-by-side	5	ADT30S ADTA30P or ADTA30CKT	

1. Cartridge estimation guidelines are available at strongtie.com/apps.

Detailed information on dispensing tools, mixing nozzles and other adhesive accessories is available at strongtie.com.

 Use only Simpson Strong-Tie[®] mixing nozzles in accordance with Simpson Strong-Tie instructions. Modification or improper use of mixing nozzle may impair AT-XP adhesive performance.

4. One AMN19Q mixing nozzle and one nozzle extension are supplied with each cartridge.

Cure Schedule

Base Materia	Temperature	Cure Time
°F	°C	(hrs.)
14	-10	24
32	0	8
50	10	3
68	20	1
86	30	30 min.
100	38	20 min.

For water-saturated concrete, the cure times must be doubled.

Anchors		
		icho

AT-XP[®] — Tension Loads for Threaded Rod In Normal-Weight Concrete

							Tension L	oad ^{6,13} (lb.)				Tension Load ¹⁰	
Anchor Dia.	Drill Bit	Emb. Depth	Critical Edge (C_C) + Critical End (C_C)					Minimum Edge (C _{min}) + Critical End (C _c)					F _u = 58 ksi Steel (lb.)	
(in.)	(in.) (in.) (in.)	Cc	LRFD ¹ (Seismic)	LRFD ¹¹ (Wind)	ASD ^{1,2} (Seismic)	ASD ^{2,11} (Wind)	C _{min}	LRFD ¹ (Seismic)	LRFD ¹¹ (Wind)	ASD ^{1,2} (Seismic)	ASD ^{2,11} (Wind)	LRFD	ASD	
		4	6	2,680	4,210	1,875	2,525	1¾	1,365	2,140	955	1,285		
1⁄2	5⁄8	7	8	4,715	6,175 ⁹	3,300	3,705	1¾	2,395	3,760	1,675	2,255	6,175	4,265
		10	8	6,175 ⁹	6,175 ⁹	4,325	3,705	1¾	3,425	5,370	2,400	3,220		
		5	8	3,985	6,175	2,790	3,705	1¾	1,870	2,930	1,310	1,760		
5⁄8	3⁄4	81⁄2	8	6,040	8,055	4,230	4,835	1¾	3,185	4,695	2,230	2,815	9,830	6,675
		12	9	8,235	9,830 ⁹	5,765	5,900	1¾	4,495	6,890	3,145	4,135		
		6	9	5,180	8,120	3,625	4,870	1¾	2,420	3,680	1,695	2,210		
3⁄4	7⁄8	10½	12	9,740	13,545	6,820	8,125	1¾	4,355	6,745	3,050	4,045	14,530	9,615
		15	15	13,915	14,530 ⁹	9,740	8,720	1¾	6,220	9,755	4,355	5,855		
		8	10	4,550	9,980	3,185	5,990	1¾	2,130	4,810	1,490	2,885		
7⁄8	1	12	12	7,735	14,355	5,415	8,615	1¾	3,350	7,565	2,345	4,540	20,095	13,070
		17½	14	11,275	20,095 ⁹	7,890	12,055	1¾	4,885	11,035	3,420	6,620		
		8	12	6,850	10,750	4,795	6,450	1¾	2,960	4,640	2,070	2,785		
1	1 1/8	14	18	12,335	19,350	8,635	11,610	1¾	5,235	8,215	3,665	4,930	26,360	17,075
		20	23	17,625	26,360 ⁹	12,340	15,815	1¾	7,480	11,735	5,235	7,040		

Note: Reference p. 188 for footnotes.

AT-XP® High-Strength Acrylic Adhesive



AT-XP[®] — Shear Loads for Threaded Rod in Normal-Weight Concrete

					Concrete Thickness (in.)				Sh	ear Load ⁶ (lb./ ⁻	ft.)				
Anchor Dia.	Drill Bit	Edge Distance	End Distance	Emb. Depth		Spacing		Conci	rete ^{4,13}		Cold-	Formed Steel (ASD)⁵		
(in.)	(in.)	(in.)	(in.)	(in.)			LRFD ¹ (Seismic)	LRFD ¹¹ (Wind)	ASD ^{1,2} (Seismic)	ASD ^{2,11} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)		
						0' - 8"	1,825	1,825	1,280	1,095	895	1,320	2,485		
						1'- 0"	1,215	1,215	850	730	595	880	1,655		
	1/2 5% 13% 5			1'- 4"	910	910	635	545	445	660	1,240				
1⁄2		4	6½	2' – 0"	610	610	425	365	300	440	830				
						2'-8"	455	455	320	275	225	330	620		
						4' – 0"	305	305	215	185	150	220	415		
						6' – 0"	205	205	145	125	100	145	275		
			5			0' - 8"	2,035	2,035	1,425	1,220	960	1,490	2,995		
						1'- 0"	1,355	1,355	950	815	640	995	2,000		
								1'- 4"	1,015	1,015	710	610	480	745	1,500
5⁄8	3⁄4	1¾		5	5 8½	2'-0"	680	680	475	410	320	495	1,000		
						2'-8"	510	510	355	305	240	375	750		
						4' – 0"	340	340	240	205	160	250	500		
						6'-0"	225	225	160	135	105	165	335		
						0'-8"	2,190	2,190	1,535	1,315	965	1,600	3,320		
						1'- 0"	1,460	1,460	1,020	875	640	1,065	2,215		
3⁄4 7⁄8 13⁄4					1'- 4"	1,095	1,095	765	655	480	800	1,660			
	1¾	5	6	10	2'-0"	730	730	510	440	320	535	1,110			
			5 0		2'-8"	550	550	385	330	240	400	830			
					4' - 0"	365	365	255	220	160	265	555			
				6' – 0"	245	245	170	145	105	180	370				

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.
 Load values based on the following: minimum concrete strength of 2,500 psi, dry hole, maximum short-term temperature of 180°F,

maximum long-term temperature of 110°F, and continuous special inspection. Reference IAPMO UES ER-263 for further information.

Shear load is applied parallel to the edge of concrete. Anchor is considered as an individual anchor without influence from other anchors.

Shear load is applied parallel to the edge of concrete. Anchor is considered as an individual anchor without initial control of other anchors.
 Cold-formed steel (CFS) shear values are based on AISI S-100, Eq. E3.3.1-1, m_f = 0.75, Ω = 2.5. Reference General Notes for CFS properties. To convert from ASD to LRFD, multiply value by 1.5.

6. Governing shear load is the lesser of concrete and CFS. Governing tension load is the lesser of concrete and steel.

7. For conditons not covered by this table, use the Simpson Strong-Tie Anchor Designer[™] software available at strongtie.com.

8. Third- and fourth-edge distances must be \geq 1.5 x Embedment Depth.

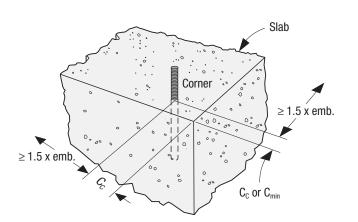
9. Failure mode governed by ductile steel rod (A307 Grade C).

10. LRFD steel strength based on ACI 318-14 Chapter 17. ASD steel strength based on AISC Steel Construction Manual, 13th Edition, Fu = 58 ksi.

11. Wind design includes SDC A&B.

12. Minimum concrete thickness must be \geq embedment depth + 5 x anchor diameter.

13. Designer shall consider the design requirements of ACI 318-14 Section 17.2.3.5.3, where applicable.



The original high-strength screw anchor for use in cracked and uncracked concrete, as well as uncracked masonry. The Titen HD offers low installation torque and outstanding performance. Designed and tested in dry, interior, non-corrosive environments or temporary outdoor applications, the Titen HD demonstrates industry-leading performance even in seismic conditions.

Features:

- Code listed under IBC/IRC in accordance with ICC-ES AC193 for cracked and uncracked concrete per ICC-ES ESR-2713
- Code listed under IBC/IRC in accordance with ICC-ES AC106 for masonry per ICC-ES ESR-1056
- · Qualified for static and seismic loading conditions
- Thread design undercuts to efficiently transfer the load to the base material
- Standard fractional sizes
- Specialized heat-treating process creates tip hardness for better cutting without compromising the ductility
- No special drill bit required designed to install using standard-sized ANSI tolerance drill bits
- Testing shows the Titen HD installs in concrete with 50% less torque than competitor anchors
- Hex-washer head requires no separate washer and provides
 a clean installed appearance
- Removable ideal for temporary anchoring (e.g., formwork, bracing) or applications where fixtures may need to be moved (reuse of the anchor to achieve listed load values is not recommended)

Codes: ICC-ES ESR-2713 (concrete); ICC-ES ESR-1056 (masonry); City of L.A. RR25741 (concrete), RR25560 (masonry); Florida FL-15730.6; FM 3017082, 3035761 and 3043442; Multiple DOT listings

Material: Carbon steel

Coating: Zinc plated or mechanically galvanized

Installation:

- A Holes in metal fixtures to be mounted should match the diameter specified in the table on p. 192.
- Use a Titen HD screw anchor one time only installing the anchor multiple times may result in excessive thread wear and reduce load capacity.
- Do not use impact wrenches to install into hollow CMU.

Caution: Oversized holes in base material will reduce or eliminate the mechanical interlock of the threads with the base material and reduce the anchor's load capacity.

- Drill a hole in the base material using a carbide drill bit the same diameter as the nominal diameter of the anchor to be installed.
 Drill the hole to the specified embedment depth plus minimum hole depth overall (see table below right) to allow the thread tapping dust to settle, and blow it clean using compressed air. (Overhead installations need not be blown clean.) Alternatively, drill the hole deep enough to accommodate embedment depth and the dust from drilling and tapping.
- Insert the anchor through the fixture and into the hole.
- Tighten the anchor into the base material until the hex-washer head contacts the fixture.



Titen HD Screw Anchor U.S. Patents 5,674,035 and 6,623,228 **1/4" Titen HD** Screw Anchor U.S. Patents 5,674,035 and 6,623,228



Serrated teeth on the tip of the Titen HD screw anchor facilitate cutting and reduce installation torque.



Titen HD[®] Anchor Product Data — Zinc Plated

	Size	Madal Na	Drill Bit	Wrench	Qua	ntity
	(in.)	Model No.	Diameter (in.)	Size (in.)	Box	Carton
	1⁄4 x 1 7⁄8	THDB25178H	1⁄4	3⁄8	100	500
	1⁄4 x 23⁄4	THDB25234H	1⁄4	3⁄8	50	250
톟	1⁄4 x 3	THDB25300H	1⁄4	3⁄8	50	250
	1⁄4 x 31⁄2	THDB25312H	1⁄4	3⁄8	50	250
	1⁄4 x 4	THDB25400H	1⁄4	3⁄8	50	250
	3∕8 X 13⁄4	THD37134H*	3⁄8	9⁄16	50	250
	3∕8 x 21⁄2	THD37212H*	3⁄8	9⁄16	50	200
	3∕8 x 3	THD37300H	3⁄8	9⁄16	50	200
	3∕8 x 4	THD37400H	3⁄8	9⁄16	50	200
	3∕8 x 5	THD37500H	3⁄8	9⁄16	50	100
	3∕8 x 6	THD37600H	3⁄8	9⁄16	50	100
	½ x 3	THD50300H	1⁄2	3⁄4	25	100
	½ x 4	THD50400H	1⁄2	3⁄4	20	80
	½ x 5	THD50500H	1⁄2	3⁄4	20	80
	½ x 6	THD50600H	1⁄2	3⁄4	20	80
	1⁄2 x 61⁄2	THD50612H	1⁄2	3⁄4	20	40
	½ x 8	THD50800H	1⁄2	3⁄4	5	25
	½ x 12	THD501200H	1⁄2	3⁄4	5	25
	½ x 13	THD501300H	1⁄2	3⁄4	5	25
	½ x 14	THD501400H	1⁄2	3⁄4	5	25
	½ x 15	THD501500H	1⁄2	3⁄4	5	25
	5∕8 x 4	THDB62400H	5⁄8	15/16	10	40
	%x5	THDB62500H	5⁄8	15/16	10	40
	5% x 6	THDB62600H	5⁄8	15/16	10	40
	5% x 61∕₂	THDB62612H	5⁄8	15/16	10	40
	5∕8 x 8	THDB62800H	5⁄8	15/16	10	20
	3⁄4 x 4	THD75400H	3⁄4	11/8	10	40
	³∕4 x 5	THD75500H	3⁄4	1 1/8	5	20
	3⁄4 x 6	THDT75600H	3⁄4	1 1⁄8	5	20
	3⁄4 x 7	THD75700H	3⁄4	11/8	5	10
	3⁄4 X 81⁄2	THD75812H	3⁄4	1 1/8	5	10
	³⁄₄ x 10	THD75100H	3⁄4	1 1⁄8	5	10

*These models do not meet minimum embedment depth requirements for strength design and require maximum installation torque of 25 ft.–lb. using a torque wrench, driver drill or cordless ¼" impact driver with a maximum permitted torque rating of 100 ft.–lb.

Titen HD[®] Anchor Product Data — Mechanically Galvanized

Size	Model	Drill Bit Diameter	Wrench Size	Quantity		
(in.)	No.	(in.)	(in.)	Box	Carton	
3% x 5	THD37500HMG	3/8	9⁄16	50	100	
3∕% X 6	THD37600HMG	9/8	9⁄16	50	100	
½ x 5	THD50500HMG			20	80	
½ x 6	THD50600HMG	1/	3/4	20	80	
½ X 6½	THD50612HMG	1/2	9/4	20	40	
1⁄2 X 8	THD50800HMG			20	40	
¾ X 8½	THD75812HMG	3/4	11/	5	10	
¾ x 10	THD75100HMG	9/4	1 1/8	5	10	
3∕8 X 4	THD37300HMG	2/	0/	50	200	
³⁄8 x 4	THD37400HMG	3/8	9⁄16	50	200	
1⁄2 X 4	THD50400HMG	1/2	3⁄4	20	80	
³⁄4 x 6	THDT75600HMG	3⁄4	1 1/8	5	20	

Mechanical galvanizing meets ASTM B695, Class 65, Type 1. Intended for some pressure-treated wood sill plate applications. Not for use in other corrosive or outdoor environments. See pp. 18-21 or visit **strongtie.com/info** for more corrosion information.

Titen HD[®] Installation Information and Additional Data¹

Ok ana ak ani aki a	Quarter	11-11-				Nomina	I Anchor	Diameter	, d _a (in.)			
Characteristic	Symbol	Units	1/	4 ⁴	3,	/8	1,	/2	5/	^{′84}	3	/4
		Install	ation Info	ormation								
Drill Bit Diameter	dbit	in.	1,	/4	3	/8	1,	/2	5	/8	3,	/4
Baseplate Clearance Hole Diameter	dc	in.	3/8 1/2		5	/8	3,	/4	7,	/8		
Maximum Installation Torque	Tinst,max	ftlbf	24 ² 50 ²			6	5²	10)0 ²	15	50 ²	
Maximum Impact Wrench Torque Rating	Timpact,max	ftlbf	12	25 ³	15	0 ³	34	10 ³	34	10 ³	38	35 ³
Minimum Hole Depth	hhole	in.	13/4 25/8 23/4 31/2		3¾	41⁄2	41⁄2	6	6	6¾		
Nominal Embedment Depth	hnom	in.	1% 2½		21⁄2	31⁄4	31⁄4	4	4	5½	5½	6¼
Critical Edge Distance	Cac	in.	3	6	2 ¹¹ /16	3%	3%16	41⁄2	4 1/2	6%	6%	75⁄16
Minimum Edge Distance	in.	1	1/2				1	3⁄4				
Minimum Spacing	Smin	in.					3	3				
Minimum Concrete Thickness	hmin	in.	31⁄4	3½	4	5	5	61⁄4	6	81⁄2	8¾	10
		Ad	ditional I	Data								
Anchor Category	Category	_						1				
Yield Strength	fya	psi	100	,000				97,	000			
Tensile Strength	futa	psi	125	,000				110	,000			
Minimum Tensile & Shear Stress Area	Ase	in ²	0.0)42	0.0)99	0.1	83	0.2	276	0.4	414
Axial Stiffness in Service Load Range – Uncracked Concrete	β _{uncr}	lb./in.	202,000 715,000									
Axial Stiffness in Service Load Range – Cracked Concrete	β _{cr}	lb./in.	. 173,000 345,000									

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17.

2. Tinst,max is the maximum permitted installation torque for the embedment depth range covered by this table using a torque wrench.

3. *T_{impact,max}* is the maximum permitted torque rating for impact wrenches for the embedment depth range covered by this table.

4. Data for 14" anchor is only valid for THDB25 series. Data for the 5%" anchor is valid only for the THDB62 series.



Additional Installation Information — Hole Dimensions

Titen HD® Diameter (in.)	Wrench Size (in.)	Recommended Fixture Hole Size in Steel (in.)	Min. Hole Depth Overdrill in Concrete (in.)
1/4	3⁄8	¾ to 7⁄16	1⁄8
3/8	%16	½ to %16	1⁄4
1/2	3⁄4	5% to 11/16	1/2
5/8	15/16	3⁄4 to 13⁄16	1/2
3/4	11⁄8	7⁄8 t0 ¹⁵ ∕16	1/2

Note: Recommended fixture hole dimensions apply to structural steel greater than ¼" thick only and do not apply to CFS in 97 mil (12 ga.) and thinner. Standard ¼e" oversized holes may be used into CFS fixtures and tracks.

Installation Sequence

|--|--|--|--|

Titen HD — Tension Loads Attaching Cold-Formed Steel To Normal-Weight Concrete (lb.)⁶

											-		. ,	
Anchor	Drill	Edge	End	Min. Emb.	Concrete	Spacing		Conc	rete ^{3,4}		(Cold-Formed	l Steel (ASD)	5
Size (in.)	Bit (in.)	Distance (in.)	Distance (in.)	Depth (in.)	Thickness (in.)	(in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
		1 1⁄2	3	1 5⁄8	31⁄4	6	470	630	330	375				
1⁄4 x 1 7⁄8		1 3⁄4	3	1%	31⁄4	6	530	705	370	425				
	17.	3	3	1%	31⁄4	6	540	715	375	430	200	505	920	1 160
	74	1/4 1 1/2	3	21⁄2	31⁄2	6	725	965	510	580	390	505	920	1,160
1⁄4 x 23⁄4		1 3⁄4	3	21⁄2	31⁄2	6	790	1,050	555	630				
		3	3	21⁄2	31⁄2	6	930	1,240	650	745				
3∕8 x 3	3⁄8	1¾	3	21⁄2	4	6	600	800	420	480	585	760	1,380	1,740
		1 3⁄4	4	31⁄4	5	8	940	1,255	660	755				
1⁄2 x 4	1⁄2	3	4	31⁄4	5	8	1,320	1,760	925	1,055	585	760	1,380	1,740
	4	4	31⁄4	5	8	1,490	1,985	1,045	1,190					

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

 Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 2,500 psi. Reference ICC-ES ESR-2713 for further information.

Cold-Formed Steel (CFS) tension pullover values are based on AISI S-100, Eq. E4.4.2-1, d_w = 0.50" (¼" THD), d_w = 0.75" (¾" and ½" THD) and Ω = 3.0. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5. Tension values do not account for weak axis bending in the sill member.

6. Governing load is the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at strongtie.com.

8. Wind design includes SDC A&B.

9. The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.4.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in sand-lightweight concrete, concrete values shall be be multiplied by 0.68.

Titen HD[®] Heavy-Duty Screw Anchor

Titen HD — Shear Loads Perpendicular to Edge in Normal-Weight Concrete (lb.)⁶

Anchor	Drill	Edmo	End	Min.	Concrete			Conc	rete ^{3,4}		(Cold-Formed	l Steel (ASD)	5
Anchor Size (in.)	Bit (in.)	Edge Distance (in.)	Distance (in.)	Emb. Depth (in.)	Thickness (in.)	Spacing (in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
		1 1⁄2	3	1%	31⁄4	6	305	305	215	185				
1⁄4 x 1 7⁄8		1 3⁄4	3	1%	31⁄4	6	385	385	270	230				
	1⁄4	3	3	1 5⁄8	31⁄4	9	555	555	390	335	350	455	830	1,045
	74	1 1⁄2	3	21⁄2	31⁄2	6	340	340	235	205	350	400	030	1,045
1⁄4 x 23⁄4		1 3⁄4	3	21⁄2	31⁄2	6	425	425	300	255				
		3	3	21⁄2	31⁄2	9	635	635	445	380				
3∕8 X 3	3/8	1 3⁄4	7	21⁄2	4	5¼	475	475	335	285	510	685	1,240	1,565
78 X 3	78	3	7	21⁄2	4	9	1,000	1,000	700	600	510	005	1,240	1,000
16 v 4	14	1 3⁄4	8	31⁄4	5	5¼	545	545	380	325	595	880	1,655	2,085
72 X 4	2 x 4 ½	3	8	31⁄4	5	9	1,225	1,225	860	735	595	000	1,000	2,000

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities

by 0.7 for seismic and 0.6 for wind.

3. Shear load is applied perpendicular to the edge of concrete. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum $f_{\rm C}'$ of 2,500 psi. Reference ICC-ES ESR-2713 for further information.

5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

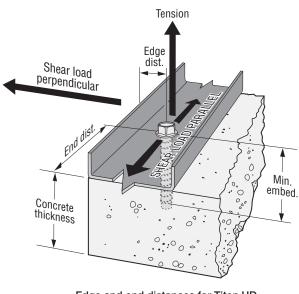
6. Governing load is the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at strongtie.com.

8. Wind design includes SDC A&B.

 The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in sand-lightweight concrete, concrete values shall be be multiplied by 0.68.



Edge and end distances for Titen HD in concrete slab corner condition

U.S. Patent 5,674,035

Titen HD[®] Heavy-Duty Screw Anchor



Titen HD — Shear Loads Parallel to Edge in Normal-Weight Concrete (lb./ft.)⁶

								Conc	rete ^{3,4}			Cold-Formed	Steel (ASD)	5
Anchor Size (in.)	Drill Bit (in.)	Edge Distance (in.)	End Distance (in.)	Min. Emb. Depth (in.)	Concrete Thickness (in.)	Spacing	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
						0' - 6"	790	790	550	470	700	910	1,660	2,090
						0' - 8"	595	595	415	355	525	685	1,245	1,570
						1'- 0"	395	395	275	235	350	455	830	1,045
1/ 17/		447		45/	01/	1'- 4"	295	295	205	175	265	340	625	785
1⁄4 x 1 7⁄8	1⁄4	1 1/2	3	1%	31⁄4	2'-0"	200	200	140	120	175	230	415	525
						2' - 8"	150	150	105	90	130	170	310	390
						4' - 0"	100	100	70	60	90	115	210	260
						6' - 0"	65	65	45	40	60	75	140	175
						0'-6"	840	840	590	500	700	910	1,660	2,090
						0' - 8"	630	630	445	375	525	685	1,245	1,570
						1'- 0"	420	420	295	250	350	455	830	1,045
¼x17%	1/.	13⁄4	3	1%	31⁄4	1'- 4"	315	315	220	190	265	340	625	785
'/4 X I '/8	1⁄4	1 %4	3	1 7/8	3 1/4	2'-0"	210	210	150	125	175	230	415	525
						2' - 8"	160	160	110	95	130	170	310	390
						4'-0"	105	105	75	65	90	115	210	260
						6' – 0"	70	70	50	40	60	75	140	175
						0' – 9"	740	740	520	445	465	605	1,105	1,395
						1'- 0"	555	555	390	335	350	455	830	1,045
	V4 x 1 7/8 1/4 3				1'- 4"	415	415	295	250	265	340	625	785	
1⁄4 x 1 7⁄8		3	1%	31⁄4	2'-0"	280	280	195	170	175	230	415	525	
						2' - 8"	210	210	145	125	130	170	310	390
						4'-0"	140	140	100	85	90	115	210	260
						6' – 0"	95	95	65	55	60	75	140	175
						0' - 6"	900	900	630	540	700	910	1,660	2,090
						0' - 8"	675	675	475	405	525	685	1,245	1,570
						1'- 0"	450	450	315	270	350	455	830	1,045
1/. y 03/.	1/.	114	2	014	214	1'- 4"	340	340	235	205	265	340	625	785
1⁄4 x 23⁄4	1⁄4	1½	3	21⁄2	3½	2' - 0"	225	225	160	135	175	230	415	525
						2' - 8"	170	170	120	100	130	170	310	390
						4' - 0"	115	115	80	70	90	115	210	260
						6' - 0"	75	75	55	45	60	75	140	175
						0'-6"	960	960	670	580	700	910	1,660	2,090
						0' - 8"	720	720	505	435	525	685	1,245	1,570
						1'- 0"	480	480	335	290	350	455	830	1,045
1⁄4 x 23⁄4	1⁄4	13⁄4	3	2½	31⁄2	1'- 4"	360	360	250	220	265	340	625	785
74 X Z 74	74	174	5	2 72	3 72	2'-0"	240	240	170	145	175	230	415	525
						2'-8"	180	180	125	110	130	170	310	390
						4' - 0"	120	120	85	75	90	115	210	260
						6' – 0"	80	80	55	50	60	75	140	175
						0' – 9"	845	845	595	505	465	605	1,105	1,395
						1'- 0"	635	635	445	380	350	455	830	1,045
						1'- 4"	475	475	335	285	265	340	625	785
1⁄4 x 23⁄4	1⁄4	3	3	21⁄2	31⁄2	2' - 0"	320	320	225	190	175	230	415	525
						2' - 8"	240	240	165	145	130	170	310	390
						4'-0"	160	160	110	95	90	115	210	260
					6' – 0"	105	105	75	65	60	75	140	175	

See footnotes on p. 195.

Titen HD — Shear Loads Parallel to Edge in Normal-Weight Concrete (lb./ft.)⁶ (cont.)

								Conci	rete ^{3,4}			Cold-Formed	Steel (ASD)	5
Anchor Size (in.)	Drill Bit (in.)	Edge Distance (in.)	End Distance (in.)	Min. Emb. Depth (in.)	Concrete Thickness (in.)	Spacing	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
						0' - 8"	1,425	1,425	995	855	765	1,030	1,860	2,350
						1'- 0"	950	950	665	570	510	685	1,240	1,565
						1'- 4"	715	715	500	430	385	515	930	1,175
%ахЗ	3⁄8	1¾	7	21⁄2	4	2'-0"	475	475	335	285	255	340	620	780
						2'-8"	355	355	250	215	190	255	465	585
						4' - 0"	240	240	170	145	130	170	310	390
						6' - 0"	160	160	110	95	85	115	205	260
						0' - 8"	1,645	1,645	1,150	985	895	1,320	2,485	3,130
						1'- 0"	1,095	1,095	765	655	595	880	1,655	2,085
				8 31/4		1'- 4"	820	820	575	490	445	660	1,240	1,565
½ x 4	1/2 x 4 1/2 13/4 8	8	31⁄4	5	2'-0"	550	550	385	330	300	440	830	1,045	
					2'-8"	410	410	285	245	220	330	620	780	
						4' - 0"	275	275	195	165	150	220	415	520
						6' – 0"	185	185	130	110	100	145	275	350
						0' - 8"	1,770	1,770	1,240	1,060	765	1,030	1,860	2,350
						1'- 0"	1,180	1,180	825	710	510	685	1,240	1,565
						1'- 4"	885	885	620	530	385	515	930	1,175
%кхЗ	3⁄8	3	7	21⁄2	4	2'-0"	590	590	415	355	255	340	620	780
						2'-8"	445	445	310	265	190	255	465	585
						4' - 0"	295	295	205	175	130	170	310	390
						6' - 0"	195	195	135	115	85	115	205	260
						0' - 8"	2,505	2,505	1,755	1,505	895	1,320	2,485	3,130
						1'- 0"	1,670	1,670	1,170	1,000	595	880	1,655	2,085
	½ x 4 ½ 3				1'- 4"	1,255	1,255	880	755	445	660	1,240	1,565	
½ x 4		8	31⁄4	5	2'-0"	835	835	585	500	300	440	830	1,045	
					2' – 8"	625	625	440	375	220	330	620	780	
					4' – 0"	420	420	295	250	150	220	415	520	
						6' – 0"	280	280	195	170	100	145	275	350

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities

by 0.7 for seismic and 0.6 for wind.

3. Shear load is applied parallel to the edge of concrete. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 2,500 psi. Reference ICC-ES ESR-2713 for further information.

 Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, m_f = 0.75, Ω = 2.5. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load is the lesser of concrete and CFS.

7. For conditons not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at strongtie.com.

8. Wind design includes SDC A&B.

 The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in sand-lightweight concrete, concrete values shall be be multiplied by 0.68.



Titen HD — Tension Loads Attaching Cold-Formed Steel to Top of Normal-Weight Concrete over Metal Deck (lb.)^{6,11}

Anchor	Drill	Min.	Min.	Emb.	Min.			Conc	rete ^{3,4}		(Cold-Formed	l Steel (ASD)	5
Size (in.)	Size Bit Di	Edge Distance (in.)	End Distance (in.)	Depth (in.)	Concrete Thickness (in.)	Spacing (in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
1⁄4 X 1 7⁄8	1⁄4	31⁄2	3¾	1 5⁄8	21⁄2	3½	580	775	405	545	390	505	920	1,160
3∕8 x 3	3⁄8	3	71⁄4	21⁄2	31⁄4	3	660	880	460	615	585	760	1,380	1,740

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities

by 0.7 for seismic and 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 3,000 psi. Reference ICC-ES ESR-2713 for further information.

- Cold-formed steel (CFS) tension pullover values are based on AISI S-100, Eq. E4.4.2-1, d_w = 0.50" (¼" THD), d_w = 0.75" (¾" THD) and Ω = 3.0. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5. Tension values do not account for weak axis bending in the sill member.
 Governing load shall be the lesser of concrete and CFS.
- For conditions not covered by this table, use the Simpson Strong-Tie[®] Anchor Designer[™] software available at strongtie.com.

8. Wind design values include SDC A&B.

 The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.4.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in top of sand-lightweight concrete over metal deck, concrete values shall be be multiplied by 0.68.

11. Metal deck configuration to comply with Figure 5 of ICC-ES ESR-2713.

Titen HD — Shear Loads Perpendicular to Edge in Top of Normal-Weight Concrete over Metal Deck (lb.)^{6,11}

Anchor	Drill	Min.	Min.	Emb.	Min.			Conci	rete ^{3,4}		(Cold-Formed	l Steel (ASD)	5
Size (in.)	ize Bit Distance	End Distance (in.)	Donth	Concrete Thickness (in.)	Spacing (in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)	
1⁄4 x 1 7⁄8	1⁄4	31⁄2	3¾	1%	21⁄2	31⁄2	450	450	315	315	350	455	830	1,045
3∕% x 3	3⁄/8	3	71⁄4	21⁄2	31⁄4	3	660	660	460	460	510	685	1,240	1,565

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

Anchor is considered as an individual anchor without influence from other anchors.

Concrete shall have a minimum f¹_c of 3,000 psi. Reference ICC-ES ESR-2713 for further information.

5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor DesignerTM software available at **strongtie.com**.

8. Wind design values include SDC A&B.

9. The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in top of sand-lightweight concrete over metal deck, concrete values shall be be multiplied by 0.68.

11. Metal deck configuration to comply with Figure 5 of ICC-ES ESR-2713.



Anchor	Drill	Min.	Min.	Emb.	Min.			Conci	rete ^{3,4}		(Cold-Formed	l Steel (ASD)	5
Size (in.)	Bit (in.)	Edge Distance (in.)	End Distance (in.)	Depth (in.)	Concrete Thickness (in.)		LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
1/. v 17/	1/4 x 17/8 1/4 31/2 33/4 1	1%	21/2	1'-0"	635	635	445	445	350	455	830	1,045		
74 X 1 78		374	178	2.72	1'-4"	475	475	335	335	265	340	625	785	
						0'-9"	1,590	1,590	1,115	1,115	680	915	1,655	2,085
3∕8 x 3	¾ x 3 ⅔ 3	3	71⁄4	21⁄2	31⁄4	1'-0"	1,195	1,195	835	835	510	685	1,240	1,565
						1'-4"	895	895	625	625	385	515	930	1,175

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

 Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 3,000 psi. Reference ICC-ES ESR-2713 for further information.

5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at **strongtie.com**.

8. Wind design values include SDC A&B.

9. The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in top of sand-lightweight concrete over metal deck, concrete values shall be be multiplied by 0.68.

11. Metal deck configuration to comply with Figure 5 of ICC-ES ESR-2713.

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A wedge-type expansion anchor designed for optimal performance in cracked and uncracked concrete as well as uncracked masonry. The Strong-Bolt 2 is available in carbon steel (¼" through 1" diameter), Type 304 (¼" diameter only) and Type 316 stainless steel (¼" through ¾" diameter).

Features:

- Code listed under IBC/IRC for cracked and uncracked concrete per ICC-ES ESR-3037
- Code listed under IBC/IRC for masonry per IAPMO UES ER-240
- Qualified for static and seismic loading conditions (seismic design categories A through F)
- Suitable for horizontal, vertical and overhead applications
- Qualified for minimum concrete thickness of 3¼", and lightweight concrete-over-metal deck thickness of 2½" and 3¼"
- Standard (ANSI) fractional sizes: fits standard fixtures and installs with common drill bit and tool sizes

Codes: ICC-ES ESR-3037 (concrete); IAPMO UES ER-240 (carbon steel in CMU); City of L.A. RR25891 (concrete), RR25936 (carbon steel in CMU); Florida FL-15731.2; FL-16230.4; UL File Ex3605; FM 3043342 and 3047639; Mulitiple DOT listings; meets the requirements of Federal Specifications A-A-1923A, Type 4

Material: Carbon-steel stud with special alloy clip; stainless-steel stud with stainless-steel clip

Coating: Zinc plated

c(VL)us FM

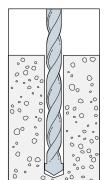
Anchors

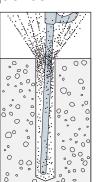
Installation: Do not use an impact wrench to set or tighten the Strong-Bolt 2 anchor.

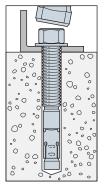
Caution: Oversized holes in the base material will make it difficult to set the anchor and will reduce the anchor's load capacity.

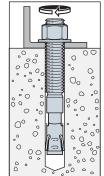
- Drill a hole in the base material using a carbide drill bit the same diameter as the nominal diameter of the anchor to be installed.
 Drill the hole to the specified minimum hole depth and blow it clean using compressed air. Overhead installations need not be blown clean. Alternatively, drill the hole deep enough to accommodate embedment depth and dust from drilling.
- Assemble the anchor with nut and washer so that the top of the nut is flush with the top of the anchor. Place the anchor in the fixture and drive into the hole until washer and nut are tight against the fixture.
- Tighten to the required installation torque.

Installation Sequence













SIMPSON

Strong



Head Stamp The head is stamped with the length identification letter, bracketed top and bottom by horizontal lines.

Strong-Bolt 2 Wedge Anchor



Strong-Bolt® 2 Anchor Product Data

Size	Type 304 Stainless Steel	Type 316 Stainless Steel	Drill Bit	Thread	Qua	ntity
(in.)	Model No.	Model No.	Diameter (in.)	Length (in.)	Box	Carton
1⁄4 x 13⁄4	STB2-251344SS	STB2-251346SS	1⁄4	1 5/16	100	500
1⁄4 x 21⁄4	STB2-252144SS	STB2-252146SS	1⁄4	1 7⁄16	100	500
1⁄4 x 31⁄4	STB2-253144SS	STB2-253146SS	1⁄4	27⁄16	100	500
3∕8 x 2¾	STB2-372344SS	STB2-372346SS	3⁄8	1 %16	50	250
3∕8 X 3	STB2-373004SS	STB2-373006SS	3/8	1%6	50	250
3∕8 x 3½	STB2-373124SS	STB2-373126SS	3⁄8	21⁄16	50	250
3∕8 X 3¾	STB2-373344SS	STB2-373346SS	3⁄8	25/16	50	250
¾ x 5	STB2-375004SS	STB2-375006SS	3⁄8	3%6	50	200
3∕8 X 7	STB2-377004SS	STB2-377006SS	3⁄8	5%6	50	200
½ x 3¾	STB2-503344SS	STB2-503346SS	1/2	21⁄16	25	125
½ x 4¼	STB2-504144SS	STB2-504146SS	1/2	2%	25	100
½ x 4¾	STB2-504344SS	STB2-504346SS	1/2	31⁄16	25	100
½ x 5½	STB2-505124SS	STB2-505126SS	1/2	313/16	25	100
½ x 7	STB2-507004SS	STB2-507006SS	1/2	5%	25	100
½ x 8½	STB2-508124SS	STB2-508126SS	1/2	6	25	50
½ x 10	STB2-501004SS	STB2-501006SS	1/2	6	25	50
5∕8 x 4½	STB2-624124SS	STB2-624126SS	5%8	27⁄16	20	80
5% x 5	STB2-625004SS	STB2-625006SS	5/8	215/16	20	80
5% x 6	STB2-626004SS	STB2-626006SS	5/8	315/16	20	80
5∕8 x 7	STB2-627004SS	STB2-627006SS	5/8	415/16	20	80
5∕8 x 8½	STB2-628124SS	STB2-628126SS	5/8	6	20	40
5∕8 x 10	STB2-621004SS	STB2-621006SS	5⁄8	6	10	20
¾ x 5½	STB2-755124SS	STB2-755126SS	3⁄4	3¾6	10	40
¾ x 6¼	STB2-756144SS	STB2-756146SS	3⁄4	315/16	10	40
3⁄4 x 7	STB2-757004SS	STB2-757006SS	3⁄4	411/16	10	40
34 x 8½	STB2-758124SS	STB2-758126SS	3⁄4	6	10	20



Carbon Steel Strong-Bolt® 2 Installation Information¹

Characteristic	Symbol	Units				No	minal An	chor Diam	ieter, d _a (in.)				
	0,	enne	1/44	3/	⁶⁵		1/25		5	85	3	⁄4 ⁵	-	15
				Instal	lation Inf	ormation								
Nominal diameter	da	in.	1⁄4	3	/8		1⁄2		5,	/8	3	/4		1
Drill bit diameter	d	in.	1⁄4	3	/8		1⁄2		5	/8	3	/4		1
Baseplate clearance hole diameter ²	d _c	in.	5⁄16	5/16 7/16			9⁄16		1]	/16	7	/8	1	1⁄8
Installation torque	Tinst	ft-lbf	4	3	0		60		g	0	1	50	2	30
Nominal embedment depth	h _{nom}	in.	1 3⁄4	17⁄8 27⁄8		2	3⁄4	37⁄8	3¾	51⁄8	41⁄8	5¾	51⁄4	9¾
Effective embedment depth	hef	in.	1 1/2	1 1/2	21⁄2	2	1⁄4	3%	2¾	4 1/2	3%	5	41⁄2	9
Minimum hole depth	h _{hole}	in.	1 7/8	2	3		3	41⁄8	3%	5%	4%	6	5½	10
Minimum overall anchor length	lanch	in.	21⁄4	2¾	31⁄2	3	3⁄4	5½	41⁄2	6	5½	7	7	13
Critical edge distance	Cac	in.	21⁄2	6½	6	6½	6½	71⁄2	71⁄2	9	9	8	18	13½
Minimum adra diatanaa	C _{min}	in.	1 3⁄4	(5	7	4	4	6	1/2	6	1/2		8
Minimum edge distance	for $s \ge$	in.	_	_		_	_	_	_			В	-	_
Minimum spacing	S _{min}	in.	21⁄4		3	7	4	4	:	5		7		8
winimum spacing	for $c \ge$	in.	_	-	_	_	_	_	_			?	-	_
Minimum concrete thickness	h _{min}	in	31⁄4	31⁄4	41⁄2	41⁄2	5½	6	5½	71⁄8	6¾	8¾	9	13½
				A	dditional	Data								
Yield strength	fya	psi	56,000	92,	000			85,000			70,	000	60,	000
Tensile strength	f _{uta}	psi	70,000				115,000				110	,000	78,	000
Minimum tensile and shear stress area	A _{se}	in.²	0.0318	0.0	514		0.105		0.1	66	0.2	270	0.4	472
Axial stiffness in service load range — Cracked and uncracked concrete	β	lb./in.	73,700 ³	34,820			63,570		91,	370	118	,840	299	,600

1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17.

2. The clearance must comply with applicable code requirements for the connected element.

3. The tabulated value of β for 1/4"-diameter carbon steel Strong-Bolt 2 anchor is for installations in uncracked concrete only.

4. The 1/4"-diameter (6.4mm) anchor may be installed in top of uncracked normal-weight and sand-lightweight concrete over profile

steel deck, where concrete thickness above upper flute meets the minimum thickness specified in this table.

5. The %" through 1"-diameter (9.5mm through 25.4mm) anchors may be installed in top of cracked and uncracked normal-weight and sand-lightweight concrete over profile steel deck, where concrete thickness above upper flute meets the minimum thickness specified in this table.

Strong-Bolt 2 Carbon Steel — Tension Loads Attaching Cold-Formed Steel to Normal-Weight Concrete (lb.)⁶

Anchor	Drill	Edge	End	Min. Emb.	Concrete	Spacing		Conci	rete ^{3,4}			Cold-Formed	Steel (ASD)	5
Dia. (in.)	Bit (in.)	Distance (in.)	Distance (in.)	Depth (in.)	Thickness (in.)	(in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
3/8	3/8	6	e	1 7⁄8	31⁄4	41⁄2	635	845	445	505	615	905	1.460	1 0 0 5
78	9/8	0	6	21⁄8	41⁄2	71⁄2	1,355	1,805	950	1,085	010	805	1,460	1,835
		7	7	2¾	41⁄2	7	1,400	1,865	980	1,120				
1/2	1/2	4	4	2¾	5½	6¾	1,400	1,865	980	1,120	825	1,080	1,955	2,460
		4	4	31⁄8	6	4	1,390	1,850	975	1,110				

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

 Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum $\rm f^{\rm t}_{C}$ of 2,500 psi. Reference ICC-ES ESR-3037 for further information.

5. Cold-formed steel (CFS) tension pullover values are based on AISI S-100, Eq. E4.4.2-1, d_w = 0.793" (%" STB2), d_w = 1.062" (½" STB2) and Ω = 3.0. Reference General Notes for CFS properties. To convert from ASD to

LRFD multiply value by 1.5. Tension values do not account for weak axis bending in the sill member.

6. Governing load shall be the lesser of concrete and CFS.

- For conditions not covered by this table, use the Simpson Strong-Tie[®] Anchor Designer[™] software available at strongtie.com.
- 8. Wind design values include SDC A&B.
- The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.4.3 for designing anchorage in Seismic Design Category C–F.

 For installation in sand-lightweight concrete, concrete values shall be multiplied by 0.68.

Strong-Bolt[®] 2 Wedge Anchor

Strong-Bolt 2 Carbon Steel -Shear Loads Perpendicular to Edge in Normal-Weight Concrete (lb.)⁶

Anchor	Drill	Edge	End	Min. Emb.	Concrete	Spacing		Conci	rete ^{3,4}		(Cold-Formed	Steel (ASD)	j
Dia. (in.)	Bit (in.)	Distance (in.)	Distance (in.)	Depth (in.)	Thickness (in.)	(in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
3/8	3/8	6	6	1 7⁄8	31⁄4	41⁄2	850	850	595	510	510	685	1.240	1,565
78	78	0	0	21⁄8	41⁄2	71⁄2	1,170	1,170	820	700	510	000	1,240	1,000
		7	7	23⁄4	41⁄2	7	1,490	1,490	1,045	895				
1/2	1⁄2	4	4	23⁄4	51⁄2	6¾	1,125	1,125	790	675	595	880	1,655	2,085
		4	4	31⁄8	6	4	1,065	1,065	745	640				

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 2,500 psi. Reference ICC-ES ESR-3037 for further information.

5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, mf = 0.75, Ω = 2.5. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

- 6. Governing load shall be the lesser of concrete and CFS.
- 7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer[™] software available at strongtie.com.
- 8. Wind design values include SDC A&B.

9. The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C-F.

10. For installation in sand-lightweight concrete, concrete values shall be multiplied by 0.68.

Strong-Bolt 2 Carbon Steel — Shear Loads Parallel to Edge in Normal-Weight Concrete (lb /ft)⁶

Anchor	Drill	Edge	End	Min. Emb.	Concrete	Spacing		Conci	rete ^{3,4}			Cold-Formed	I Steel (ASD) ⁵	5
Dia. (in.)	Bit (in.)	Distance (in.)	Distance (in.)	Depth (in.)	Thickness (in.)	(in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
						1'-0"	1,090	1,090	765	655	510	685	1,240	1,565
						1'- 4"	820	820	575	490	385	515	930	1,175
3⁄8	3/8	6	6	17/8	31⁄4	2'-0"	545	545	380	325	255	340	620	780
78	78	0	0	178	3 74	2'-8"	410	410	285	245	190	255	465	585
						4' - 0"	275	275	195	165	130	170	310	390
						6' – 0"	180	180	125	110	85	115	205	260
						0'-8"	1,755	1,755	1,230	1,055	765	1,030	1,860	2,350
						1'-0"	1,170	1,170	820	700	510	685	1,240	1,565
						1'- 4"	880	880	615	530	385	515	930	1,175
3⁄8	3⁄8	6	6	2-7⁄8	41⁄2	2'-0"	585	585	410	350	255	340	620	780
					2'-8"	440	440	310	265	190	255	465	585	
						4'-0"	295	295	205	175	130	170	310	390
						6'-0"	195	195	135	115	85	115	205	260
						1'-0"	1,345	1,345	940	805	595	880	1,655	2,085
						1'- 4"	1,010	1,010	705	605	445	660	1,240	1,565
1/2	1/2	4	4	2¾	5½	2'-0"	675	675	470	405	300	440	830	1,045
72	72	4	4	274	3 72	2'-8"	505	505	355	305	220	330	620	780
						4'-0"	335	335	235	200	150	220	415	520
						6'-0"	225	225	160	135	100	145	275	350
						1'-0"	1,520	1,520	1,065	910	595	880	1,655	2,085
						1'- 4"	1,140	1,140	800	685	445	660	1,240	1,565
1/2	1/2	4	Λ	4 376	6	2'-0"	760	760	530	455	300	440	830	1,045
72	72	2 4 4 37/8	578	0	2'-8"	570	570	400	340	220	330	620	780	
				4'-0"	380	380	265	230	150	220	415	520		
						6' - 0"	255	255	180	155	100	145	275	350

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

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Allowable Stress Design (ASD) values are obtained by multiplying Load 2. Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

- 3. Anchor is considered as an individual anchor without influence from other anchors.
- Concrete shall have a minimum f'c of 2,500 psi. Reference ICC-ES 4. ESR-3037 for further information.
- 5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS

properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer[™] software available at **strongtie.com**.

8. Wind design values include SDC A&B.

The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C-F.

10. For installation in sand-lightweight concrete, concrete values shall be multiplied by 0.68.





Carbon Steel Strong-Bolt 2 — Tension Loads Attaching Cold-Formed Steel to Top of Normal-Weight Concrete over Metal Deck (lb.)^{6,11}

								Conci	rete ^{3,4}			Cold-Formed	l Steel (ASD)	5
Anchor Size (in.)	Drill Bit (in.)	Min. Edge Distance (in.)	Min. End Distance (in.)	Emb. Depth (in.)	Min. Concrete Thickness (in.)	Spacing (in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
3⁄8 x 23⁄4	3⁄8	4¾	4¾	1 7⁄8	21⁄2	7	695	925	485	555	615	805	1.400	1.005
3⁄8 X 23⁄4	3⁄8	41⁄2	41⁄2	1 1 1/8	31⁄4	6½	690	925	480	222	610	805	1,460	1,835
1⁄2 X 3¾	1⁄2	43⁄4	4¾	2¾	31⁄4	8	1,530	2,040	1,070	1,225	825	1,080	1,955	2,460

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

 Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

Anchor is considered as an individual anchor without influence from other anchors.

Concrete shall have a minimum f¹_c of 3,000 psi. Reference ICC-ES ESR-3037 for further information.

 Cold-formed steel (CFS) tension pullover values are based on AISI S-100, Eq. E4.4.2-1, d_w = 0.793" (% STB2), d_w = 1.062" (½" STB2) and Ω = 3.0. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5. Tension values do not account for weak axis bending in the sill member.

6. Governing load shall be the lesser of concrete and CFS.

For conditions not covered by this table, use the Simpson Strong-Tie[®] Anchor Designer[™] software available at **strongtie.com**.
 Wind design values include SDC A&B.

 While design values include obor AdD.
 The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.4.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in top of sand-lightweight concrete over metal deck, concrete values shall be be multiplied by 0.68.

11. Metal deck configuration to comply with Figure 5 of ICC-ES ESR-3037.

Carbon Steel Strong-Bolt 2 — Shear Loads Perpendicular to Edge in Top of Normal-Weight Concrete over Metal Deck (lb.)^{6,11}

Anchor	Drill	Min.	Min.	Emb.	Min.			Conci	rete ^{3,4}			Cold-Formed	Steel (ASD)	5
Size (in.)	Bit (in.)	Edge Distance (in.)	End Distance (in.)	Depth (in.)	Concrete Thickness (in.)	Spacing (in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
3⁄8 x 23⁄4	3⁄8	4¾	4¾	1 7⁄8	21⁄2	7	790	790	555	475	510	685	1.240	1 505
3⁄8 X 23⁄4	3⁄8	41⁄2	41⁄2	1 7⁄8	31⁄4	6½	850	850	595	510	510	080	1,240	1,565
½ x 3¾	1⁄2	4¾	4¾	2¾	31⁄4	8	1,125	1,125	790	675	595	880	1,655	2,085

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities

by 0.7 for seismic and 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 3,000 psi. Reference ICC-ES ESR-3037 for further information.

5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie[®] Anchor Designer[™] software available at **strongtie.com**.

8. Wind design values include SDC A&B.

 The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in top of sand-lightweight concrete over metal deck, concrete values shall be be multiplied by 0.68.

11. Metal deck configuration to comply with Figure 5 of ICC-ES ESR-3037.

Carbon Steel Strong-Bolt 2 — Shear Loads Parallel to Edge in Top of Normal-Weight Concrete over Metal Deck (lb./ft.)^{6,11}

Anchor	Drill	Min.	Min.	Emb.	Min.			Conc	rete ^{3,4}			Cold-Formed	Steel (ASD)	5
Size (in.)	Bit (in.)	Edge Distance (in.)	End Distance (in.)	Depth (in.)	Concrete Thickness (in.)	Spacing (in.)	LRFD ^{1,9,10} (Seismic)	LRFD ^{1,8,10} (Wind)	ASD ^{2,9,10} (Seismic)	ASD ^{2,8,10} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	68 mil (14 ga.)
3% x 23⁄4	3/8	43⁄4	43⁄4	1%	21/2	1'-0"	995	995	695	595	510	685	1,240	1,565
98 X 294	9/8	4%	4%	1 1/8	2 1/2	1'-4"	745	745	520	445	385	515	930	1,175
3/8 X 23/4	3/8	41/2	41/2	17/8	21/	1'-0"	1,075	1,075	755	645	510	685	1,240	1,565
98 X ∠94	78	4 72	4 72	1 1/8	31⁄4	1'-4"	805	805	565	485	385	515	930	1,175
1⁄2 x 33⁄4	1/2	43⁄4	43⁄4	23⁄4	21/	1'-0"	1,345	1,345	940	805	595	880	1,655	2,085
72 X 394	/2	4%	4%	2%4	3/4 31/4	1'-4"	1,005	1,005	705	605	445	660	1,240	1,565

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume cracked concrete with no supplementary reinforcement.

 Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.7 for seismic and 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f_c of 3,000 psi. Reference ICC-ES ESR-3037 for further information.

5. Cold-formed steel (CFS) shear values are based on AISI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at strongtie.com.

8. Wind design values include SDC A&B.

 The listed load values are governed by a brittle failure mode, the Designer shall consider ductility requirements of ACI 318-14 Section 17.2.3.5.3 for designing anchorage in Seismic Design Category C–F.

10. For installation in top of sand-lightweight concrete over metal deck, concrete values shall be be multiplied by 0.68.

11. Metal deck configuration to comply with Figure 5 of ICC-ES ESR-3037.

Titen[®] 2 Concrete and Masonry Screws

With patented undercutting threads that make installation easier and increase load capacity, the Titen® 2 concrete and masonry screw is ideal for attaching all types of components to concrete and masonry. The improved thread design undercuts the base material more efficiently, reducing installation torgue and making it easier to drive without binding, snapping or stripping, even during installation into hard base material.

Features:

- · Patented undercutting threads reduce installation torque
- · Innovative design increases load capacity
- Code listed in accordance with ICC-ES AC193 for concrete application (IAPMO UES ER-449) and ICC-ES AC106 for masonry application (IAPMO UES ER-466)
- Suitable for near-edge concrete installations without expansion forces and cracking
- · Installs with Standard ANSI drill bits (bit included in larger count boxes)
- · Zinc plated with a baked-on ceramic coating
- · Preservative treated wood applications: suited for use in non-ammonia formulations of CCA, ACQ-C, ACQ-D, CA-B, BX/DOT and zinc borate
- Use in dry interior environments only

Patent pending serrated cutting teeth

reduce torque for

reliable installations



U.S. Patent Pending



head for full drive engagement

the base material increasing loads compared to other screw anchors of this type

> Phillips flat head for flush or countersunk applications

Patented threads undercut

Tightly controlled material hardness gives Titen[®] 2 consistent performance screw after screw

Full thread engagement throughout entire

embedment

Slotted high-hex

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC

Titen® 2 Concrete and Masonry Screws

Blue Titen® 2 Product Data (3/16" diameter)

Size	Head Style	Model No.	Drill Bit	Qua	ntity
5120	neau Style	wouer no.	Diameter	Box	Carton
3⁄16" x 1 1⁄4"		TTN2-18114H		100	1,600
3⁄16" X 1 3⁄4"		TTN2-18134H		100	500
3⁄16" X 21⁄4"		TTN2-18214H		100	500
3⁄16" X 23⁄4"	1⁄4" Hex	TTN2-18234H	5⁄32"	100	500
3⁄16" x 31⁄4"	TIOX .	TTN2-18314H		100	400
3⁄16" X 33⁄4"		TTN2-18334H		100	400
3⁄16" х 4"		TTN2-18400H		100	1,600
3⁄16" x 1 1⁄4"		TTN2-18114PF		100	500
3⁄16" X 1 3⁄4"		TTN2-18134PF		100	500
3⁄16" X 21⁄4"		TTN2-18214PF		100	500
3⁄16" X 23⁄4"	#2 Phillins Flat	TTN2-18234PF	5/32"	100	400
3⁄16" x 31⁄4"	Phillips Flat	TTN2-18314PF		100	400
3⁄16" X 33⁄4"		TTN2-18334PF		100	400
3∕16" x 4"		TTN2-18400PF		100	400

Blue Titen® 2 Product Data (1/4" diameter)

Size	Head Style	Model No.	Drill Bit	Qua	ntity
5120	neau Style	wouer no.	Diameter	Box	Carton
1⁄4" x 1 1⁄4"		TTN2-25114H		100	1,600
1⁄4" x 1 3⁄4"		TTN2-25134H		100	500
1⁄4" x 21⁄4"		TTN2-25214H		100	500
1⁄4" x 2³⁄4"		TTN2-25234H		100	500
1⁄4" x 31⁄4"	5∕16" Hex	TTN2-25314H	3⁄16"	100	400
1⁄4" x 3¾"	TION	TTN2-25334H		100	400
1⁄4" x 4"		TTN2-25400H		100	1,600
1⁄4" x 5"		TTN2-25500H		100	500
1⁄4" x 6"	-	TTN2-25600H		100	500
1⁄4" x 1 1⁄4"		TTN2-25114PF		100	500
1⁄4" x 1 3⁄4"		TTN2-25134PF		100	400
1⁄4" x 21⁄4"	-	TTN2-25214PF		100	400
1⁄4" x 2¾"	#3	TTN2-25234PF		100	400
1⁄4" x 31⁄4"	Phillips	TTN2-25314PF	3⁄16"	100	400
1⁄4" x 3¾"	Phillips Flat	TTN2-25334PF		100	400
1⁄4" x 4"		TTN2-25400PF		100	400
1⁄4" x 5"		TTN2-25500PF		100	400
1⁄4" x 6"		TTN2-25600PF		100	400

White Titen® 2 Product Data (Phillips Flat-Head)

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Size	Head Style	Model No.	Drill Bit	Qua	ntity
5120	neau Style	wouer no.	Diameter	Box	Carton
3⁄16" x 1 1⁄4"		TTN2W18114PF		100	1,600
3⁄16" x 1 3⁄4"	#2 Phillips Flat	TTN2W18134PF		100	500
³ ⁄16" X 2 ¹ ⁄4"	=	TTN2W18214PF	5/32"	100	500
3⁄16" X 23⁄4"		TTN2W18234PF	732	100	500
³ ⁄16" X 3 ¹ ⁄4"		TTN2W18314PF		100	400
3⁄16" X 33⁄4"		TTN2W18334PF		100	400
1⁄4" x 1 1⁄4"		TTN2W25114PF		100	1,600
1⁄4" x 1 3⁄4"	#2	TTN2W25134PF		100	500
1⁄4" x 21⁄4"	#3 Dhilling	TTN2W25214PF	3/16"	100	500
1⁄4" x 23⁄4"	#3 Phillips Flat	TTN2W25234PF	9/16	100	500
1⁄4" x 31⁄4"		TTN2W25314PF		100	400
1⁄4" x 3¾"		TTN2W25334PF		100	400

Titen® 2 Concrete and Masonry Screws

Titen 2 — Tension Loads Attaching Cold-Formed Steel to Normal-Weight Concrete (lb.)⁶

Anchor	Drill	Edge	End	Min.	Concrete	Min.	Conc	rete ^{3,4}	Cold-Formed	Steel (ASD)⁵
Diameter (in.)	Bit (in.)	Distance (in.)	Distance (in.)	Emb. Depth (in.)	Thickness (in.)	Spacing (in.)	LRFD ^{1,8,9} (Wind)	ASD ^{2,8,9} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)
3⁄16	5⁄32	1¾	3	1 3⁄4	31⁄4	4	690	415	240	315
1⁄4	3⁄16	1 3⁄4	3	1 3⁄4	31⁄4	4	690	415	290	380

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume uncracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'_{c} of 2,500 psi. Reference IAPMO UES ER-449 for further information.

5. Cold-formed steel (CFS) tension pullover values are based on AISI S-100, Eq. E4.4.2-1, $d_W = \frac{5}{16^{\circ}}$ ($\frac{5}{16^{\circ}}$ Titen 2),

 $d_W = \%$ " (χ " Titen 2) and $\Omega = 3.0$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply

value by 1.5. Tension values do not account for weak axis bending in the sill member. 6 Governing load shall be the lesser of concrete and CES

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at **strongtie.com**.

8. Wind design values include SDC A&B.

9. For installation in sand-lightweight concrete, concrete values shall be be multiplied by 0.68.

Titen 2 — Shear Loads Perpendicular to Edge in Normal-Weight Concrete (lb.)⁶

Anchor	Drill	Edge	End	Min.	Concrete	Min.	Conci	rete ^{3,4}	Cold-Formed	Steel (ASD)⁵
Diameter (in.)	Bit (in.)	Distance (in.)	Distance (in.)	Emb. Depth (in.)	Thickness (in.)	Spacing (in.)	LRFD ^{1,8,9} (Wind)	ASD ^{2,8,9} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)
3⁄16	5⁄32	1 3⁄4	3	1 3⁄4	31⁄4	51⁄4	465	280	265	340
1⁄4	3⁄16	1¾	3	1 3⁄4	31⁄4	51⁄4	500	300	350	455

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume uncracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f'c of 2,500 psi. Reference IAPMO UES ER-449 for further information.

5. Cold-formed steel (CFS) shear values are based on AlSI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie[®] Anchor Designer[™] software available at **strongtie.com**.

8. Wind design values include SDC A&B.

9. For installation in sand-lightweight concrete, concrete values shall be be multiplied by 0.68.

Titen® 2 Concrete and Masonry Screws

Anchor	Deill	Edmo	End	Min.	Conorata	Min	Conc	rete ^{3,4}	Cold-Formed	l Steel (ASD)⁵
Anchor Diameter (in.)	Drill Bit (in.)	Edge Distance (in.)	End Distance (in.)	Emb. Depth (in.)	Concrete Thickness (in.)	Min. Spacing (in.)	LRFD ^{1,8,9} (Wind)	ASD ^{2,8,9} (Wind)	33 mil (20 ga.)	43 mil (18 ga.)
						0' - 8"	750	450	400	510
						1'- 0"	500	300	265	340
					1'- 4"	375	225	200	255	
3⁄16	5⁄32	1 3⁄4	3	1 3⁄4	31⁄2	2'-0"	250	150	135	170
						2'-8"	190	115	100	130
						4' - 0"	125	75	65	85
						6' – 0"	85	50	45	55
						0' - 8"	810	485	525	685
						1'0"	540	325	350	455
						1'- 4"	405	245	265	340
1⁄4	3⁄16	1 3⁄4	3	1 3⁄4	31⁄2	2'-0"	270	160	175	230
						2'-8"	205	125	130	170
						4' – 0"	135	80	90	115
						6'-0"	90	55	60	75
						0' - 8"	885	530	400	510
						1'0"	590	355	265	340
						1'- 4"	445	265	200	255
3⁄16	5⁄32	1 3⁄4	5½	13⁄4	31⁄2	2'-0"	295	175	135	170
						2'-8"	220	130	100	130
						4' – 0"	150	90	65	85
						6' – 0"	100	60	45	55
						0' - 8"	1,120	670	525	685
						1'0"	745	445	350	455
						1'- 4"	560	335	265	340
1⁄4	3⁄16	1 3⁄4	5½	1 3⁄4	31⁄2	2'-0"	375	225	175	230
						2' - 8"	280	170	130	170
						4' – 0"	185	110	90	115
						6' – 0"	125	75	60	75

1. Anchorage designs conform to ACI 318-14 Chapter 17 and assume uncracked concrete with no supplementary reinforcement.

2. Allowable Stress Design (ASD) values are obtained by multiplying Load Resistance Factor Design (LRFD) capacities by 0.6 for wind.

3. Anchor is considered as an individual anchor without influence from other anchors.

4. Concrete shall have a minimum f_{C}^{\prime} of 2,500 psi. Reference IAPMO UES ER-449 for further information.

5. Cold-formed steel (CFS) shear values are based on AlSI-S100, Eq. E3.3.1-1, $m_f = 0.75$, $\Omega = 2.5$. Reference General Notes

for CFS properties. To convert from ASD to LRFD multiply value by 1.5.

6. Governing load shall be the lesser of concrete and CFS.

7. For conditions not covered by this table, use the Simpson Strong-Tie® Anchor Designer™ software available at strongtie.com.

8. Wind design values include SDC A&B.

9. For installation in sand-lightweight concrete, concrete values shall be be multiplied by 0.68.

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The crimp anchor is an easy-to-install expansion anchor for use in concrete and grout-filled block. The pre-formed curvature along the shaft creates an expansion mechanism that secures the anchor in place and eliminates the need for a secondary tightening procedure. This speeds up anchor installation and reduces the overall cost.

Five crimp anchor head styles are available to handle different applications that include fastening wood or light-gauge steel, attaching concrete formwork, hanging overhead support for sprinkler pipes or suspended ceiling panels.

Material: Carbon steel

Coating: Zinc plated and mechanically galvanized

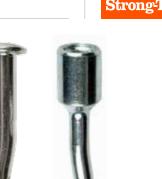
Codes: Factory Mutual 3031136 for the %" rod coupler.

Head Styles: Mushroom, rod coupler, countersunk, tie-wire and duplex

Installation:

Warning: Industry studies show that hardened fasteners can experience performance problems in wet or corrosive environments. Accordingly, with the exception of the duplex anchor, use these products in dry, interior and non-corrosive environments only.

- 1. Drill a hole using the specified diameter carbide bit into the base material to a depth of at least 1/2" deeper than the required embedment.
- 2. Blow the hole clean of dust and debris using compressed air. Overhead application need not be blown clean. Where a fixture is used, drive the anchor through the fixture into the hole until the head sits flush against the fixture.
- 3. Be sure the anchor is driven to the required embedment depth. The rod coupler and tie-wire models should be driven in until the head is seated against the surface of the base material.



SIMPSON



Rod Coupler



Not Valid for

Internationa **Building Code**



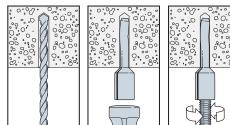
Countersunk Head

Duplex

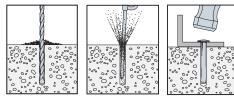
Installation Sequence

Rod Coupler

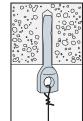
Anchors



Mushroom Head



Tie-Wire

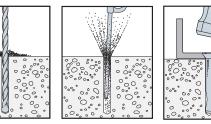


Duplex

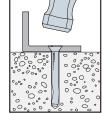


Duplex-head anchor may be removed with a claw hammer

Countersunk Head



Tie-Wire



Crimp Drive® Anchor Product Data

Size	Model	Head	Drill Bit	Min. Fixture	Min.	Quantity		
(in.)	No.	Style/Finish	Diameter (in.)	Hole Size (in.)	Embed. (in.)	Pkg. Quantity	Carton Quantity	
³ ⁄16 Х 1 ¼	CD18114M				7⁄8	100	1600	
3∕16 Х 2	CD18200M				1 1⁄4	100	500	
3⁄16 X 21⁄2	CD18212M		3/	1/	1 1⁄4	100	500	
3⁄16 Х З	CD18300M		3⁄16	1⁄4	1 1⁄4	100	500	
³∕16 X З 1⁄2	CD18312M				1 1⁄4	100	500	
3∕16 X 4	CD18400M				1 1⁄4	100	500	
1⁄4 x 1	CD25100M				7⁄8	100	1,600	
1⁄4 x 1 1⁄4	CD25114M	Mushroom head/			7⁄8	100	1,600	
1⁄4 x 1 1⁄2	CD25112M	zinc plated		5⁄16	1 1⁄4	100	1,600	
1⁄4 x 2	CD25200M		1/		1 1⁄4	100	500	
1⁄4 x 21⁄2	CD25212M		1/4		1 1⁄4	100	500	
1⁄4 x 3	CD25300M				1 1⁄4	100	500	
1⁄4 x 31⁄2	CD25312M				1 1⁄4	100	500	
1⁄4 x 4	CD25400M	-			1 1⁄4	100	500	
3∕8 X 2	CD37200M			7/	1 3⁄4	25	125	
3∕8 x 3	CD37300M	-	3/8	7⁄16	1 3⁄4	25	125	
1⁄4 x 3	CD25300MG	Mushroom head/ mechanically galvanized	1⁄4	5⁄16	1 1⁄4	100	500	
1/4" rod coupler	CD25114RC	Rod coupler/	3⁄16	N/A	1 1⁄4	100	500	
3/8" rod coupler	CD37112RC	zinc plated	1⁄4	N/A	11/2	50	250	
³ ⁄16 X 2 ¹ ⁄2	CD18212C				11⁄4	100	500	
3⁄16 Х З	CD18300C	-	3⁄16	1⁄4	11⁄4	100	500	
3∕16 х 4	CD18400C				1 1⁄4	100	500	
1⁄4 x 1 1⁄2	CD25112C				1 1⁄4	100	500	
1⁄4 x 2	CD25200C	Countersunk head/ zinc plated			1 1⁄4	100	500	
1⁄4 x 21⁄2	CD25212C				1 1⁄4	100	500	
1⁄4 x 3	CD25300C		1⁄4	5⁄16	1 1⁄4	100	500	
1⁄4 x 31⁄2	CD25312C				1 1⁄4	100	400	
1⁄4 x 4	CD25400C				1 1⁄4	100	400	
1⁄4 x 3	CD25300CMG	Countersunk head/		5/	1 1⁄4	100	500	
1⁄4 x 4	CD25400CMG	mechanically galvanized ¹	1⁄4	5⁄16 -	1 1⁄4	100	400	
1/4" Tie-Wire	CD25118T	Tie-Wire/zinc plated	1⁄4	N/A	1 1/8	100	500	
1/4" duplex	CD25234D	Duplex head/zinc plated	1/4	5⁄16	1 1⁄4	100	500	

1. Mechanical galvanizing meets ASTM B695, Class 55, Type 1. Intended for some pressure-treated wood sill plate applications. Not for use in other corrosive or outdoor environments. See pp. 18–21 for details.

Length Identification Head Marks on Mushroom and Duplex-Head Crimp Anchors (corresponds to length of anchor — inches)

Mark		А	В	C	D	E	F
From	1	1 1⁄2	2	21⁄2	3	31⁄2	4
Up to but not including	1 1⁄2	2	21⁄2	3	31⁄2	4	41⁄2

Tension and Shear Loads in Normal-Weight Concrete

					Tensio	n Load	Shear	r Load	
Size (in.)	Drill Bit Diameter	Embed. Depth	Min. Min. Spacing Edge Dist.	f' _C ≥ 2,000 psi Concrete	f' _C ≥ 4,000 psi Concrete	f' _C ≥ 2,000 psi Concrete	f' _C ≥ 4,000 psi Concrete		
	(in.)	(in.)	(in.)	(in.)	Allowable Load (lb.)	Allowable Load (lb.)	Allowable Load (lb.)	Allowable Load (lb.)	
Mushroom Head									
3⁄16	3⁄16	1 1⁄4	3	3	145	250	340	450	
1⁄4	1⁄4	1 1⁄4	3	3	175	275	395	610	
3/8	3⁄8	1 3⁄4	4	4	365	780	755	1,305	
				Duplex	Head				
1/4	1⁄4	1 1⁄4	3	3	175	275	395	610	
				Tie-	Wire				
1⁄4	1⁄4	1 1⁄8	3	3	155	215	265	325	
				Rod Co	oupler ⁴				
1⁄4	3⁄16	1 1⁄4	3	3	145	250	_	_	
3/8	1⁄4	1 1⁄2	4	4	265	600	_	_	

1. The allowable loads listed are based on a safety factor of 4.0.

2. The minimum concrete thickness is 1 ½ times the embedment depth.

3. Allowable loads may be linearly interpolated between concrete strengths listed.

4. For rod coupler, mechanical and plumbing design codes may prescribe lower allowable loads; verify with local codes.



Tension and Shear Loads in Sand-Lightweight Concrete over Metal Deck

Size (in.)	Drill Bit Diameter (in.)	Embed. Depth (in.)	Min. Spacing (in.)	Min. Edge Dist. (in.)	Tension Load (Install in Concrete) f'c ≥ 3,000 psi Concrete Allowable Load (Ib.)	Tension Load (Install through Metal Deck) f'c ≥ 3,000 psi Concrete Allowable Load (lb.)	Shear Load (Install in Concrete) f'c ≥ 3,000 psi Concrete Allowable Load (lb.)	Shear Load (Install through Metal Deck) f'c ≥ 3,000 psi Concrete Allowable Load (lb.)	
Mushroom Head									
3⁄16	3⁄16	1 1⁄4	4	4	115	85	345	600	
1⁄4	1⁄4	1 1⁄4	4	4	145	130	375	890	
3/8	3⁄8	1 3⁄4	51⁄2	51⁄2	315	330	1,030	1,085	
				Duplex	(Head				
1⁄4	1⁄4	1 1⁄4	4	4	145	130	375	890	
				Tie-	Wire				
1⁄4	1⁄4	1 1/8	3	3	130	90	275	210	
				Rod Co	oupler ⁶				
1⁄4	3⁄16	1 1⁄4	4	4	115	85	_	_	
3⁄8	1⁄4	1 1⁄2	5	5	300	280	_		

1. The allowable loads listed are based on a safety factor of 4.0.

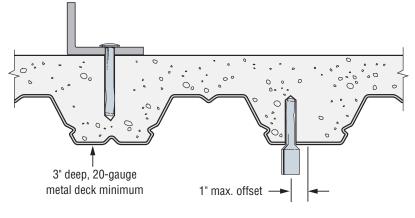
2. The minimum concrete thickness is 11/2 times the embedment depth.

3. Anchors may be installed off-center in the flute, up to 1" from the center of flute.

4. Anchor may be installed in either upper or lower flute.

5. Deck profile shall be 3" deep, 20-gauge minimum.

For rod coupler, mechanical and plumbing design codes may prescribe lower allowable loads; verify with local codes.



Sand-Lightweight Concrete Over Metal Deck

Zinc Nailon[™] Pin Drive Anchors

Zinc Nailon[™] anchors are low-cost, easy-to-install anchors for applications under static loads.

Features:

- · Available with carbon and stainless-steel pins
- Pin and head configuration designed to make anchor tamper-resistant

Materials: Body - Die-cast Zamac 3 alloy Pin - Carbon steel; Type 304 stainless steel

Code: Meets Federal Specification A-A-1925A, Type 1

Installation:

Caution: Not for use in overhead applications.



Caution: Nailon anchors are not recommended for eccentric tension (prying) loads - capacity will be greatly reduced in such applications

- 1. Drill a hole in base material using a carbide drill bit the same diameter as the nominal diameter of the anchor to be installed. Drill the hole to specified embedment depth, plus 1/4" for pin extension, and blow hole clean using compressed air. Alternatively, drill the hole deep enough to accommodate embedment depth and dust from drilling.
- 2. Position fixture and insert Nailon anchor.
- 3. Tap with hammer until flush with fixture, then drive pin until flush with top of head.



Anchors

Size	Carbon Steel Pin	Stainless Steel Pin		Quantity			
(in.)	Model No.	Model No.	Box	Carton	Bulk		
3⁄16 X 7⁄8	ZN18078	—	100	1,600	3,000		
1⁄4 X 3⁄4	ZN25034	ZN25034SS	100	500	2,000		
1⁄4 x 1	ZN25100	ZN25100SS	100	500	1,500		
1⁄4 x 1 1⁄4	ZN25114	ZN25114SS	100	500	1,500		
1⁄4 x 1 1⁄2	ZN25112	ZN25112SS	100	500	1,000		
1⁄4 x 2	ZN25200	ZN25200SS	100	400	1,000		
1⁄4 x 21⁄2	ZN25212	ZN25212SS	100	400	1,000		
1⁄4 x 3	ZN25300	ZN25300SS	100	400	1,000		

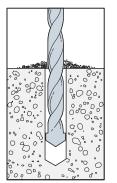
Zinc Nailon[™] Product Data

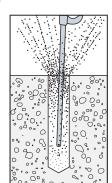
Allowable Tension and Shear Loads for Zinc Nailon[™] in Normal-Weight Concrete

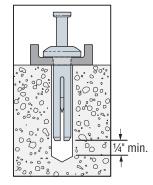
	DIII Dit	Freehard	Ultimate	Load (lb.)	Allowable Load (lb.)1		
Size (in.)	Drill Bit Dia.	Embed. Depth	f' _C ≥ 3,0	000 psi	f' _c ≥ 3,000 psi		
	(in.)	(in.)	Tension	Shear	Tension	Shear	
3⁄16	3⁄16	5⁄8	460	465	115	115	
		5⁄8	590	635	150	160	
1⁄4	1⁄4	3⁄4	780	765	195	190	
		1 1⁄2	1,050	1,050	265	265	

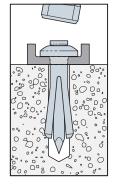
1. The allowable loads are based on a safety factor of 4.0.

Installation Sequence









Tie-Wire Wedge Anchor

The Tie-Wire anchor is a wedge-style expansion anchor for use in normalweight concrete or in concrete over metal deck. With a tri-segmented, dual-embossed clip, the Tie-Wire anchor is ideal for the installation of acoustic ceiling grid and is easily set with the claw of a hammer.

Features:

- 1/4" eyelet for easy threading of wire
- · Sets with claw of hammer
- Tri-segmented clip each segment adjusts independently to hole irregularities
- Dual embossments on each clip segment enable the clip to undercut into the concrete, increasing follow-up expansion
- Wedge-style expansion anchor for use in normal weight concrete or concrete over metal deck

Material: Carbon steel

Coating: Zinc plated

Installation:

Size

(in.)

1/4 x 1 1/2

- Drill a hole at least $1 \frac{1}{2}$ " deep using a $\frac{1}{4}$ " diameter carbide tipped bit
- Drive the anchor into the hole until the bottom of the head is flush with the base material
- Set the anchor by prying/pulling the head with the claw end of the hammer

Drill Bit

Diameter

(in.)

1⁄4

Eyelet

Hole Size

(in.)

1⁄4





Tie-Wire Anchor

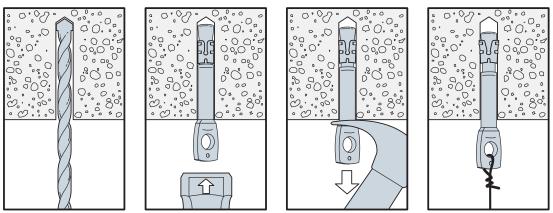
Ancho	or

Model

No.

TW25112

Installation Sequence



Quantity

Carton

500

Box

100

SIMPSON

Tie-Wire Wedge Anchor

Tie-Wire Anchor — Allowable Tension and Shear Loads in Normal-Weight Concrete

		Embed	Critical	Critical	Tensio	n Load	Shear	r Load
Size in.	Drill Bit Diameter	Depth in.	Fad Dist On sain a		f'c ≥ 2,500 psi (17.2 MPa)		f'c ≥ 2,500 psi (17.2 MPa)	
(mm)	in.	(mm) (mm)	(mm)	Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)	
1/4 (6.4)	1/4	1 ½ (38)	2½ (64)	5 (127)	1,155 (5.1)	290 (1.3)	380 (1.7)	95 (0.4)

1. The allowable loads listed are based on a safety factor of 4.0.

2. The minimum concrete thickness is 11/2 times the embedment depth.

Tie-Wire Anchor — Allowable Tension and Shear Loads in the Soffit of Normal-Weight Concrete or Sand-Lightweight Concrete over Metal Deck

		Embed	Critical	Critical	Critical Tension Load		Shear Load	
Size in.	Drill Bit Diameter	Depth in.	oth End Dist. ⁵ Spacing		f'c ≥ 3,000 psi (20.7 MPa)		f'c ≥ 3,000 psi (20.7 MPa)	
(mm)	in.	(mm)	(mm)		Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)
1⁄4 (6.4)	1/4	1 ½ (38)	2½ (64)	5 (127)	1,155 (5.1)	290 (1.3)	460 (2.0)	115 (0.5)

1. The allowable loads listed are based on a safety factor of 4.0.

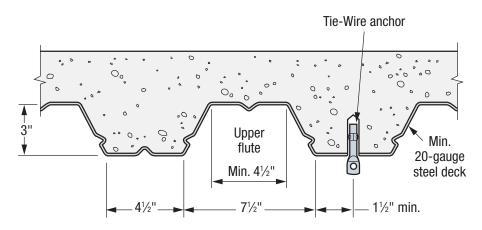
2. The minimum concrete thickness is 1 ½ times the embedment depth.

3. Metal deck must be minimum 20-gauge thick with minimum yield strength of 33 ksi.

4. Anchors installed in the bottom flute of the steel deck must have a minimum edge distance of 11/2" away

from inclined edge of the bottom flute. See the figure below.

5. Critical end distance is defined as the distance from the end of the slab in the direction of the flute.



Installation in the Soffit of Concrete over Metal Deck

Connectors for Cold-Formed Steel Construction

Drop-In Internally Threaded Anchor (DIAB)

Expansion Shell Anchors for Use in Solid Base Materials

Simpson Strong-Tie introduces a new, redesigned Drop-In Anchor (DIAB) that provides easier installation into base materials. Improved geometry in the preassembled expansion plug improves setting capability so the anchor installs with 40% fewer hammer strikes than previous versions. These displacement-controlled expansion anchors are easily set by driving the plug toward the bottom of the anchor using either the hand-or power-setting tools. DIAB anchors feature a positive-set marking indicator at the top of the anchor — helping you see more clearly when proper installation has taken place.

Use a Simpson Strong-Tie fixed-depth stop bit to take the guesswork out of drilling to the correct depth. The fluted design of the tip draws debris away from the hole during drilling, allowing for a cleaner installation.

Features:

- New design offers easier installation then previous drop-in anchor design sets with 40% fewer hammer hits
- Positive-set marking system indicates when anchor is properly set
- Lipped drop-in version available for flush installation
- Hand- and power-setting tools available for fast, easy and economical installation
- Fixed-depth stop bit helps you drill to the correct depth every time

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

Material: Carbon steel Coating: Zinc plated









Drop-In

Lipped Drop-In

Drop-In Anchor Hand Setting Tool

Model No.	For Use With	Box Quantity
DIABST25	DIAB25, DIABL25	10
DIABST37	DIAB37, DIABL37	10
DIABST50	DIAB50, DIABL50	10
DIABST62	DIAB62	5
DIABST75	DIAB75	5

1. Setting tools sold separately; tools may be ordered by the piece.

Drop-In Anchor Power Setting Tool

Model No.	For Use With	Box Quantity
DIABST25-SDS	DIAB25, DIABL25	10
DIABST37-SDS	DIAB37, DIABL37	10
DIABST50-SDS	DIAB50, DIABL50	10



Hand Setting Tool



Power Setting Tool

Drop-In Internally Threaded Anchor (DIAB)



DIAB — Allowable Tension and Shear Loads in Normal-Weight Concrete

Model No.	Rod Size in. (mm)	Drill Bit Diameter in.	Embed Depth in. (mm)	Critical Edge Dist. in. (mm)	Critical Spacing in. (mm)	f' _C ≥ 2,500 psi (17.2 MPa)				f' _c ≥ 4,000 psi (27.6 MPa)			
						Tension Load		Shear Load		Tension Load		Shear Load	
						Ultimate Ib. (kN)	Allowable Ib. (kN)	Ultimate Ib. (kN)	Allowable Ib. (kN)	Ultimate Ib. (kN)	Allowable Ib. (kN)	Ultimate Ib. (kN)	Allowable Ib. (kN)
DIAB25 DIABL25	1⁄4 (6.4)	3%8	1 (25)	3 (76)	4 (102)	1,565 (7.0)	390 (1.7)	1,840 (8.2)	460 (2.0)	1,965 (8.7)	490 (2.2)	1,840 (8.2)	460 (2.0)
DIAB37 DIABL37	³ ⁄8 (9.5)	1/2	1 %16 (40)	41⁄2 (114)	6 (152)	2,950 (13.1)	740 (3.3)	4,775 (21.2)	1,195 (5.3)	3,910 (17.4)	980 (4.4)	4,775 (21.2)	1,195 (5.3)
DIAB50 DIABL50	1⁄2 (12.7)	5%8	2 (51)	6 (152)	8 (203)	5,190 (23.1)	1,300 (5.8)	6,760 (30.1)	1,690 (7.5)	6,515 (29.0)	1,630 (7.3)	6,760 (30.1)	1,690 (7.5)
DIAB62	5% (15.9)	7⁄8	21⁄2 (64)	7½ (191)	10 (254)	7,010 (31.2)	1,755 (7.8)	12,190 (54.2)	3,050 (13.6)	9,060 (40.3)	2,265 (10.1)	12,190 (54.2)	3,050 (13.6)
DIAB75	³ ⁄ ₄ (19.1)	1	31⁄8 (79)	9 (229)	12½ (318)	9,485 (42.2)	2,370 (10.5)	15,960 (71.0)	3,990 (17.7)	11,660 (51.9)	2,915 (13.0)	15,960 (71.0)	3,990 (17.7)

1. The allowable loads listed are based on a safety factor of 4.0.

2. Refer to allowable load-adjustment factors for edge distance and spacing on p. 217.

3. Allowable loads may be linearly interpolated between concrete strength listed.

4. The minimum concrete thickness is 1 ½ times the embedment depth.

DIAB — Allowable Tension and Shear Loads in Soffit of Sand-Lightweight Concrete over Metal Deck

Model No.	Rod Size in. (mm)	Drill Bit Diameter in.	Embed Depth in. (mm)	Critical End Dist. ⁶ in. (mm)	Critical Spacing in. (mm)	f' _C ≥ 3,000 psi (20.7 MPa)				
						Tensio	n Load	Shear Load		
						Ultimate Ib. (kN)	Allowable Ib. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)	
DIAB37 DIABL37	3⁄8 (9.5)	1/2	1 %16 (40)	41⁄2 (114)	6 (152)	2,895 (12.9)	725 (3.2)	3,530 (15.7)	885 (3.9)	
DIAB50 DIABL50	1⁄2 (12.7)	5⁄8	2 (51)	6 (152)	8 (203)	4,100 (18.2)	1,025 (4.6)	4,685 (20.8)	1,170 (5.2)	

1. The allowable loads listed are based on a safety factor of 4.0.

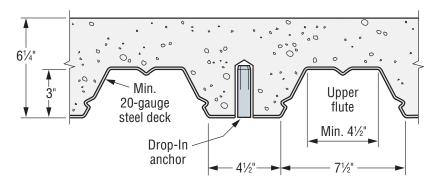
2. Allowable loads may not be increased for short-term loading due to wind or seismic forces.

3. Refer to allowable load-adjustment factors for edge distance and spacing on p. 217.

4. Anchors were installed in the center of the bottom flute of the steel deck.

5. Metal deck must be minimum 20-gauge thick with minimum yield strength of 33 ksi.

6. Critical end distance is defined as the distance from end of the slab in the direction of the flute.



Lightweight Concrete over Metal Deck



DIAB — Load-Adjustment Factors in Normal-Weight Concrete and Sand-Lightweight Concrete over Metal Deck: Edge Distance and Spacing, Tension and Shear Loads

Edge	Size	1⁄4	3⁄8	1/2	5⁄8	3⁄4
Dist.	Ccr	3	4 ½	6	7½	9
Cact	Cmin	1 ¾	2 5⁄8	3½	4 ¾	5 ¼
(in.)	fcmin	0.77	0.77	0.77	0.77	0.77
1 3⁄4		0.77				
2		0.82				
21/2		0.91				
2%		0.93	0.77			
3		1.00	0.82			
31⁄2			0.88	0.77		
4			0.94	0.82		
43⁄8			0.98	0.85	0.77	
41/2			1.00	0.86	0.78	
5				0.91	0.82	
51⁄4				0.93	0.83	0.77
51/2				0.95	0.85	0.79
6				1.00	0.89	0.82
61⁄2					0.93	0.85
7					0.96	0.88
71/2					1.00	0.91
8						0.94
81⁄2						0.97
9						1.00

Edge Distance Tension (f_c)

1. cact = actual edge distance at which anchor is installed (inches).

2. ccr = critical edge distance for 100% load (inches).

3. *cmin* = minimum edge distance for reduced load (inches).

4. fc = adjustment factor for allowable load at actual edge distance.

5. f_{ccr} = adjustment factor for allowable load at critical edge distance. f_{ccr} is always = 1.00.

6. *fcmin* = adjustment factor for allowable load at minimum edge distance. 7. *fc* = *fcmin* + [(1 - fcmin) (cact - Cmin) / (Ccr - Cmin)].

Spacing Tension (fs)

-	Size	1/	3/	1/	5/	3/
Spacing		1⁄4 4	³ / ₈ 6	1/2 8	5% 10	3/4
Sact	Scr	•	-	3		12½
(in.)	Smin	1½	21/4	-	3 ³ ⁄ ₄	4 ³ ⁄ ₄
	f smin	0.72	0.72	0.80	0.80	0.80
1½		0.72				
2		0.78	0.70			
21/4		0.80	0.72			
21/2		0.83	0.74			
3		0.89	0.78	0.80		
31⁄2		0.94	0.81	0.82		
3¾		0.97	0.83	0.83	0.80	
4		1.00	0.85	0.84	0.81	
41⁄2			0.89	0.86	0.82	
43⁄4			0.91	0.87	0.83	0.80
5			0.93	0.88	0.84	0.81
51⁄2			0.96	0.90	0.86	0.82
6			1.00	0.92	0.87	0.83
61⁄2				0.94	0.89	0.85
7				0.96	0.90	0.86
71⁄2				0.98	0.92	0.87
8				1.00	0.94	0.88
81⁄2					0.95	0.90
9					0.97	0.91
91⁄2					0.98	0.92
10					1.00	0.94
101/2						0.95
11						0.96
11 1/2						0.97
12						0.99
121⁄2						1.00

1. sact = actual spacing distance at which anchor is installed (inches).

2. s_{cr} = critical spacing distance for 100% load (inches).

3. smin = minimum spacing distance for reduced load (inches).

4. $f_{\rm S}$ = adjustment factor for allowable load at actual spacing distance.

5. f_{scr} = adjustment factor for allowable load at critical spacing distance. f_{scr} is always = 1.00.

6. f_{smin} = adjustment factor for allowable load at minimum spacing distance. 7. $f_s = f_{smin} + [(1 - f_{smin}) (s_{act} - s_{min}) / (s_{cr} - s_{min})].$

Edge Distance Shear (fc)

Edge	Size	1⁄4	3⁄8	1/2	5⁄8	3⁄4
Dist.	Ccr	3	4 ½	6	7 ½	9
Cact	Cmin	1 ¾	2 %	3½	4 3%	51⁄4
(in.)	f cmin	0.54	0.54	0.64	0.64	0.64
1 3⁄4		0.54				
2		0.63				
21/2		0.82				
2%		0.86	0.54			
3		1.00	0.63			
31⁄2			0.75	0.64		
4			0.88	0.71		
43%			0.97	0.77	0.64	
41/2			1.00	0.78	0.65	
5				0.86	0.71	
51⁄4				0.89	0.74	0.64
51⁄2				0.93	0.77	0.66
6				1.00	0.83	0.71
61⁄2					0.88	0.76
7					0.94	0.81
71/2					1.00	0.86
8						0.90
81⁄2						0.95
9						1.00

1. cact = actual edge distance at which anchor is installed (inches).

- 2. ccr = critical edge distance for 100% load (inches).
- 3. cmin = minimum edge distance for reduced load (inches).

4. f_c = adjustment factor for allowable load at actual edge distance. 5. f_{corr} = adjustment factor for allowable load at critical edge distance.

fccr is always = 1.00. 6. fcmin = adjustment factor for allowable load at minimum edge distance. 7. fc = fcmin + [(1 - fcmin) (cact - Cmin) / (Ccr - Cmin)].

Spacing Shear (f_s)

o .	Size	1⁄4	3⁄8	1/2	5/8	3⁄4
Spacing	Scr	4	6	8	10	12 ½
Sact	Smin	1½	2 1⁄4	3	3 ¾	4 ¾
(in.)	f smin	1.00	1.00	1.00	1.00	1.00
1 1/2		1.00				
2		1.00				
21⁄4		1.00	1.00			
21/2		1.00	1.00			
3		1.00	1.00	1.00		
31/2		1.00	1.00	1.00		
3¾		1.00	1.00	1.00	1.00	
4		1.00	1.00	1.00	1.00	
41/2			1.00	1.00	1.00	
43⁄4			1.00	1.00	1.00	1.00
5			1.00	1.00	1.00	1.00
51/2			1.00	1.00	1.00	1.00
6			1.00	1.00	1.00	1.00
61⁄2				1.00	1.00	1.00
7				1.00	1.00	1.00
71/2				1.00	1.00	1.00
8				1.00	1.00	1.00
81⁄2					1.00	1.00
9					1.00	1.00
91⁄2					1.00	1.00
10					1.00	1.00
101/2						1.00
11						1.00
11 1/2						1.00
12						1.00
121/2						1.00

1. s_{act} = actual spacing distance at which anchor is installed (inches).

2. s_{cr} = critical spacing distance for 100% load (inches).

 $3. s_{min} =$ minimum spacing distance for reduced load (inches).

4. f_s = adjustment factor for allowable load at actual spacing distance. 5. f_{scr} = adjustment factor for allowable load at critical spacing distance.

 f_{scr} is always = 1.00.

6. fsmin = adjustment factor for allowable load at minimum spacing distance.

7. fs = fsmin + [(1 - fsmin) (Sact - Smin) / (Scr - Smin)].

Drop-In Internally Threaded Anchor (DIA)

Drop-In anchors are internally threaded drop-in expansion anchors for use in flush-mount applications in solid base materials. Available in stainless steel (DIA), short (DIAS) or coil-thread (DIAC) versions. Minimum thread engagement should be equal to the nominal diameter of the threaded insert.

Features:

- Lipped edge (DIAS) eliminates need for precisely drilled-hole depth
- Available in coil-thread version for $\ensuremath{1\!/}{2^{"}}$ and $\ensuremath{3\!/}{4^{"}}$ coil-threaded rod
- Short length (DIAS) enables shallow embedment to help avoid drilling into rebar or pre-stressed/post-tensioned cables
- Short Drop-In anchors include a setting tool compatible with the anchor to ensure consistent installation

Material: Carbon and stainless steel

Coating: Carbon steel; zinc plated

Codes: Drop-In — DOT; Factory Mutual 3017082; Underwriters Laboratories File Ex3605. Meets requirements of Federal Specifications A-A-55614, Type I.

Short Drop-In - Factory Mutual 3017082 and Underwriters Laboratories File Ex3605.

Caution: The load tables list values based upon results from the most recent testing and may not reflect those in current code reports. Where code jurisdictions apply, consult the current reports for applicable load values.

Installation:

Anchors

- Drill a hole in the base material using the appropriate diameter carbide drill bit as specified in the table. Drill the hole to the specified embedment depth plus 1/8" for flush mounting. Blow the hole clean using compressed air. Overhead installations need not be blown clean.
- Insert designated anchor into hole. Tap with hammer until flush against surface.
- Using the designated drop-in setting tool, drive expander plug toward the bottom of the anchor until shoulder of setting tool makes contact with the top of the anchor.
- Minimum thread engagement should be equal to the nominal diameter of the threaded insert.
- **Caution:** Oversized holes will make it difficult to set the anchor and will reduce the anchor's load capacity.

Material Specifications

Anchor		Component Material		
Component	Zinc-Plated Carbon Steel	Type 303/304 Stainless Steel	Type 316 Stainless Steel	
Anchor body	Meets minimum 70,000 psi tensile	AISI 303. Meets chemical requirements of ASTM A582	Type 316	
Expander plug	Meets minimum 50,000 psi tensile	AISI 303	Type 316	
Thread	UNC / Coil-thread	UNC	UNC	

Note: DIA37S, DIA50C and DIA75C are not available in stainless steel.







SIMPSON

Strong

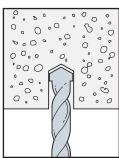
Drop-In Stainless Steel

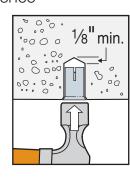
Short Drop-In



Coil-Thread Drop-In

Installation Sequence





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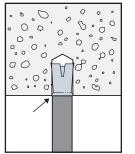
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Drop-In Internally Threaded Anchor (DIA)



Drop-In Anchor Product Data — Stainless Steel

Rod Size	Type 303/304 Stainless	Type 316 Stainless	Drill Bit			Thread	Quantity		
(in.)	Model No.	Model No.	DiameterThreadLength(in.)(per in.)(in.)			Box	Carton		
1⁄4	DIA25SS	DIA256SS	3⁄8	20	1	3⁄8	100	500	
3⁄8	DIA37SS	DIA376SS	1⁄2	16	1 %16	5⁄8	50	250	
1/2	DIA50SS	DIA506SS	5⁄8	13	2	3⁄4	50	200	
5⁄8	DIA62SS	—	7⁄8	11	21⁄2	1	25	100	
3⁄4	DIA75SS		1	10	31⁄8	1 1⁄4	20	80	

Short Drop-In Anchor Product Data

Rod Size	Model	Drill Bit Diameter	Bolt Thread	Body	Thread Length	Qua	ntity
(in.)	No.	(in.)	(per in.)	Length (in.)	(in.)	Box	Carton
3⁄8	DIA37S1	1/2	16	3⁄4	1⁄4	100	500
1/2	DIA50S1	5⁄8	13	1	5⁄16	50	200

1. A dedicated setting tool is included with each box of DIA37S and DIA50S.

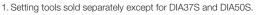
Coil-Thread Drop-In Anchor Product Data

1	Rod Size	Carbon Steel	Drill Bit Diameter	Bolt Thread	Body Length	Thread Length	Qua	ntity
	(in.)	Model No.	(in.)	(per in.)	(in.)	(in.)	Box	Ctn.
	1⁄2	DIA50C1	5⁄8	6	2	3⁄4	50	200
	3⁄4	DIA75C1	1	5	31⁄8	1 1⁄4	20	80

1. DIA50C and DIA75C accept 1/2" and 3/4" coil-thread rod, respectively.

Drop-In Anchor Setting Tool Product Data

Model No.	For Use With	Box Qty.
DIAST25	DIA25SS, DIA256SS	10
DIAST37	DIA37SS, DIA376SS	10
DIAST50	DIA50SS, DIA506SS, DIA50C	10
DIAST62	DIA62SS	5
DIAST75	DIA75SS, DIA75C	5



2. Setting tools for use with carbon and stainless-steel drop-in anchors.

3. Setting tools may be ordered by the piece.

Drop-In Anchor (DIA) Power Setting Tool

Model No.	For Use With	Box Qty.
DIAST37S-SDS	DIA37S	10
DIAST50S-SDS	DIA50S	10

Also sold by the piece.

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Drop-In Anchor Stop Bit

Model No.	Drill Bit Diameter (in.)	Drop-In Anchor (in.)	Drill Depth (in.)		
MDPL037DIA	3⁄8	1⁄4	1 1⁄16		
MDPL050DIA	1/2	3⁄8	13/16		
MDPL062DIA	1/2	1/2	1 11⁄16		
MDPL050DIAS	5⁄8	3⁄8	1 1⁄16		
MDPL062DIAS	5⁄8	1/2	21⁄16		



Drop-In Anchor Setting Tool



Power Setting Tool



Stop Bit



Drop-In (Stainless Steel) and Coil-Thread Drop-In (Carbon Steel) Anchors — Allowable Tension Loads in Normal-Weight Concrete

			Critical					Tension Load				
Rod Size in.	Drill Bit Dia.	Embed. Depth in.	Edge Dist.	Critical Spacing in.		"c ≥ 2,000 ps 8 MPa) Conc		f' _c ≥ 3,000 psi (20.7 MPa) Concrete		$f'_C \ge 4,000 \text{ psi}$ (27.6 MPa) Concrete		
(mm) (in	(in.)	(mm)	in. (mm)	(mm)	Ultimate Ib. (kN)	Std. Dev. Ib. (kN)	Allowable lb. (kN)	Allowable Ib. (kN)		Allowable Ib. (kN)		
1⁄4	3⁄8	1	3	4	1,400	201	350	405	1,840	451	460	
(6.4)		(25)	(76)	(102)	(6.2)	(0.9)	(1.6)	(1.8)	(8.2)	(2.0)	(2.0)	
³ ⁄8	1/2	1 %16	41⁄2	6	2,400	251	600	795	3,960	367	990	
(9.5)		(40)	(114)	(152)	(10.7)	(1.1)	(2.7)	(3.5)	(17.6)	(1.6)	(4.4)	
¹ / ₂	5/8	2	6	8	3,320	372	830	1,178	6,100	422	1,525	
(12.7)		(51)	(152)	(203)	(14.8)	(1.7)	(3.7)	(5.2)	(27.1)	(1.9)	(6.8)	
5⁄8	7⁄8	21⁄2	71⁄2	10	5,040	689	1,260	1,715	8,680	971	2,170	
(15.9)		(64)	(191)	(254)	(22.4)	(3.1)	(5.6)	(7.6)	(38.6)	(4.3)	(9.7)	
³ ⁄ ₄	1	31⁄8	9	12½	8,160	961	2,040	2,365	10,760	1,696	2,690	
(19.1)		(79)	(229)	(318)	(36.3)	(4.3)	(9.1)	(10.5)	(47.9)	(7.5)	(12.0)	

1. The allowable loads listed are based on a safety factor of 4.0.

2. Refer to allowable load-adjustment factors for edge distance and spacing on pp. 223-224.

3. Allowable loads may be linearly interpolated between concrete strengths listed.

4. The minimum concrete thickness is 11/2 times the embedment depth.

Drop-In (Stainless Steel) and Coil-Thread Drop-In (Carbon Steel) Anchors — Allowable Shear Loads in Normal-Weight Concrete

			Oritical			Shear Load						
Rod Size in.	Drill Bit Dia.	Embed. Depth in.	Critical Edge Dist. in.	Critical Spacing in.	(1:	f' _c ≥ 2,000 psi 3.8 MPa) Concre	ete	f' _c ≥ 3,000 psi (20.7 MPa) Concrete	f' _c ≥ 4,000 psi (27.6 MPa) Concrete			
(mm)	(mm) (in.) (mm		(mm) (mm)		Ultimate Ib. (kN)	Std. Dev. Ib. (kN)	Allowable lb. (kN)	Allowable lb. (kN)	Allowable lb. (kN)			
1⁄4	3⁄8	1	31⁄2	4	1,960	178	490	490	490			
(6.4)		(25)	(89)	(102)	(8.7)	(0.8)	(2.2)	(2.2)	(2.2)			
³ ⁄ ₈	1/2	1%	5¼	6	3,240	351	810	925	1,040			
(9.5)		(40)	(133)	(152)	(14.4)	(1.6)	(3.6)	(4.1)	(4.6)			
¹ / ₂	5⁄8	2	7	8	7,000	562	1,750	1,750	1,750			
(12.7)		(51)	(178)	(203)	(31.1)	(2.5)	(7.8)	(7.8)	(7.8)			
⁵ ⁄8	7⁄8	21⁄2	8¾	10	11,080	923	2,770	2,770	2,770			
(15.9)		(64)	(222)	(254)	(49.3)	(4.1)	(12.3)	(12.3)	(12.3)			
³ ⁄ ₄	1	31⁄8	10½	12½	13,800	1,781	3,450	3,725	4,000			
(19.1)		(79)	(267)	(318)	(61.4)	(7.9)	(15.3)	(16.6)	(17.8)			

1. The allowable loads listed are based on a safety factor of 4.0.

2. Refer to allowable load-adjustment factors for edge distance and spacing on pp. 223-224.

3. Allowable loads may be linearly interpolated between concrete strengths listed.

4. The minimum concrete thickness is 11/2 times the embedment depth.

Anchors



3/8" and 1/2" Short Drop-In Anchor — Allowable Tension and Shear Loads in Sand-Lightweight Concrete Fill over Metal Deck

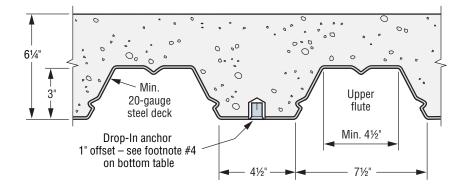
	Rod	Drill Emb.		Tension	Shear Critical		Install through the Lower Flute or Upper Flute of Metal Deck, $f_{\ C}^i \geq 3{,}000$ psi Concrete (20.7 MPa)				
Model No.	Size	Bit Dia.	Depth			n Load	Shear	Shear Load			
	(in.)	(in.)	(in.) (iii.)			()		Allowable (lb.)	Ultimate (lb.)	Allowable (lb.)	
DIA37S	3⁄8	1/2	3⁄4	6	7	8	1,344	335	1,649	410	
DIA50S	1/2	5⁄8	1	8	9%	10%	1,711	430	2,070	515	

1. The allowable loads listed are based on a safety factor of 4.0.

2. Allowable loads may not be increased for short-term loading due to wind or seismic forces.

3. Refer to allowable load-adjustment factors for edge distances and spacing on pp. 223-224.

4. Anchors were installed with a 1" offset from the centerline of the flute.







%" and ½" Short Drop-In Anchor — Allowable Tension and Shear Loads in Normal-Weight Concrete

		Drill		Tension	Shear		Normal	-Weight Con	crete, f' _C ≥	2,500 psi	Normal	-Weight Con	crete, $f'_{C} \ge$	4,000 psi
Model Rod No. (in.)	Bit Dia.	Depth	Critical Edge	Critical Critical Edge Spacing	Spacing Tension Load		Shear Load		Tension Load		Shear Load			
	(in.)	(in.)	(in.)	Distance (in.)	Distance (in.)	(in.)	Ultimate (lb.)	Allowable (lb.)	Ultimate (lb.)	Allowable (lb.)	Ultimate (lb.)	Allowable (lb.)	Ultimate (lb.)	Allowable (lb.)
DIA37S	3⁄8	1/2	3⁄4	41⁄2	51⁄4	3	1,500	375	2,274	570	2,170	540	3,482	870
DIA50S	1⁄2	5⁄8	1	6	7	4	2,039	510	3,224	805	3,420	855	5,173	1,295

1. The allowable loads listed are based on a safety factor of 4.0.

2. Allowable loads may not be increased for short-term loading due to wind or seismic forces.

3. Refer to allowable load-adjustment factors for edge distances and spacing on pp. 223–224.

4. Allowable loads may be linearly interpolated between concrete strengths.

5. The minimum concrete thickness is 1 $\!\!\!\!/_2$ times the embedment depth.

36" and 1/2" Short Drop-In Anchor — Allowable Tension and Shear Loads in Hollow-Core Concrete Panel

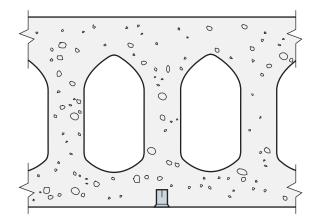
		Drill		Tension	Shear		Н	ollow Core Concrete	si	
Model Size No. (in)	Bit	Bit Depth	Critical Edge	Critical Edge	Critical Spacing	Tension Load		Shear Load		
	(in.)		Distance (in.)	Distance (in.)	(in.)	Ultimate (lb.)	Allowable (lb.)	Ultimate (lb.)	Allowable (lb.)	
DIA37S	3⁄8	1⁄2	3⁄4	41⁄2	51⁄4	3	1,860	465	3,308	825
DIA50S	1⁄2	5⁄8	1	6	7	4	2,650	660	4,950	1,235

1. The allowable loads listed are based on a safety factor of 4.0.

2. Allowable loads may not be increased for short-term loading due to wind or seismic forces.

3. Refer to allowable load-adjustment factors for edge distances and spacing on pp. 223-224.

4. Allowable loads may be linearly interpolated between concrete strengths.



Hollow-Core Concrete Panel (anchor can be installed below web or hollow core)



Drop-In (Stainless Steel), Coil Thread (Carbon Steel) and Short Drop-In Anchors — Allowable Load-Adjustment Factors in Normal-Weight Concrete: Edge Distance and Spacing, Tension and Shear Loads

How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- 2. Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the edge distance (*cact*) or spacing (*sact*) at which the anchor is to be installed.

Edge	Size	1⁄4	3⁄8	1/2	5⁄8	3⁄4
Dist.	Ccr	3	4 ½	6	7 ½	9
Cact	Cmin	1¾	2 %	31/2	4 ¾	5 ¼
(in.)	fcmin	0.65	0.65	0.65	0.65	0.65
13⁄4		0.65				
2		0.72				
21/2		0.86				
2%		0.90	0.65			
3		1.00	0.72			
31⁄2			0.81	0.65		
4			0.91	0.72		
43%			0.98	0.77	0.65	
41/2			1.00	0.79	0.66	
5				0.86	0.72	
51⁄4				0.90	0.75	0.65
51⁄2				0.93	0.78	0.67
6				1.00	0.83	0.72
61⁄2					0.89	0.77
7					0.94	0.81
71/2					1.00	0.86
8						0.91
81⁄2						0.95
9						1.00

Edge Distance Tension (fc)

See notes below.

Edge Distance Shear (fc)

Edge	Size	1⁄4	3⁄8	1/2	5⁄8	3⁄4
Dist.	Ccr	31/2	5 ¼	7	8¾	10½
Cact	Cmin	1 ¾	25%	31/2	43%	5 ¼
(in.)	<i>f</i> cmin	0.45	0.45	0.45	0.45	0.45
1 3⁄4		0.45				
2		0.53				
21/2		0.69				
25%		0.73	0.45			
3		0.84	0.53			
31⁄2		1.00	0.63	0.45		
4			0.74	0.53		
43%			0.82	0.59	0.45	
41/2			0.84	0.61	0.47	
5			0.95	0.69	0.53	
51⁄4			1.00	0.73	0.56	0.45
51⁄2				0.76	0.59	0.48
6				0.84	0.65	0.53
61⁄2				0.92	0.72	0.58
7				1.00	0.78	0.63
71/2					0.84	0.69
8					0.91	0.74
81⁄2					0.97	0.79
8¾					1.00	0.82
9						0.84
91⁄2						0.90
10						0.95
101/2						1.00

1. cact = actual edge distance at which anchor is installed (inches).

2. ccr = critical edge distance for 100% load (inches).

3. *cmin* = minimum edge distance for reduced load (inches).

4. f_C = adjustment factor for allowable load at actual edge distance.

5. f_{ccr} = adjustment factor for allowable load at critical edge distance. f_{ccr} is always = 1.00.

6. fcmin = adjustment factor for allowable load at minimum edge distance.

 $7. \ \textit{fc} = \textit{fcmin} + [(1 - \textit{fcmin}) (\textit{Cact} - \textit{Cmin}) / (\textit{Ccr} - \textit{Cmin})].$

- 4. The load adjustment factor (f_c or f_s) is the intersection of the row and column.
- 5. Multiply the allowable load by the applicable load adjustment factor.
- 6. Reduction factors for multiple edges or spacing are multiplied together.

Spacing Tension and Shear (fs)

	Size	1⁄4	3⁄8 9	3⁄8	1/210	1/2	5⁄8	3⁄4
	Ε	1	3⁄4	1 ½	1	2	2 ½	31/8
Sact (in.)	Scr	4	3	6	4	8	10	12 ½
()	Smin	2	1 ½	3	2	4	5	61⁄4
	<i>f</i> smin	0.50	0.50	0.50	0.50	0.50	0.50	0.50
11⁄2			0.50					
2		0.50	0.67		0.50			
21⁄2		0.63	0.83		0.63			
3		0.75	1.00	0.50	0.75			
31⁄2		0.88		0.58	0.88			
4		1.00		0.67	1.00	0.50		
41⁄2				0.75		0.56		
5				0.83		0.63	0.50	
5½				0.92		0.69	0.55	
6				1.00		0.75	0.60	
6¼						0.78	0.63	0.50
7						0.88	0.70	0.56
8						1.00	0.80	0.64
9							0.90	0.72
10							1.00	0.80
11								0.88
12								0.96
121/2								1.00

1. E = Embedment depth (inches).

2. sact = actual spacing distance at which anchors are installed (inches).

3. s_{cr} = critical spacing distance for 100% load (inches).

s_{min} = minimum spacing distance for reduced load (inches).

5. f_s = adjustment factor for allowable load at actual spacing distance.

- fsc = adjustment factor for allowable load at critical spacing distance.
 fscr = adjustment factor for allowable load at critical spacing distance.
- 7. fsmin = adjustment factor for allowable load at minimum spacing distance.
- 8. fs = fsmin + [(1 fsmin) (sact smin) / (scr smin)].

9. %" short drop-in (DIA37S).

10. 1/2" short Drop-in (DIA50S).



Short Drop-in Anchors — Allowable Load-Adjustment Factors in Sand-Lightweight Concrete over Metal Deck: Edge Distance and Spacing, Tension and Shear Loads

How to use these charts:

- 1. The following tables are for reduced edge distance and spacing.
- 2. Locate the anchor size to be used for either a tension and/or shear load application.
- 3. Locate the edge distance (*cact*) or spacing (*sact*) at which the anchor is to be installed.

Edge Distance Tension (fc)

			()
Edge	Size	3⁄8	1/2
Dist.	Ccr	6	8
Cact	Cmin	3 ½	4 ¾
(in.)	fcmin	0.65	0.65
31⁄2		0.65	
4		0.72	
41⁄2		0.79	
4¾		0.83	0.65
5		0.86	0.68
51⁄2		0.93	0.73
6		1.00	0.78
61⁄2			0.84
7			0.89
71⁄2			0.95
8			1.00

See notes below.

Edge Distance Shear (f_c)

0		,	
Edgo	Size	3⁄8	1/2
Edge Dist.	Ccr	7	9 3⁄8
Cact	Cmin	31/2	4 ¾
(in.)	fcmin	0.45	0.45
31⁄2		0.45	
4		0.53	
41⁄2		0.61	
4¾		0.65	0.45
5		0.69	0.48
51⁄2		0.76	0.54
6		0.84	0.60
6½		0.92	0.66
7		1.00	0.72
71⁄2			0.78
8			0.84
81⁄2			0.90
9			0.96
93⁄8			1.00

1. cact = actual edge distance at which anchor is installed (inches).

2. c_{CT} = critical edge distance for 100% load (inches).

3. cmin = minimum edge distance for reduced load (inches).

4. f_c = adjustment factor for allowable load at actual edge distance.

5. f_{ccr} = adjustment factor for allowable load at critical edge distance. f_{ccr} is always = 1.00.

6. *fcmin* = adjustment factor for allowable load at minimum edge distance.

7. fc = fcmin + [(1 - fcmin) (Cact - Cmin) / (Ccr - Cmin)].

- 4. The load adjustment factor (f_c or f_s) is the intersection of the row and column.
- 5. Multiply the allowable load by the applicable load adjustment factor.
- 6. Reduction factors for multiple edges or spacing are multiplied together.

Spacing Tension and Shear (fs)

	Size	3⁄8	1/2
Sact	Scr	8	10%
(in.)	Smin	4	5 1⁄4
	f smin	0.50	0.50
4		0.50	
41⁄2		0.56	
5		0.63	
51⁄4		0.66	0.50
6		0.75	0.57
61⁄2		0.81	0.62
7		0.88	0.66
71⁄2		0.94	0.71
8		1.00	0.76
81⁄2			0.80
9			0.85
91⁄2			0.90
10			0.94
10%			1.00

1. sact = actual spacing distance at which anchors are

installed (inches).

 $2. s_{CT} = critical spacing distance for 100% load (inches).$

3. *smin* = minimum spacing distance for reduced load (inches).

4. f_s = adjustment factor for allowable load at actual spacing distance.

5. f_{scr} = adjustment factor for allowable load at critical spacing distance. f_{scr} is always = 1.00.

6. fsmin = adjustment factor for allowable load at minimum spacing distance.

7. fs = fsmin + [(1 - fsmin) (sact - smin) / (scr - smin)].

GCN-MEP Gas-Actuated Concrete Nailer

The GCN-MEP MAG tool is a portable fastener tool for attaching light fixtures to concrete, steel and concrete block (CMU), lightweight concrete over metal deck, and cold formed steel. This tool is ideal for attaching drywall track, furring strips, hat track and angle track using GDP and GDPA collated pins. The GDPMEPMAG tool is actuated with GFC34 fuel cells which means there no need for cords or hoses.

The GCN-MEPMAG tool has a removable magazine and nosepiece which can be removed with no additional hand tools for general maintenance of the tool or for clearing occasional pin jams. This tool also features a pin embedment control for fastening into low strength to high strength base materials.

Features:

- Easy magazine attachment with no extra tools
- 3,300 pins per battery charge
- · Pin-depth control dial
- · Easy nose-piece change out (for 0.25" and 0.300" headed fasteners) with no extra tools
- High-voltage spark for cleaner fuel combustion
- Comfortable "sure grip" rubber handle
- Battery charge indicator light
- Ladder hook

Code: ICC-ES ESR-2811



GCN-MEPMAGKT or GCN-MEPKT (with no magazine)

GCN-MEPMAG

GDP Pins

GDP concrete pins are designed to work with the GCN-MEPMAG (with magazine-attached) gas-actuated concrete nailers as well as with most major-brand gas concrete-nailer tools. The patented 10-fastener strip is designed with break-away plastic. The pins are designed for use in A36 and A572 steel, concrete and CMU block.

Codes: ICC-ES ESR-2811; Florida FL15730; City of L.A. RR25837

GDPS Pins

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The GDPS pins are also designed to work in the GCN-MEPMAG gas-actuated nailer tool for installation into A36 and A572 structural steel. The step-shank pin, with a smaller-diameter tip, facilitates easier penetration into the steel, while the larger diameter upper shank provides more shear resistance and successful installation.

Codes: ICC-ES ESR-2811; Florida FL15730

Spiral Knurl Gas Pins

GDPSK gas pins are designed for attaching plywood and OSB to cold-formed steel studs. The spiral knurl provides a positive lock and resists back out. Installed with the GCN-MEPMAG, the GDPSK-138 gas pin provided faster installation and setup times, which contributes to lower labor costs. The hardened pins quickly and cleanly pierce the cold-form steel and leave the pin head flush with the wood fixture. The 1%" length pin can be used for ½"-¾" thick plywood, and 22-14 gauge steel.







GCN-MEP Gas-Actuated Concrete Nailer

GDP and GDPS Pins - Shear and Tension Loads

					Base Material			A	ttached Materi	al
Model	Diameter	Spacing	Normal-Weig	ght Concrete ¹		A36 Steel ²		Co	ld-Formed Ste	el ³
Туре	(in.)		2,000 psi	4,000 psi	1⁄8"	3⁄16"	1⁄4"	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)
		0'-4"	135	225	855	675	615	445	580	1,055
		0'-6"	90	150	570	450	410	295	385	700
GDP 0.106	0.100	0'-8"	70	115	430	340	305	225	290	525
GDP	0.106	1'-0"	45	75	285	225	205	150	195	350
		2'-0"	25	40	145	115	100	75	95	175
		3'-0"	15	25	95	75	65	50	65	115
		0'-4"	—	—	—	540	795	430	560	1,015
		0'-6"	—	—	—	360	530	285	370	675
GDPS	0.118/0.102	0'-8"	_	—	_	270	400	215	280	505
GDP3	0.110/0.102	1'-0"	—	—	—	180	265	145	185	340
		2'-0"	_	—	_	90	135	70	95	170
		3'-0"	—	_	_	60	90	50	60	115
				Allow	able Tension L	.oad⁴ (lb.)				·
GDP	0.106	_	30	30	125	210	220	195	255	460
GDPS	0.118/0.102		_	_	_	95	170	190	250	405

1. For Normal-Weight Concrete the minimum edge distance and spacing is 3" and 4" respectively with 34" minimum embedment.

2. For A36 steel, the minimum edge distance and spacing is 0.5" and 1" respectively. Entire pointed portion of the fastener must fully penetrate steel base material to obtain tabulated values.

3. Cold-formed steel (CFS) values are based on AISI-S100, Section E4. Reference General Notes for CFS properties.

4. Governing load is the lesser of the base material and CFS.

GDPSK Pin — Shear and Tension Loads

			Base Material: Cold-Formed Steel						
Model Type	Diameter (in.)	Spacing		1inimum trength	50 ksi Minimum Yield Strength				
			33 mils (20 ga.)	43 mils (18 ga.)	54 mils (16 ga.)	68 mils (14 ga.)			
		Allowa	ble Shear Load⁴	(lb./ft.)					
		0'-4"	210	267	450	654			
	0.109	0'-6"	140	178	300	436			
GDPSK		0'-8"	105	134	225	327			
GDPSK		1'-0"	70	89	150	218			
		2'-0"	35	45	75	109			
		3'-0"	23	30	50	73			
		Allowa	able Tension Loa	d4 (lb.)					
GDPSK	0.109	_	30	48	92	73			

1. Entire pointed portion of the fastener must penetrate through the steel to obtain tabulated values.

The allowable tension and shear values are for the fastener only. Members connected to the steel must be investigated separately in accordance with accepted design criteria.

3. Fastener is to be installed in the center of the stud flange.

4. Minimum edge distance and spacing is 13/16" and 4", respectively.

Gas-Actuated Concrete Nailing

GDP Pins

GDP concrete pins are designed to work with the GCN-MEPMAG gas-actuated concrete nailer as well as with most major-brand gas-actuated concrete-nailer tools. The patented 10-fastener strip is designed with break-away plastic. The pins are designed for use in A36 and A572 steel, concrete, CMU block and sand-lightweight concrete over metal deck.

Codes: ICC-ES ESR-2811; Florida FL15730; City of L.A. RR25837

0.106"-Diameter Shank Drive Pins

Model No.	Length (in.)	Qty. Pins / Pack +1 Fuel Cell	Packs / Carton	Compatible with These Tools
GDP-50KT	1/2	1,000	5	
GDP-62KT	5⁄8	1,000	5	
GDP-75KT	3⁄4	1,000	5	Simpson Strong-Tie GCN-MEPMAG
GDP-100KT	1	1,000	5	Others: TF1100, C3, TF1200
GDP-125KT	11⁄4	1,000	5	
GDP-150KT	11⁄2	1,000	5	

GDPS Pins

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The GDPS pins are also designed to work in the GCN-MEPMAG gas-actuated nailer tool for installation into steel and concrete. The step-shank pin, with a smaller-diameter tip, facilitates easier penetration, while the larger-diameter upper shank provides more shear resistance and successful installation.

0.118"/0.102"-Diameter Stepped-Shank Drive Pins

Model No.	Length	Qty. Pins / Pack	Baaka / Cartan	Compatible Tools		
mouel no.	Adel No. (in.) + 1 fuel cell Packs / Carton		Simpson Strong-Tie	Others		
GDPS-50KT	1/2	1,000	5		TF1100, C3, TF1200	
GDPS-62KT	5/8	1,000	5	GCN-MEPMAG		
GDPS-75KT	3⁄4	1,000	5			

GDP U.S. Patent 605,016

GDPS

Gas-Actuated Concrete Nailing

SIMPSON Strong-Tie

Spiral Knurl Gas Pins

GDPSK gas pins are designed for attaching plywood and OSB to coldformed-steel studs. The spiral knurl provides a positive lock and resists back-out. Installed with the GCN-MEPMAG, the GDPSK-138 gas pin provides faster installation and setup times, which contributes to lower labor costs. The hardened pins quickly and cleanly pierce the cold-formed steel and leave the pin head flush with the wood fixture. The 1%" length pin can be used for ½"–¾" thick plywood, and 14–22 gauge steel.



GDPSK

Spiral Knurl Gas Pins

Model No.	Length (in.)	Qty. Pins / Pack + 1 fuel cell	Packs / Carton	Compatible with These Tools
GDPSK-138KT	1%	1,000	5	Simpson Strong-Tie: GCN-MEPMAG Others: TF1100, C3

GWL-100 Lathing Washer and GMR-2 Magnetic Ring

The GWL-100 lathing washer is used with the GCN-MEPMAG tool and attaches lath to the wall surface for overlaying scratch coats, brown coats and stucco. The washers are held onto the nose of the tool with the new GMR-2 magnetic ring and are attached to the substrate (including concrete and CMU) with GDP pins, which fasten through the washer. No extra tools are needed to install the magnetic ring to the nosepiece of the tool.

Lathing Washer and Magnetic Ring

Model No.	Description	Pack Qty.	Carton Qty.	
GWL-100	Lathing washer, 1" diameter	1,000	5,000	
GMR-2	Magnetic ring for GCN150	10	900	

Lathing washer and magnetic rings are sold separately.



GWL-100



GMR-2

Fuel Cell

The GFC34 fuel cell is designed to operate with the GCN-MEPMAG and GCN-MEP, and with many major-brand gas-actuated concrete-nailer tools. The fuel cell provides 1,200 shots and can operate at temperatures between 20° and 120°F ($-6^{\circ}-49^{\circ}$ C). The fuel cells are offered individually or in a two-per-pack clamshell. Additionally, one fuel cell is included with each pack of 1,000 pins.

Gas Fuel Cells for the GCN-MEP

Model No.	Description	Pack Qty.	Packs/ Carton	Compatible with These Tools
GFC34	34-gram fuel cells	12	—	Simpson Strong-Tie: GCN-MEP and GCN-MEPMAG
GFC34-RC2	(2) 34-gram fuel cells	2	6	Others: TrakFast® TF1100, Trak-It® C3



GFC Fuel Cell

PT-27 General Purpose Tool

The PT-27 is a semi-automatic and fast-cycling fastening tool that is engineered for continuous use, high reliability and low maintenance. This versatile tool fires a variety of fastener types and lengths.

Key Fastening Applications

- Acoustical ceilings
- · Electrical applications
- Sill plates
- Drywall track
- Water proofing material and/or lathing

Specifications

- Fastener Length: 1/2" 21/2" (3" or 4" washered)
- Fastener Type: .300" or 8 mm-headed fasteners or 1/4" - 20 threaded studs
- Firing Action: Semi-automatic
- Load Caliber: 0.27 strip loads, brown through red (levels 2-5)
- Length: 131/2"
- Weight: 5 lb., 4 oz.

Tool is sold in a rugged tool box complete with:

- Operator's manual
- Spall suppressor
- Tools for disassembly
- Safety glasses / ear plugs
- Tool lubricant

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- Cleaning brushes
- · Operator's exam and caution sign

Replacement Parts - PT-27

Description	Model No.		
Annular Spring	PT-301014		
Ball Bearing (6 mm)	PT-301013		
Barrel	PT-301006		
Baseplate	PT-301009		
Piston – Concave (includes ring)	PT-301217		
Piston – Flat (includes ring)	PT-301903		
Piston Ring	PT-301208		
Piston Stop	PT-301012		
Shear Clip	PT-301011		

For tool repair and maintenance kits and complete tool schematics and parts list, visit strongtie.com.

Complementary Products

Extension pole tool for the PT-27 available in 6' and 8' lengths



PEPT6 Tool and PEPT8 Tool





The full line of Simpson Strong-Tie® powder loads and fasteners begins on p. 232.



PTP-27S and PTP-27SMAGR Premium Tools



Features:

- PTP-27S: Automatic fastening: no sliding barrel, just load and shoot
- PTP-27SMAGR: Fully automatic tool with rotating fastener magazine

Both Tools Feature:

- · Adjustable power for fastening versatility
- Operator comfort: cushioned grip, reduced recoil and sound dampening muffler for quiet operation
- · No manual resetting of piston required
- · Easy disassembly for cleaning and maintenance
- The PTP-27S can be converted to a magazine tool and the PTP-27SMAGR can be converted to a single-shot tool

Specifications:

- Fastener Length: PTP-27S 1/2" thru 1 5%" PTP-27SMAGR - 1/2" thru 1 1/4"
- Fastener Type: 0.300" or 8mm diameter
- Firing Action: PTP-27S Automatic PTP-27SMAGR - Fully automatic
- Load Caliber: 0.27 strip loads, brown through purple (Levels 2-6)
- Length: 163/4" (PTP-27S), 171/2" (LMAGR)
- Weight: PTP-27S 6.25 lbs. PTP-27SMAGR - 8.1 lbs.

Key Fastening Applications:

PTP-27S:

Anchors

- Conduit clips
- · Ceiling clips
- Drywall track
- Metal decking
- PTP-27SMAGR:
- Drywall track
- Hat channel
- HVAC duct straps

Tool is sold in a rugged tool box

- complete with:
- · Operator's manual
- Spall suppressor
- · Tools for disassembly
- Safety glasses / ear plugs
- Tool lubricant
- Cleaning brushes
- · Operator's exam and caution sign
- Tool box also sold separately
- Gloves

Options:

- Extension pole tool for the PTP-27S available in 6' and 8' lengths. 6' Tool: PET-6PKT 8' Tool: PET-8PKT
- Single shot conversion kit (PTP-27SCON)





Adjustable power increases versatility





The patent-pending quick-disconnect baseplate makes it easy to convert the PTP-27SMAGR from a magazine to a single-shot tool

PTP-27S and PTP-27SMAGR Premium Tools

Common Repair Parts — PTP-27S

Description	Model No.		
Baseplate	PTP-273800		
Nosepiece	PTP-273820		
Piston	PTP-273320		
Piston Disc	PTP-273306		
Rubber Returner	PTP-273305		

1. Complete tool schematics and parts list available at **strongtie.com**.

Common Repair Parts – PTP-27SMAGR

Description	Model No.		
Magazine Body	PTP-741000		
Nosepiece	PTP-740001		
Nosepiece Screw	PTP-750002		
Piston	PTP-842001		
Piston Disc	PTP-730071CH		
Rubber Returner	PTP-742101		



Extension Pole Tool (for the PTP-27S)



Rotating magazine allows for installation flexibility



Collated pins for fully automatic fastening and quick loading



Simpson Strong-Tie[®] Powder-Actuated Fasteners are featured on pp. 232–233.

0.300" Headed Fasteners with 0.157" Shank Diameter

Length	Model		Pack	Carton	Compati	ble Tools	
(in.)	No.	Description	Qty.	Qty.	Simpson Strong-Tie	Others	
1/2	PDPA-50	0.157 x ½"	100	1,000			
1⁄2 knurled	PDPA-50K	0.157 x 1⁄2" knurl	100	1,000	PTP-27L PTP-27S PT-27 PT-27HD PT-22		
5∕% knurled	PDPA-62K	0.157 x %" knurl	100	1,000			
3⁄4	PDPA-75	0.157 x ¾"	100	1,000		721, D-60, U-2000 and most other low-velocity tools.	
1	PDPA-100	0.157 x 1"	100	1,000			
1 1⁄4	PDPA-125	0.157 x 1 ¼"	100	1,000			
1 1⁄2	PDPA-150	0.157 x 1 ½"	100	1,000	PT-22GS		
1 1⁄8	PDPA-187	0.157 x 1 1/8"	100	1,000	PT-22P PT-22H		
2	PDPA-200	0.157 x 2"	100	1,000			
21⁄2	PDPA-250	0.157 x 21⁄2"	100	1,000			
2 7/8	PDPA-287	0.157 x 21⁄8"	100	1,000			

This model available in mechanically galvanized finish (PDPA-287MG)

0.300" Headed Fasteners with 0.157" Shank Diameter 10 Pin Collation

Longth	Model		Pack	Carton	Compati	ble Tools
Length (in.)	No.	Description	Qty.		Simpson Strong-Tie	Others
1/2	PDPAS-50	0.157 x ½"	100	1,000		
1⁄2 knurled	PDPAS-50K	0.157 x ½" knurl	100	1,000	PTP-27L PTP-27S PT-27	
% knurled	PDPAS-62K	0.157 x %" knurl	100	1,000		
3/4	PDPAS-75	0.157 x ¾"	100	1,000		721, D-60, U-2000 and most other low-velocity tools.
1	PDPAS-100	0.157 x 1"	100	1,000		
1 1⁄16	PDPAS-106	0.157 х 1 1⁄16''	100	1,000		
1 1⁄4	PDPAS-125	0.157 x 1 ¼"	100	1,000	PT22,	
1 5⁄16	PDPAS-131	0.157 х 1 %і́6"	100	1,000	PT-22GS PT-22P	
1 1⁄2	PDPAS-150	0.157 x 1 ½"	100	1,000	PT-22H	
1 1⁄8	PDPAS-187	0.157 x 1 1%"	100	1,000		
2	PDPAS-200	0.157 x 2"	100	1,000		
2 1/2	PDPAS-250	0.157 x 21⁄2"	100	1,000		
27⁄8	PDPAS-287	0.157 x 21⁄8"	100	1,000		

0.300" Headed Tophat Fasteners with 0.157" Shank Diameter

Length	Model No.	Description	Pack	Carton	Compatible Tools	
(in.)			Qty.	Qty.	Simpson Strong-Tie	Others
1/2 knurled	PDPAT-50K	0.157 x ½" knurl	100	1,000	PTP-27L PTP-27S PT-27 PT-27HD PT-22 PT-22GS PT-22GS PT-22P PT-22H	DX-460, 721, D-60, U-2000 and most other low-velocity tools.
5% knurled	PDPAT-62K	0.157 x %" knurl	100	1,000		
5% knurled	PDPAT-62KP	0.157 x %" knurl	100	1,000		
3⁄4	PDPAT-75	0.157 x ¾"	100	1,000		
1	PDPAT-100	0.157 x 1"	100	1,000		



PDPAS









Anchors

Pre-Assembled Ceiling Clips — 0.300" Headed Fasteners with 0.157" Shank Diameter

Length	Model		Pack	Pack	Pack	Pack	Carton	Compati	ble Tools
(in.)	No.	Description	Qty.	Qty.	Simpson Strong-Tie	Others			
7/8	PCLDPA-87	Ceiling clip with %" pin	100	1,000					
1 1⁄16	PCLDPA-106	Ceiling clip with 1 1⁄16" pin	100	1,000	PTP-27L PTP-27S PT-27 PT22 PT-22GS PT-22P PT-22H	DX-350, DX-460, System 1 721 and most other tools.			
1%6	PCLDPA-131	Ceiling clip with 15⁄16" pin	100	1,000					
1 1⁄16	PECLDPA-106	Compact ceiling clip with 1 1/16" pin	100	1,000					
1 5%6	PECLDPA-131	Compact ceiling clip with 15⁄16" pin	100	1,000					



PECLOPA

PCLDPA and PECLDPA Series Ceiling Clips — Tension and Oblique Loads in Sand-Lightweight Concrete over Metal Deck¹

			Installe	ed through Lower Flute	e of Metal Deck into C	oncrete	
Model	Shank Diameter	Minimum Embedment		p Deck oncrete Fill²	11/2"-Deep Deck with 2" Concrete Fill ³		
No.	in. (mm)	in. (mm)	Allowable Tension Load Ib. (KN)	Allowable Oblique Load Ib. (kN)	Allowable Tension Load Ib. (KN)	Allowable Oblique Load Ib. (kN)	
PCLDPA-87		³ ⁄ ₄ (19)	115 (0.51)	155 (0.69)	60 (0.27)	175 (0.78)	
PCLDPA-106		1 (25)	140 (0.63)	175 (0.78)	160 (0.72)	240 (1.08)	
PCLDPA-131	0.157 (4.0)	1 ¼ (32)	160 (0.72)	185 (0.83)	180 (0.81)	280 (1.25)	
PECLPDA-106		⁷ / ₈ (22)	80 (0.36)	110 (0.49)	95 (0.42)	110 (0.49)	
PECLPDA-131		1 (25)	120 (0.54)	145 (0.65)	135 (0.60)	175 (0.78)	

1. Fasteners shall not be driven until the concrete has reached a minimum concrete compressive strength of 3,000 psi.

2. The steel deck must have a minimum thickness of 20 gauge (0.0359-inch-thick-base-steel thickness) and a minimum yield strength of 38,000 psi. Oblique loads are applied at a 45-degree angle to the fastener. The fastener must be a minimum of 11/2" from the edge of the deck web and 4" from the end of the deck. The minimum fastener spacing is 4".

3. The steel deck must have a minimum thickness of 20 gauge (0.0359-inch-thick-base-steel thickness) and a minimum yield strength of 38,000 psi. Oblique loads are applied at a 45-degree angle to the fastener. The fastener must be a minimum of 7%" from the edge of the deck web and 4" from the end of the deck. The minimum fastener spacing is 4".



PDPA and PDPAT Pins - Shear and Tension Loads

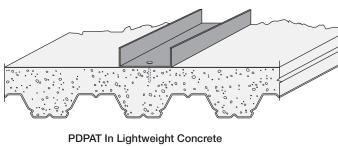
								Base Mater	ial					Attached Material		
Model	Dia.	Pin		rmal Wei Concrete			ightweight (ed Steel De				A36 Steel ²			Cold	-Formed S	teel ³
Туре	(in.)	Spacing	Emb. Depth (in.)	2,500 psi	4,000 psi	Emb. Depth (in.)	Concrete ¹ (Top)	Lower Flute ⁸ (Bottom)	^{3⁄16} "	1⁄4"	3⁄8"	1⁄2"	3⁄4"	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)
	Allowable Shear Load ^{4,5,13} (lb./ft.)															
		0'-6"	1	570	620	1	450	560	820	730	770 ¹⁰	770 ¹⁰	650 ⁹	760 ¹²	990 ¹²	1,80012
		0-0	1¼	720	840	1¼	840	640	020	730	110.5	110.2	000-	700	990	1,000
		0'-8"	1	430	465	1	340	420	615	550	580 ¹⁰	580 ¹⁰	490 ⁹	570 ¹²	745 ¹²	1,350 ¹²
		0-0	1¼	540	630	1¼	630	480	015	000	500.5	000.4	490-	570-	740**	1,300
	0.157	1'-0"	1	285	310	1	225	280	410	365	385 ¹⁰	385 ¹⁰	325°	380 ¹²	495 ¹²	900 ¹²
PDPA	0.157	1-0	1¼	360	420	1¼	420	320	410	305	300	300	323	300	490	900
PDPAT		2'-0"	1	145	155	1	115	140	205	185	195 ¹⁰	195 ¹⁰	165°	190 ¹²	250 ¹²	450 ¹²
		2 -0	1¼	180	210	1¼	210	160	205	100	190	190	105	190	200	400
		3'-0"	1	95	105	1	75	95	135	35 120	130 ¹⁰	130 ¹⁰	110 ⁹	125 ¹²	165 ¹²	300 ¹²
		3-0	1¼	120	140	1¼	140	105	155	120	130.4	130.4	110-	120-	100	300
							AI	llowable Te	nsion Load	l ^{4,5} (lb.)						
	0.157		1	210	310	1	150	145	260	370	380 ¹⁰	530 ¹⁰	195 ⁹	22511	295 ¹¹	535 ¹¹
	0.137		1¼	320	380	1¼	320	170	200	570	500	550	190	225	290	555

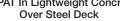
1. For concrete the minimum edge distance and spacing is 31/2" and 5" respectively.

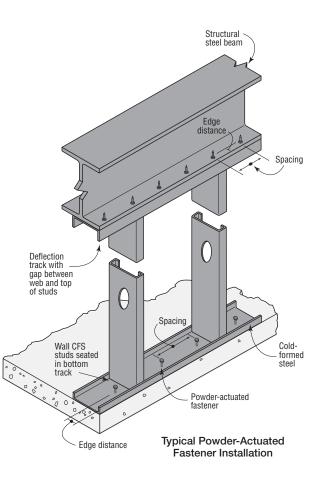
2. For A36 steel, the minimum edge distance and spacing is 0.5" and 1" respectively.

Entire pointed portion of the fastener must fully penetrate steel base material unless noted otherwise.

- Cold-formed steel (CFS) values are based on AISI-S100, Section E4. Reference General Notes for CFS properties.
- 4. Governing load is the lesser of the base material and CFS.
- 5. Allowable loads are based on ICC-ES ESR-2138.
- 6. Concrete shall have a minimum compressive strength of $f_{\rm C}^{\prime}$ = 3,000 psi.
- For steel deck, the minimum depth and thickness is 3" and 33 mil (20 ga.) respectively. Steel deck must have a minimum yield strength of 38,000 psi.
- 8. For installation through steel deck, the minimum edge and end distance is 1% and 4" respectively with 4" minimum spacing.
- Based upon a minimum penetration depth of 0.46" (11.7 mm).
- 10. For applications to structural steel, the fastener must be driven to where at
- least some of the point of the fastener penetrates through the steel substrate. 11. The following CFS allowable tension loads may be used for PDPAT:
- 390 lb. (33 mil), 505 lb. (42 mil), 915 lb. (54 mil). 12. CFS allowable shear load may be multiplied by 1.15 for PDPAT fastener.
- Shear loads listed do not account for indirect tension due to eccentricity of load at the deflection track. Designer to evaluate combined loading as needed.







PDPA in 4,000 psi Normal-Weight Concrete

Fastener	Shank Diameter (in.)	Fastener Length (in.)	Edge Distance (in.)	Spacing in Load Dir. (in.)	Track Width (in.)	Number of Fasteners	Allowable Shear Load (lb.)	
				_	35%	1	240	
		-	21/4	3⁄4	378	2	310	
PDPA-100		I	2 74	—	6	1	240	
	0.157			21⁄4	0	2	510	
	0.157			—	05/	1	325	
PDPA-125		1.25	21⁄4	3⁄4	35%	- 3%	2	490
FUFA-120		1.20	∠ 74	_	6	1	325	
	2		21⁄4	0	2	590		

1. Allowable loads are based on fasteners installed through 16 ga. cold-formed steel (Fy > 33 ksi).

2. Minimum concrete thickness must be three times the fastener length.

3. Edge distance and spcaing are shown in figures below.

PDPA in 3,000 psi Sand-Lightweight Concrete

Fastener	Shank Diameter (in.)	Fastener Length (in.)	Edge Distance (in.)	Spacing in Load Dir. (in.)	Track Width (in.)	Number of Fasteners	Allowable Shear Load (lb.)
				—	35%	1	235
PDPA-100		1	21⁄4	3⁄4	378	2	310
PDPA-100		I	∠ 74	_	6	1	235
				21⁄4	0	2	445
				—	35%	1	245
PDPA-125	0.157	1.25	21⁄4	3⁄4	378	2	455
PDPA-120	0.157	1.20	∠ 74	—	6	1	245
				21⁄4	U	2	530
				_	35%	1	245
PDPA-150		1.5	21⁄4	3⁄4	3%8	2	470
FUFA-150		1.0	∠ 1/4	_	6	1	245
				21⁄4	Ŭ	2	530

1. Allowable loads are based on fasteners installed through 16 ga. cold-formed steel ($F_y > 33$ ksi).

2. Minimum concrete thickness must be three times the fastener length.

3. Edge distance and spcaing are shown in figures below.

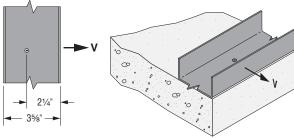


Figure 1: 3%" Track - One Fastener

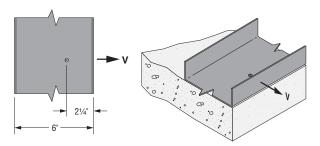


Figure 3: 6" Track - One Fastener

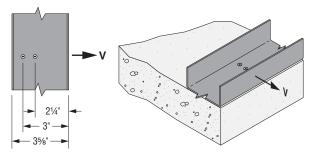


Figure 2: 3%" Track – Two Fasteners

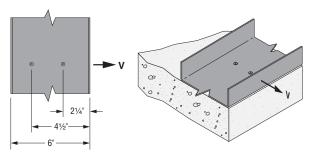


Figure 4: 6" Track – Two Fasteners

SB Anchor Bolt

The SB%x24 anchor bolt offers a load-tested anchorage solution that exceeds the capacity of all of our holdowns that call for a 5/4"-diameter anchor. Similarly, the SB1x30 covers holdowns utilizing a 1"-diameter anchor that exceed the capacity of our SSTB bolts. The SB7/xx24 is designed to maximize performance with minimum embedment for holdowns utilizing a 7/8"-diameter anchor.

SB anchor bolts are code listed by ICC-ES under the 2009, 2012 and 2015 IBC and IRC to meet the requirements of ICC-ES acceptance criteria - AC 399. ICC-ES ESR-2611 is the industry's first code report issued for proprietary anchor bolts evaluated to the criteria of AC 399.

Special Features:

- · Identification on the bolt head showing embedment angle and model
- Sweep geometry to optimize position in form
- · Rolled thread for higher tensile capacity
- · Hex nuts and plate washer fixed in position
- · Available in HDG for additional corrosion resistance

Material: ASTM F-1554, Grade 36

Finish: None. May be ordered HDG. Contact Simpson Strong-Tie.

Installation:

- SB is only for concrete applications poured monolithically except where noted.
- Top nuts and washers for holdown attachment are not supplied with the SB; install standard nuts, couplers and/or washers as required.
- On HDG SB anchors, chase the threads to use standard nuts or couplers or use overtapped products in accordance with ASTM A563 - for example, Simpson Strong-Tie® NUT%-OST, NUT%-OST, NUT1-OST, CNW%-OST, CNW%-OST and CNW1-OST.
- Install SB before the concrete pour using AnchorMates®. Install the SB per the plan view detail.
- Minimum concrete compressive strength is 2,500 psi.
- When rebar is required it does not need to be tied to the SB.

Codes: See p. 11 for Code Reference Key Chart

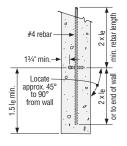
SB Bolts at Stemwall

Dimensions (in.)				Allowable Tension Load (lb.)								
Model No.			Lenath	Min. Embed.	Wind and SDC A&B				Code Ref.			
	Width	Dia.	Lengui	(le)		Midwall	Corner	End Wall	Midwall	Corner	End Wall	
SB%x24	6	5⁄8	24	18	6,675	6,675	6,675	6,675	5,730	5,730	15.	
SB1/8x24	8	7⁄8	24	18	10,470	9,355	6,820	8,795	7,855	5,730	FL,	
SB1x30	8	1	30	24	13,655	9,905	7,220	11,470	8,315	6,065	L5	

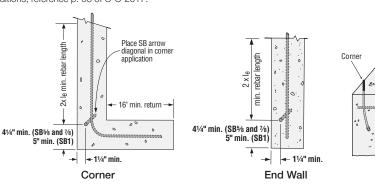
Stemwall Plan Views

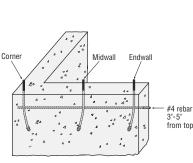
These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. For other SB anchorage conditions, reference p. 56 of C-C-2017.

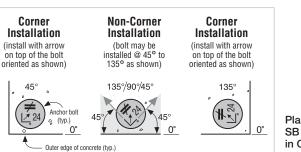


Midwall





Perspective View



(standard on all models) Embedment

6"

Length

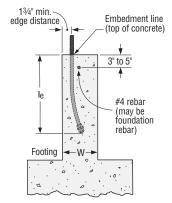
le

SIMPSON

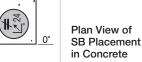
Strong

line (top of concrete)

SB1x30 (Other models similar)







Anchors

SSTB® Anchor Bolt



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The SSTB[®] anchor bolt is designed for maximum performance as an anchor bolt for holdowns and Simpson Strong-Tie[®] Strong-Wall[®]

shearwalls. Extensive testing has been done to determine the design load capacity of the SSTB when installed in many common applications. The Simpson Strong-Tie SSTB anchor bolts are now code listed by ICC-ES under the 2009, 2012 and 2015 IBC® and IRC® to meet the requirements of ICC-ES acceptance criteria AC 399. ICC-ES ESR-2611 is the industry's first code report issued for proprietary anchor bolts evaluated to the criteria of AC 399.

Special Features:

- Identification on the bolt head showing embedment angle and model
- Offset angle reduces side bursting, and provides more concrete cover
- Rolled thread for higher tensile capacity
- Stamped embedment line aids installation
- Available in HDG for additional corrosion resistance
- Material: ASTM F-1554, Grade 36

Finish: None. May be ordered HDG; contact Simpson Strong-Tie. Installation:

- SSTB is suitable for monolithic and two-pour concrete applications.
- Nuts and washers for holdown attachment are not supplied with the SSTB; install standard nuts, couplers and/or washers as required.
- On HDG SSTB anchors, chase the threads to use standard nuts or couplers or use overtapped products in accordance with ASTM A563 — for example Simpson Strong-Tie NUT%-OST, NUT%-OST, CNW%-OST, CNW%-OST.
- Install SSTB before the concrete pour using AnchorMates[®]. Install the SSTB per the plan view detail.
- Minimum concrete compressive strength is 2,500 psi.
- When rebar is required, it does not need to be tied to the SSTB.
- Order SSTBL models (example: SSTB16L) for longer thread length (16L= $5\frac{1}{2}$ ", 20L = $6\frac{1}{2}$ ", 24L = 6", 28L = $6\frac{1}{2}$ "). SSTB and SSTBL load values are the same. SSTB34 and SSTB36 feature $4\frac{1}{2}$ " and $6\frac{1}{2}$ " of thread respectively and are not available in "L" versions.

CMU

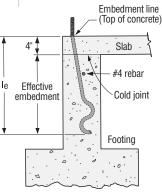
- One horizontal #4 rebar in the second course
- One vertical #4 rebar in adjacent cell for 5%" diameter SSTB
- One vertical #4 rebar in an adjacent cell and additional vertical #4 rebar(s) at 24" o.c. max. for 7/s" diameter SSTBs (two total vertical rebars for end-wall corner, three total vertical rebars for midwall)

Codes: See p. 11 for Code Reference Key Chart

SSTB Bolts at Stemwall

embedment angle 16 and model. 51/2 Embedment line 31/2' Embedment line (top of concrete) when not using (top of concrete) ¥ when not using 2x wood sill plate 11⁄2" 11⁄2" 2x wood sill plate Embedment line Embedment line (top of concrete) (top of concrete) Length Length when using 2x when using 2x wood sill plate wood sill plate le le SSTB16L SSTB16 (other models similar) (other models similar)

See pp. 238–239 for additional installation details.



For two-pour (4" slab) installation loads:

- When using the SSTB20, use the equivalent loads of the SSTB16
- When using the SSTB24, use the equivalent loads of the SSTB20
- When using the SSTB34 or 36, use the equivalent loads of the SSTB28

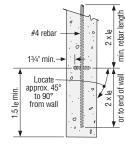
[Madal		D	imensions (in.)		Allowable Tension Load (lb.)						
	Model No.	Stemwall Dia Length		Longth	Min. Embed.	bed. Wind and SDC A&B				SDC C–F		
	NU.	Width	Dia.	Lengui	(l _e)	Midwall	Corner	End Wall ²	Midwall	Corner	End Wall ²	Ref.
	SSTB16	6	5⁄8	175% (16L = 195%)	12%	3,610	3,610	3,610	2,550	2,550	2,550	
	SSTB20	6	5⁄8	21 5% (20L = 245%)	16%	4,315	4,040	4,040	3,145	2,960	2,960	
	SSTB24	6	5⁄8	25% (24L = 281/s)	20 %	5,025	4,470	4,470	3,740	3,325	3,325	15, FL,
	SSTB28	8	7⁄8	29 1/8 (28L = 321/8)	24 7/8	9,900	8,710	7,615	8,315	7,315	6,395	L5
	SSTB34	8	7⁄8	34 7/8	287⁄8	9,900	8,710	7,615	8,315	7,315	6,395	
	SSTB36	8	7⁄8	36 7⁄8	287⁄8	9,900	8,710	7,615	8,315	7,315	6,395	

These products are available with additional corrosion protection. Additional products on

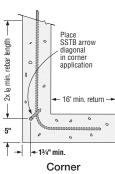
this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. See p. 14 for notes to the Designer.

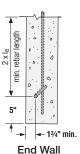
2. SSTB28, SSTB34 and SSTB36 with 37%" end distance allowable loads are 6,605 lb.(Wind and SDC A&B) and 5,550 lb. (SDC C-F).

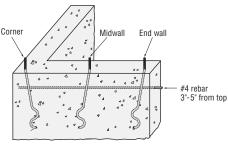


Midwall



Stemwall Plan Views





Perspective View

Anchors

dentification on the

bolt head showing

SSTB® Anchor Bolt

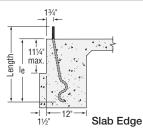


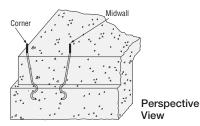
SSTB Bolts at Stemwall: Garage Front

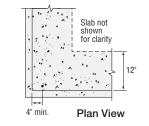
				0					
		Dimensi	ions (in.)			Allowable Ten	sion Load (lb.)		
Model No.	Stemwall	Die	Lawath	Min.	Wind and	SDC A&B	SDC	C–F	Code Ref.
	Width	Dia.	Length	Embed. (l _e)	Step-Down End	Corner	Step-Down End	Corner	
SSTB28	8	7⁄8	297%	24 7⁄8	7,015	7,045	5,895	5,920	15, FL, L5
Length		nwall age Front			Pe	erspective	4" min. – – – 16	ti t	n. 1 View

SSTB Bolts at Slab on Grade: Edge

	Dimensions (in.)								
Model No.	Footing	Die	Longth	Min.	Wind and	SDC A&B	SDC	C–F	Code Ref.
	Width	Dia.	Length	Embed. (I _e)	Midwall	Corner	Midwall	Corner	
SSTB16	12	5⁄8	17 %	12%	5,355	5,355	3,780	3,780	
SSTB20	12	5⁄8	25%	16%	6,550	6,550	4,785	4,785	
SSTB24	12	5⁄8	25%	20%	6,675	6,675	5,790	5,790	
SSTB28	12	7⁄8	297%	24 1/8	13,080	13,080	11,060	11,645	15, FL, L5
SSTB34	12	7⁄8	34%	28 %	13,080	13,080	11,060	11,645	
SSTB36	12	7⁄8	36 7%	28 %	13,080	13,080	11,060	11,645	

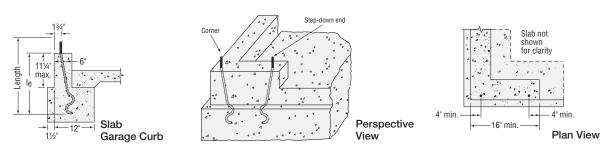






SSTB Bolts at Slab on Grade: Garage Curb

		Dimensi	ons (in.)			Allowable Tension Load (lb.)						
Model No.	Curb Dia.		Curb Dia Laurati		Min. Lenath Embed.		Wind and	Wind and SDC A&B		SDC C-F		
	Width	Dia.	Length	(le)	Step-Down End	Corner	Step-Down End	Corner				
SSTB28	6	7⁄8	29 7/8	24 1/8	10,085	12,375	8,475	10,395	15, FL, L5			



Notes to the Designer:

1. Rebar is required at top of stemwall foundations but is not required for Slab-on-Grade Edge and Garage Curb, or Stemwall Garage Front installations.

2. Minimum end distances for SSTB bolts are as shown in graphics.

3. To obtain LRFD values for cast-in-place bolts, multiply ASD seismic load values by 14 and wind load values 1.6 (1.67 for 2015 IBC).

4. Per Section 1613 of the IBC, detached one- and two-story dwellings in SDC C may use "Wind and SDC A&B" allowable loads.

5. See ESR-2611 for additional information.

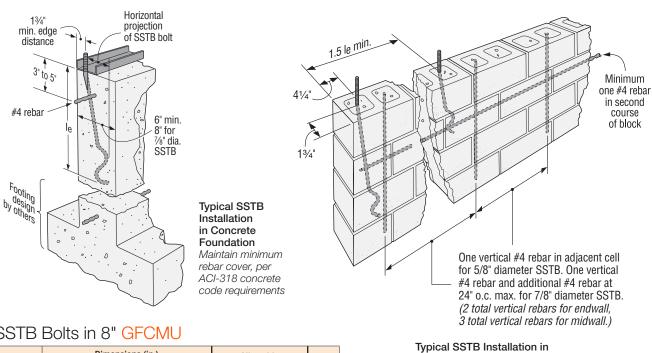
6. Midwall loads apply when anchor is 1.5 le or greater from the end. For bolts acting in tension simultaneously, the minimum bolt center-to-center spacing is 3 le.

Anchors

Connectors for Cold-Formed Steel Construction

SSTB® Anchor Bolt





SSTB	Bolts	s in 8" <mark>GFCN</mark>	1U			
		Dimensions (in.)	Allov			
Model			Min.	Tension	Load (lb.)	Code
No.	Dia.	Length	Embed. (I _e)	Midwall	Corner/ End Wall	Ref.
SSTB16	5⁄8	17 % (16L = 19%)	12%	2,865	1,220	
SSTB20	5⁄/8	21 5% (20L = 245%)	16%	2,865	1,220	
SSTB24	5⁄8	25 % (24L = 281/s)	20%	2,865	1,220	170
SSTB28	7⁄8	291/8 (28L = 321/8)	24 1/8	4,185	3,000	170

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

34 7/8

367/8

- 1. Loads are based on a minimum CMU compressive strength, f'm, of 1,500 psi.
- 2. Minimum end distance required to achieve midwall table loads is 1.5 le.
- 3. Minimum end distance for corner/end wall loads is 41/4"
- 4. Loads may not be increased for duration of load.
- 5. Allowable loads are based on the average ultimate load with a safety factor of 5.0 per ACI 530.

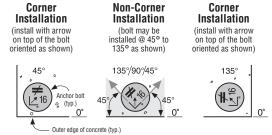
281%

28%

4,185

4 185

Grouted Concrete Block



Plan View of SSTB Placement in Concrete

RFB Retrofit Bolts

7/8

7⁄8

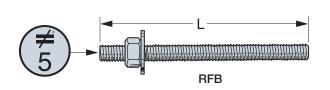
SSTB34

SSTB36

RFBs are clean, oil-free, pre-cut threaded rod, supplied with nut and washer. Offers a complete engineered anchoring system when used with Simpson Strong-Tie® adhesive. Inspection is easy; the head is stamped with rod length and No-Equal symbol for identification after installation.

Material: ASTM F1554 Grade 36

Finish: Zinc-Plated (unless otherwise noted), available in HDG (per ASTM A153); stainless steel (RFB#5x8SS only)



Model No.	Length L (in.)	Bolt Diameter (in.)
RFB#4X4	4	1/2
RFB#4X5	5	1/2
RFB#4X6	6	1/2
RFB#4X7	7	1/2
RFB#4X10	10	1/2
RFB#4x8HDG-R	8	1/2
RFB#5X5	5	5⁄8
RFB#5X8	8	5⁄8
RFB#5X10	10	5⁄8
RFB#5X12HDG-R	12	5⁄8
RFB#5X16	16	5⁄8
RFB#6X10.5	101⁄2	3⁄4

 These products are available with additional corrosion protection.
 Additional products on this page may also be available with this option.
 Check with Simpson Strong-Tie for details.

[.] RFB#4X8HDG-R and RFB#5X12HDG-R are only available with a hot-dip galvanized coating. They are retail packaged and are sold 10 per carton. 2. Washer provided on all RFB (except RFB#5x8SS).

PAB Pre-Assembled Anchor Bolt

The PAB anchor bolt is a versatile cast-in-place anchor bolt ideal for high-tension-load applications, such as rod systems and shearwalls. It features a plate washer at the embedded end sandwiched between two fixed hex nuts and a head stamp for easy identification after the pour.

- Available in diameters from ½" to 1¼" in lengths from 12" to 36" (in 6" increments)
- Available in standard and high-strength steel
- Head stamp contains the No Equal sign, diameter designation and an "H" on high-strength rods

Material:

Standard Steel — ASTM F1554 Grade 36, A36 or A307 — $F_{\rm u}$ = 58 ksi High-Strength Steel (up to 1" dia.) — ASTM A449 — $F_{\rm u}$ = 120 ksi High-Strength Steel (1¼" and 1¼" dia.) — ASTM A193 B7 or F1554 Grade 105 — $F_{\rm u}$ = 125 ksi

Finish: None. May be ordered in HDG; contact Simpson Strong-Tie.

Installation:

Anchors

 On HDG PABs, chase the threads to use standard nuts or couplers or use overtapped products in accordance with ASTM A563; for example, Simpson Strong-Tie[®] NUT %-OST, NUT %-OST, CNW %-OST, CNW %-OST. Some OST couplers are typically oversized on one end of the coupler nut only and will be marked with an "O" on oversized side. Couplers may be oversized on both ends. Contact Simpson Strong-Tie.

Codes: See p. 11 for Code Reference Key Chart

The Simpson Strong-Tie[®] Anchor Designer[™] Software analyzes and suggests anchor solutions using the ACI 318-14 strength-design methodology (or CAN/CSA A23.3 Annex D Limit States Design methodology). It provides cracked- and uncracked-concrete anchorage solutions for numerous Simpson Strong-Tie mechanical and adhesive anchors as well as the PAB anchor bolt. With its easy-to-use graphical user interface, the software makes it easy for the Designer to identify anchorage solutions without having to perform time-consuming calculations by hand.

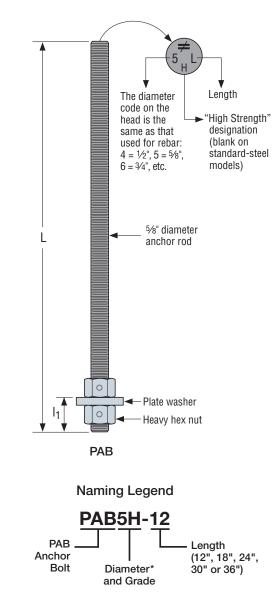
How to specify and order:

When calling out PAB anchor bolts, substitute the desired length for the "XX" in the Root Model Number.

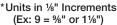
For a $\frac{1}{2}$ anchor bolt, the model number would be PAB5-18 (or PAB5H-18 for high strength).

PAB Anchor Bolt – Standard Steel

Diameter (in.)	Plate Washer Size (in.)	l ₁ (in.)	Root Model No.	Lengths (in.)
1/2	3∕8 x 1 ½ x 1 ½	11⁄8	PAB4-XX	
5⁄8	1⁄2 x 13⁄4 x 13⁄4	1%	PAB5-XX	
3⁄4	1⁄2 x 21⁄4 x 21⁄4	1½	PAB6-XX	12" to 36"
7⁄8	1⁄2 x 21⁄2 x 21⁄2	1%	PAB7-XX	(in 6"
1	% x <mark>3</mark> x 3	17⁄8	PAB8-XX	increments)
1 1/8	5∕8 x <mark>3½</mark> x 3½	2	PAB9-XX	
11⁄4	¾ x 3½ x 3½	21⁄4	PAB10-XX	



Strong



PAB Anchor Bolt – High-Strength Steel

Diameter (in.)	Plate Washer Size (in.)	l ₁ (in.)	Root Model No.	Lengths (in.)
1/2	3% x 1½ x 1½	1 1/8	PAB4H-XX	
5⁄8	½ x 1¾ x 1¾	1%	PAB5H-XX	
3⁄4	1⁄2 x 21⁄4 x 21⁄4	1½	PAB6H-XX	12" to 36"
7⁄8	1⁄2 x 21⁄2 x 21⁄2	1%	PAB7H-XX	(in 6"
1	% x <mark>3</mark> x 3	17⁄8	PAB8H-XX	increments)
11/8	5% x <mark>3½</mark> x 3½	2	PAB9H-XX	
11⁄4	3⁄4 x 31⁄2 x 31⁄2	21⁄4	PAB10H-XX	

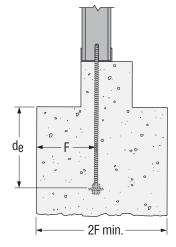
1. Lengths longer than 36" available as special order.

2. Plate washers are designed to develop the capacity of the bolt.

PAB Pre-Assembled Anchor Bolt

PAB Anchor Bolt — Anchorage Solutions

		D011 —			Concrete		-	3.000 psi	Concrete			
Design	Diameter	Anchor	Dimensi	ions (in.)	Tension I	oad (lb.)	Dimensi	ons (in.)	Tension I	_oad (lb.)	Code	
Criteria	(in.)	Bolt	de	F	ASD	LRFD	de	F	ASD	LRFD	Ref.	
	1/2	PAB4	41/2	7	4,270	6,405	4	6	4,270	6,405		
			4	6	4,030	6,720	4	6	4,415	7,360		
	5⁄8	PAB5	6	9	6,675	10,010	5½	8½	6,675	10,010		
			5½	8½	6,500	10,835	5	7½	6,175	10,290		
	3⁄4	PAB6	7½	11 1⁄2	9,610	14,415	7	10½	9,610	14,415		
		D4.D7	6	9	7,405	12,345	5½	81⁄2	7,120	11,870		
	7/	PAB7	9	13½	13,080	19,620	8½	13	13,080	19,620		
)A(in al	7/8	DADZU	9	13½	13,610	22,680	8½	13	13,680	22,805		
Wind		PAB7H	14	21	27,060	40,590	13½	201⁄2	27,060	40,590		
		DADO	8	12	11,405	19,005	7½	11½	11,340	18,900		
	1	PAB8	10½	16	17,080	25,565	10	15	17,080	25,560		
		PAB8H	10½	16	17,150	28,580	10	15	17,460	29,100		
		PADON	16½	25	35,345	53,015	15½	231⁄2	35,345	53,015		
	11/8	PAB9	9	13½	13,610	22,680	8	12	12,495	20,820		
	178	FAD9	12½	19	21,620	32,430	12	18	21,620	32,430		
	11⁄4	PAB10	14	21	26,690	40,035	13½	201⁄2	26,690	40,035	200	
	1/2	PAB4	5	7½	4,270	6,405	4½	7	4,270	6,405	200	
	5⁄8	PAB5	6½	10	6,675	10,010	6	9	6,675	10,010		
	3/4	PAB6	7½	11½	9,060	12,940	7	10½	8,945	12,780		
	/4	1 7.00	8	12	9,610	14,415	7½	11½	9,610	14,415		
		PAB7	9	13½	11,905	17,010	8½	13	11,970	17,100		
	76	TADI	10	15	13,080	19,620	91⁄2	14½	13,080	19,620		
	7/8	7/8	PAB7H	14½	22	25,350	36,215	13½	201⁄2	24,650	35,215	
Seismic				15½	231⁄2	27,060	40,590	14½	22	27,060	40,590	
Oubline		PAB8	11	16½	15,996	22,850	10½	16	16,435	23,480		
	1		11½	17½	17,080	25,625	11	16½	17,080	25,625		
		PAB8H	17	251⁄2	33,045	47,205	16	24	32,720	46,740		
		170011	18	27	35,345	53,015	17	25½	35,345	53,015		
	11/8	PAB9	12½	19	19,795	28,275	12	18	20,255	28,940		
	. /0		13½	201⁄2	21,620	32,430	12½	19	21,620	32,430		
	11⁄4	PAB10	14½	22	25,350	36,215	14	21	26,190	37,415		
	1/4	171010	15	221⁄2	26,690	40,035	14½	22	26,690	40,035		



Design loads are calculated using a full shear cone. Coverage on each side of the bolt shall be a minimum of F or reductions must be taken.

1. Anchorage designs conform to ACI 318-14 and assume cracked concrete with no supplementary reinforcement.

2. Seismic indicates Seismic Design Category C through F and designs comply with ACI 318-14 Section 17.2.3.4. Per Section 1613 of the IBC, detached one- and two-family dwellings in SDC C may use wind values.

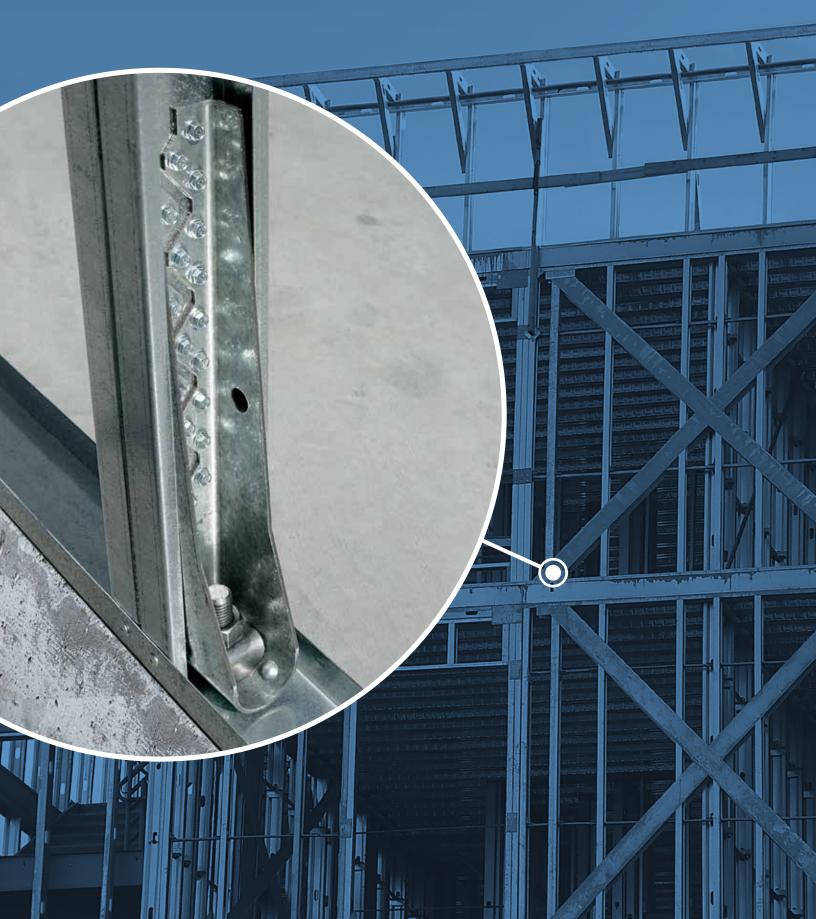
3. Wind includes Sesmic Design Category A and B.

4. Foundation dimensions are for anchorage only. Foundation design (size and reinforcement) by Designer. The registered design professional may specify alternate embedment, footing size, and anchor bolt.

5. Where tension loads are governed by anchor steel, the design provisions from AISC 360-10 are used to determine the tensile steel limit. LRFD values are calculated by multiplying the nominal AISC steel capacity by a 0.75 phi factor and allowable values are calculated by dividing the AISC nominal capacity by a 2.0 omega factor.

6. Where tension loads are governed by an Appendix D concrete limit, the allowable Stress Design (ASD) values are obtained by multiplying Load Factor Resistance Design (LRFD) capacities by 0.7 for Seismic and 0.6 for Wind.





S/HDU Holdowns

The S/HDU series of holdowns combines performance with ease of installation. The pre-deflected geometry virtually eliminates material stretch, resulting in low deflection under load. Installation using self-drilling screws into the studs reduces installation time and saves labor cost.

Material: 118 mil (10 ga.)

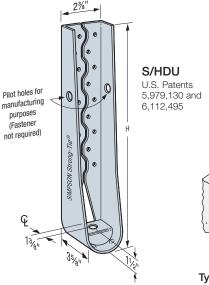
Finish: Galvanized

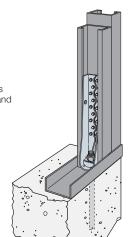
Installation:

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

- Use all specified fasteners; see General Notes
- Use standard #14 self-drilling screws to fasten to studs
- Anchor bolt washer is not required
- See SB, SSTB and PAB anchor bolts on pp. 236–241 for cast-in-place anchorage options
- See SET-XP[®] and AT-XP[®] adhesive products for anchor bolt retrofit options

Codes: See p. 11 for Code Reference Key Chart



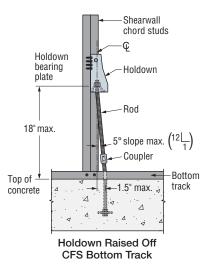


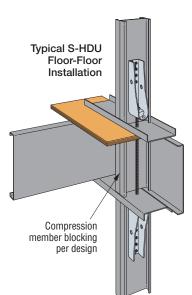
Typical S/HDU Installation

[Faste	eners		ASD	(lb.)	LRFE) (lb.)								
	Model	H (in.)	Anchor Bolt Diameter ¹ (in.)	Stud Fasteners ⁷	Stud Member Thickness ² mil (ga.)	Tension Load	Deflection at ASD Load⁵	Tension Load	Deflection at LRFD Load ⁵	Nominal Tension Load ⁶ (Ib.)	Code Ref.						
					2-33 (2-20)	2,320	0.093	3,705	0.149	5,685							
	S/HDU4	71/8	5/8	(6) #14	2-43 (2-18)	3,825	0.115	6,105	0.190	9,365							
	3/11D04	1 78	78	(0) #14	2-54 (2-16)	3,970	0.093	6,345	0.156	9,730							
					Steel fixture	4,470	0.063	7,165	0.103	12,120							
					2-33 (2-20)	4,895	0.125	8,495	0.250	10,470							
	S/HDU6	10%	54	(10) #14	2-43 (2-18)	6,125	0.119	9,690	0.250	15,460							
	3/ 1000	10%8	5⁄8	9/8	(12) #14	2-54 (2-16)	6,125	0.108	9,785	0.234	15,005						
					Steel fixture	5,995	0.060	9,580	0.136	14,695							
					2-33 (2-20)	6,965	0.103	11,125	0.189	13,165	IP1,						
	S/HDU9	127⁄8	7/8	(10) #14	2-43 (2-18)	9,255	0.125	15,485	0.250	21,810	L2, FL						
	3/HDU9	12'/8	78	(18) #14	2-54 (2-16)	9,990	0.106	15,960	0.225	24,480							
					Steel fixture	12,715	0.125	20,510	0.177	31,455							
					2-33 (2-20)	6,965	0.103	11,125	0.189	13,165							
			7⁄8	(27) #14	2-43 (2-18)	9,595	0.096	15,330	0.162	23,515							
		105/			2-54 (2-16)	9,675	0.110	15,460	0.158	23,710							
	S/HDU11	16%	7/8		2-43 (2-18) ⁶	11,100	0.125	17,500	0.250	24,955							
			with heavy (27) #14	/ -		y (27) #14	(27) #14	(27) #14		(27) #14	2-54 (2-16) ⁶	12,175	0.125	19,445	0.243	29,825	
			hex nut		Steel fixture ⁶	12,945	0.111	20,680	0.163	31,715							

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

- 1. The Designer shall specify the foundation anchor material type, embedment and configuration. Some of the tabulated holdown tension loads exceed the tension strength of typical ASTM A36 or A307 anchor bolts.
- Stud design by Specifier. Tabulated loads are based on a minimum stud thickness for fastener connection.
 ¼" self-drilling screws may be substituted for
- 3. 4 self-drilling screws may be substituted for #14 self-tapping screws.
 4. A begin bay but for the apphor bolt is required to
- 4. A heavy hex nut for the anchor bolt is required to achieve the table loads for S/HDU11.
- 5. Deflection at ASD or LRFD includes fastener slip, holdown deformation and anchor rod elongation for holdowns installed up to 4" above top of concrete. Holdowns may be installed raised, up to 18" above top of concrete, with no load reduction provided that additional elongation of the anchor rod is accounted for.
- 6. The Nominal Tension Load is based on the tested average ultimate (peak) load and is provided for design in accordance with section C5 of AISI S213 that requires a holdown to have a nominal strength to resist the lesser of the amplified seismic load or the maximum force the system can deliver.
- 7. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.





S/LTT, S/DTT and HTT Tension Ties

SIMPSON Strong-Tie

The HTT is a single-piece formed tension tie - no rivets, and a 4-ply formed seat. No washers are required.

S/DTT2Z tension tie is suitable for lighter-duty hold-down applications on single or back-to-back studs, and installed easily with #14 self-drilling screws.

The HTT, S/DTT and S/LTT tension ties are ideal for retrofit or new construction projects. They provide high-strength, post-pour, concrete-to-steel connections.

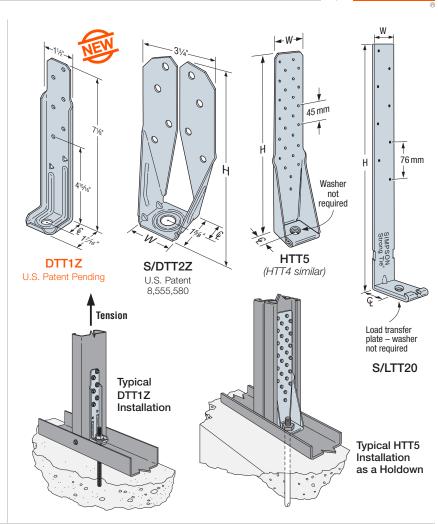
Material: HTT — 111 mil (11 ga.) DTT1Z, S/DTT2Z — 68 mil (14 ga.) S/LTT20B — Strap: 97 mil (12 ga.); Plate: 229 mil (3 ga.)

Finish: HTT, S/LTT — Galvanized; DTT1Z, S/DTT2Z — ZMAX[®] coating

Installation:

- Use all specified fasteners.
- Use the specified number of type of screws to attach the strap portion to the steel stud. Bolt the base to the wall or foundation with a suitable anchor; see table for the required bolt diameter.
- S/DTT2Z requires a standard cut washer (included) be installed between the nut and the seat.
- Do not install S/LTT20B raised off of the bottom track.
- See SB and SSTB Anchor Bolts on pp. 236–239 for anchorage options.
- See SET-XP[®] and AT-XP[®] adhesive products for anchor bolt retrofit options.

Codes: See p. 11 for Code Reference Key Chart



	Dime	ensions	(in.)	Faste	ners	Stud	ASD	(lb.)	LRFD) (lb.)	Nominal	
Model	w	н	ଜ	Anchor Bolt Diameter ¹ (in.)	Stud Fasteners⁵	Member Thickness mil (ga.)	Tension Load	Deflection at ASD Load ³	Tension Load	Deflection at LRFD Load ³	Tension Load⁴ (Ib.)	Code Ref.
DTT1Z	1½	71⁄8	3⁄4	3⁄8	(6) #10	33 (20)	905	0.156	1,270	0.250	3,485	
S/LTT20	2	20	1½	1/2	(8) #10	33 (20)	1,200	0.125	1,890	0.250	4,625	
						33 (20)	1,570	0.138	2,200	0.250	4,265	
S/DTT2Z	1%	6 ¹⁵ ⁄16	¹³ ⁄16	1/2	(8) #14	43 (18)	1,685	0.151	2,355	0.250	5,570	
						2-33 (2-20)	1,735	0.153	2,430	0.250	5,735	IP1,
	01/	103/	13/	5/	(10) #10	33 (20)	3,180	0.104	4,770	0.187	8,215	L2, FL
HTT4	2½	12%	1¾	5⁄8	(18) #10	2-33 (2-20)	4,395	0.125	6,675	0.250	11,835	
						43 (18)	4,240	0.125	6,505	0.250	11,585	1
HTT5	21⁄2	16	1¾	5⁄8	(26) #10	2-43 (2-18)	4,670	0.125	6,970	0.250	12,195	
						1-54 (1-16)	4,150	0.125	6,425	0.250	12,365	1

These products are available with additional corrosion protection. Additional products on

this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. The Designer shall specify the foundation anchor material type, embedment and configuration.

2. Stud design by Specifier. Tabulated loads are based on a minimum stud thickness for fastener connection.

3. Deflection at ASD or LRFD includes fastener slip, holdown deformation and anchor rod elongation for holdowns installed up to 4" above top of concrete. Holdowns may be installed raised, up to 18" above top of concrete, with no load reduction provided that additional elongation of the anchor rod is accounted for. See bottom of p. 243 for installation detail.

4. The Nominal Tension Load is based on the tested average ultimate (peak) load and is provided for design in accordance with section C5 of AISI S213 that requires a tension tie to have a nominal strength to resist the lesser of the amplified seismic load or the maximum force the system can deliver.

5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

S/HDS and S/HDB Holdowns

The S/HD series of holdowns is designed for installation with either screws or bolts into the studs or column. The S/HDS series installs with #14 screws and has been designed to utilize fewer fasteners to reduce installation time. The S/HDB series is ideal for bolt-on applications where the cold-formed stud manufacturer can prepunch the bolt holes.

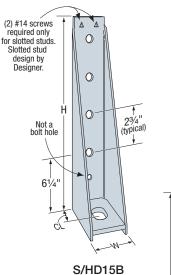
Material: See table

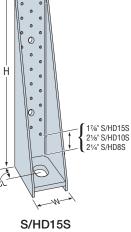
Finish: Simpson Strong-Tie[®] gray paint. Hot-dip galvanized is available; see Corrosion-Information, pp. 18–21.

Installation:

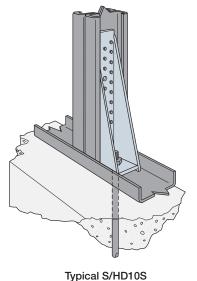
- Use all specified fasteners; some models have extra fastener holes. See General Notes.
- Anchor bolt washer is not required.
- Standard washers are required on stud bolt nuts for model S/HDB.
- Thin wall socket (OD = 2" maximum) is required for S/HD15 to tighten the 1" anchor bolt.
- Stud bolts use A307.
- Boundary members (back-to-back studs) design shall be by Designer.
- S/HDS and S/HDB holdowns can be welded per Designer's recommendation and specification. To tie back-to-back stud members together, the Designer must determine the fasteners required to bind members to act as one unit. Welders and welding procedures shall be qualified as specified in AWS D1.3. Welded connections used for cold-formed steel structural members in which the thickness of the thinnest connected part is 0.18 inch or less shall comply to AISI S100 Specification Section E2.
- See SB, SSTB and PAB Anchor Bolts on pp. 236–241 for anchorage options.
- See SET-XP[®] and AT-XP[®] adhesive products for anchor bolt retrofit options.

Codes: See p. 11 for Code Reference Key Chart

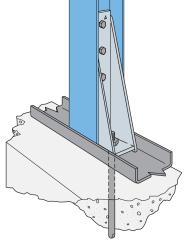




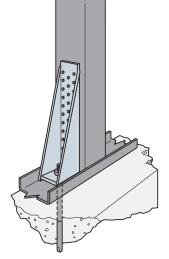




Typical S/HD10S Back-to-Back Stud Installation



Typical S/HD10B PACO Column Installation See Code Report



Typical S/HD10S Heavy-Duty (Large Flange) Stud Application See Code Report

S/HDS and S/HDB Holdowns

					Fast	eners	Ohud Manshau	ASD	(lb.)	LRF) (lb.)	Nominal	
Model No.	Mil (Ga.)	H (in.)	W (in.)	ዊ (in.)	Anchor Bolt Dia. ¹ (in.)	Stud Fasteners ⁷	Stud Member Thickness mil (ga.)	Tension Load	Deflection at ASD Load⁴	Tension Load	Deflection at LRFD Load ⁴	Tension Load (lb.)	Code Ref.
							2-33 (2-20)	7,335	0.12	11,715	0.204	13,720	
S/HD8S	118	11	25/16	1½	7/8	(17) #14 ⁷	2-43 (2-18)	8,750	0.086	13,975	0.146	21,435	
3/1003	(10)	11	2 716	1 72	'/8	(17) #14	2–54 (2–16)	8,855	0.106	14,145	0.162	21,700	
							Steel fixture	10,840	0.053	17,335	0.072	32,525	
							2-33 (2-20)	7,400	0.122	11,815	0.192	13,835	
0/110100	118	101/	05/		7/	(00) //147	2–43 (2–18)	11,120	0.112	17,755	0.124	20,795	
S/HD10S	(10)	13½	25⁄16	1½	7/8	(22) #147	2-54 (2-16)	12,220	0.096	19,520	0.145	29,940	
							Steel fixture	12,375	0.043	19,820	0.061	33,535	
							2-43 (2-18)	12,110	0.096	19,340	0.164	22,645	
S/HD15S	171 (7)	17	25⁄16	1%	1	(30) #147	2–54 (2–16)	13,500	0.11	21,565	0.13	33,075	
							Steel fixture	15,810	0.043	25,320	0.065	42,845	IP1,
							2–33 (2–20)	3,895	0.081	5,620	0.144	8,645	L2, FL
S/HD8B	171	44	05/	11/	7/	(2) ¾" dia.	2–43 (2–18)	5,345	0.098	7,710	0.146	11,865	
2/HD8B	(7)	11	25⁄16	1½	7/8	(Z) %4 UIA.	2–54 (2–16)	8,950	0.082	14,280	0.141	20,310	
							Steel fixture	9,080	0.069	14,545	0.104	22,975	
							2-33 (2-20)	5,840	0.070	8,430	0.124	12,970	
S/HD10B	118	101/	05/	11/	7/	(0) 3/ 11 dia	2–43 (2–18)	8,015	0.087	11,565	0.12	17,795	
S/HD IUB	(10)	13½	25⁄16	1½	7/8	(3) ¾" dia.	2–54 (2–16)	12,090	0.125	19,720	0.23	28,050	
							Steel fixture	15,635	0.102	24,955	0.123	35,495	
							2-43 (2-18)	16,020	0.118	15,425	0.179	22,165	
S/HD15B	171 (7)	17	25⁄16	1 %16	1	(4) ¾" dia.	2–54 (2–16)	16,020	0.090	25,565	0.121	36,360	
	. ,						Steel fixture	18,690	0.104	29,825	0.139	42,425	

These products are available with additional corrosion protection. Additional products on

this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. The Designer shall specify the foundation anchor material type, embedment and configuration. Some of the tabulated holdown tension loads exceed the tension strength of typical ASTM A36 or A307 anchor bolts. 2. Stud design by Specifier. Tabulated loads are based on a minimum stud thickness for fastener connection.

3.1/4" self-drilling screws may be substituted for #14 self-tapping screws.

4. Deflection at ASD or LRFD includes fastener slip, holdown deformation and anchor rod elongation for holdowns installed up to 4" above top of concrete. Holdowns may be installed raised, up to 18" above top of concrete, with no load reduction provided that additional elongation of the anchor rod is accounted for. See bottom of p. 243 for installation detail.

5. The Nominal Tension Load is based on the tested average ultimate (peak) load and is provided for design in accordance with section C5 of AISI S213 that requires a holdown to have a nominal strength to resist the lesser of the amplified seismic load or the maximum force the system can deliver.

6. Not all fastener holes for S/HDS holdowns need to be filled, as additional fastener holes provided. Install fasteners symmetrically.

7. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

PA/HPA Purlin Anchors

PA/HPA purlin anchors offer solutions for CFS to concrete and concrete block connections which satisfy code requirements. The PAs dual embedment line allows installation in concrete or concrete block.

Material: PA - 12 gauge; HPA - 10 gauge

Finish: Galvanized. PAs available in HDG or ZMAX® coating.

Installation:

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

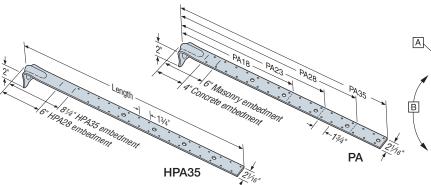
- Use all specified fasteners; some models have extra fastener holes. See General Notes.
- Purlin anchor must hook around rebar.
- · Allowable loads are for a horizontal installation into the side of a concrete or masonry wall.
- Strap may be bent one full cycle. (Bent vertical 90° then bent horizontal.)

Edge Distance — Minimum concrete edge distance is 5". Minimum concrete block left-to-right edge distance is 20".

Concrete Block Wall – The minimum wall specifications are:

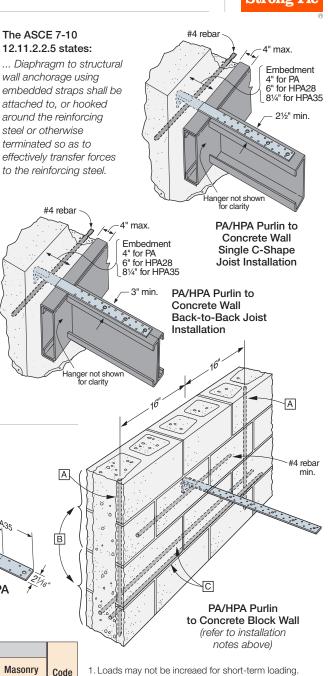
- A One #4 vertical rebar, 32" long, 16" each side of anchor.
- B Two courses of grout filled block above and below the anchor (no cold joints allowed).
- C A horizontal bond beam with two #4 rebars, 40" long, a maximum of two courses above or below the anchor.
- D Minimum masonry compressive strength, f'm = 1,500 psi.

Options: See S/LTT and HTT Tension Ties for alternate retrofit solutions Codes: See p. 11 for Code Reference Key Chart



			Wind	and SDC	A&B — AI	lowable Lo	oad (lb.)			
Model	Strap Length	le		Fasteners ud /Joist 1		Non Cracked	Cracked	Max. Allowable Strap	Masonry Installation	Code Ref.
No.	L (in.)	(in.)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	Tension Load	Tension Load	Tensile Capacity	Tension Load	
PA18	18½	4	(16) #10	(16) #10	(8) #10	2,830	2,360	NA	1,895	
PA23	23¾	4	(22) #10	(16) #10	(8) #10	3,220	2,360	NA	2,815	
PA28	29	4	(22) #10	(16) #10	(8) #10	3,370	2,360	NA	2,815	
PA35	35	4	(22) #10	(16) #10	(8) #10	3,370	2,360	NA	2,815	
HPA28	321⁄2	6	(28) #10	(20) #10	(10) #10	4,845	4,845	NA	—	
HPA35	38½	81⁄4	(32) #10	(22) #10	(12) #10	5,145	5,145	NA	—	
				SDC C-F	— Allowal	ole Load (I	b.)			18
PA18	18½	4	(16) #10	(16) #10	(8) #10	2,830	1,980	3,220	1,895	
PA23	23¾	4	(22) #10	(16) #10	(8) #10	2,830	1,980	3,220	2,815	
PA28	29	4	(22) #10	(16) #10	(8) #10	2,830	1,980	3,935	2,815	
PA35	35	4	(22) #10	(16) #10	(8) #10	2,830	1,980	3,935	2,815	
HPA28	321⁄2	6	(28) #10	(20) #10	(10) #10	4,845	4,090	5,145	—	
HPA35	38½	8¼	(32) #10	(22) #10	(12) #10	5,145	5,145	5,145	—	

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.



- 2. For concrete installs, the minimum compressive strength, f'_c = 3,000 psi.
- 3. Multiply Seismic and Wind ASD load values by 1.4 or 1.6 respectively to obtain LRFD capacities.
- 4. In accordance with 2006 and 2009 IBC Section 1613.1, detached one- and two-family dwellings in Seismic Design Category (SDC) C may use "Wind and SDC A&B" allowable loads
- 5. Minimum center-to-center spacing is 3 times the required embedment $(S_{min} = 3 \times I_e)$ for PA/HPAs acting in tension simultaneously, where I_e = embedment depth. Standard installation is based on minimum 5" end distance.
- 6. Install fasteners symmetrically and with a minimum of 4 of the required fasteners between the embedment line and the first tooling hole. In some cases, not all of the fastener holes will need to be filled.
- 7. Per ASCE7-10, 12.11.2.2.2, for diaphragms in structures assigned to SDC C-F, maximum allowable strap tensile capacity shall be no less than 1.4 times the design load. Not applicable (NA) for Wind and SDC A&B designs.
- 8. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

STHD/LSTHD Strap Tie Holdowns



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The STHD is an embedded strap-tie holdown offering high-load capacity. The STHD incorporates many features that aid correct installation and improve performance. When installed on the forms with the StrapMate® strap holder, the unique design of the STHD delivers enhanced stability before and during the pour to help prevent both parallel and perpendicular movement (relative to the form). This results in accurate positioning of the strap and reduced possibility of spalling.

Features

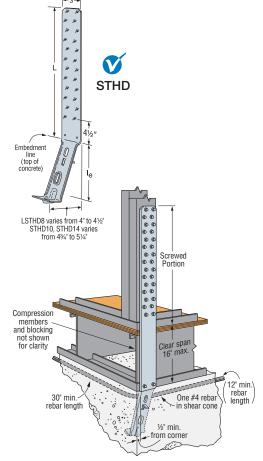
- The fastener pattern allows for fastening to the edges of back-to-back studs
- Strap nail slots are countersunk to provide a lower nail head profile
- The slots below the embedment line enable increased front-to-back concrete bond and help to reduce spalling
- Rim joist models accommodate up to a 17" clear span without any loss of strap fastening
- Material: LSTHD8, LSTHD8RJ 14 gauge, all others 12 gauge

Finish: Galvanized

Installation: • Use all specified fasteners; see General Notes.

- Use table below for both standard concrete and post-tension slab installations.
- Install before concrete pour with a StrapMate® or other holding device.
- Fasten strap from the bottom up.
- Strap may be bent one full cycle (bent horizontal 90° then bent vertical) to aid wall
 placement, but may cause spalling behind the strap. If the spall is 1" or less, measured
 from the embedment line to the bottom of the spall, full loads apply. 1" to 4" spalls for
 LSTHD8 achieve 0.9 times table loads. STHD10 and STHD14 achieve full load for spalls
 less than 4". Any portion of the strap left exposed should be protected against corrosion.
- Other than where noted in the two-pour detail, do not install where: (a) a horizontal cold joint exists within the embedment depth between the slab and foundation wall or footing beneath, unless provisions are made to transfer the load, or the slab is designed to resist the load imposed by the anchor; or (b) slabs are poured over concrete block foundation walls.
- Additional studs attached to the shearwall studs or post may be required by the Designer for wall sheathing fastening.
- For installation in severe corrosion environments, refer to strongtie.com for additional considerations.

Codes: See p. 11 for Code Reference Key Chart



SIMPSON

Strong-

Typical STHD14RJ Rim Joist Application

Allowable Stress Design (ASD) Loads for STHD Strap Style Hold-Down on CFS - 2,500 psi Concrete

			Wine	d and SD	C A&B —	Allowable Tensio	n Load (lb.) — 33 mi	l (20 ga.) S	tuds				
Min.	Mode	el No.	Strap Ler	ngth (L)									Code	
Stem Wall	Standard	Rim Joist	Standard	Rim Joist	le (in.)	Non-Cracked				Cracke	d		Ref.	
(in.)	otandara		(in.)	(in.)	(,	Req'd Screws⁵	Mid Wall	Corner	End Wall	Req'd Screws⁵	Mid Wall	Corner	End Wall	
	LSTHD8	LSTHD8RJ	18%	321/8	8	(20) #10	3,115	2,700	1,690	(16) #10	2,675	2,320	1,455	
6	STHD10	STHD10RJ	24%	381⁄8	10	(24) #10	3,690	3,820	2,050	(22) #10	3,140	3,140	1,705	
	STHD14	STHD14RJ	261/8	39%	14	(30) #10	5,150	5,150	3,200	(30) #10	5,150	5,150	3,200	
	LSTHD8	LSTHD8RJ	18%	321/8	8	(20) #10	3,115	2,700	2,230	(16) #10	2,675	2,320	1,915	
8	STHD10	STHD10RJ	24%	381⁄8	10	(28) #10	4,755	4,120	3,145	(26) #10	4,195	3,500	2,585	
	STHD14	STHD14RJ	261/8	39%	14	(30) #10	5,300	5,300	4,210	(30) #10	5,300	5,300	4,210	
				SDC C-	F — Allov	vable Tension Loa	ad (Ib.) — 3	33 mil (20	ga.) Studs					
Min	Mode	No	Stran Ler	nath (L)										

	Min.	Mode	el No.	Strap Ler	igth (L)										
	Stem Wall	Standard	Rim Joist	Standard	Rim Joist	le (in.)		Non-Crac	ked			Cracke	d		18, FL
	(in.)			(in.)	(in.)		Req'd Screws ⁵	Mid Wall	Corner	End Wall	Req'd Screws ⁵	Mid Wall	Corner	End Wall	
		LSTHD8	LSTHD8RJ	18%	321⁄8	8	(16) #10	2,270	2,090	1,220	(14) #10	2,250	1,950	1,220	
	6	STHD10	STHD10RJ	24%	381⁄8	10	(18) #10	2,750	2,750	1,615	(18) #10	2,640	2,640	1,435	
		STHD14	STHD14RJ	261/8	39%	14	(22) #10	3,695	3,695	2,685	(22) #10	3,695	3,695	2,685	
ł		L STHD8	LSTHD8RJ	18%	321/8	8	(16) #10	2.615	2.125	1.635	(14) #10	2,250	1.820	1,610	
						÷	(- / -	,	, .	,	. ,	,	,		
	8	STHD10	STHD10RJ	24%	381⁄8	10	(20) #10	3,400	2,940	2,295	(20) #10	3,400	2,940	2,175	
		STHD14	STHD14RJ	261/8	39%	14	(24) #10	3,815	3,815	3,500	(24) #10	3,815	3,815	3,500	

For SI: 1 in. = 25.4 mm, 1 lb. = 4.45 N, 1 psi = 6.895 kPa.

1. Deflection at highest allowable loads for install over CFS double studs are as follows: LSTHD8 = 0.065", STHD8 = 0.071", STHD10 = 0.096" and STHD14 = 0.115".

2. Multiply Seismic and Wind ASD load values by 1.4 or 1.6 respectively to obtain LRFD capacities.

3. Per 2009 and 2013 IBC Section 1613, detached one- and two-family

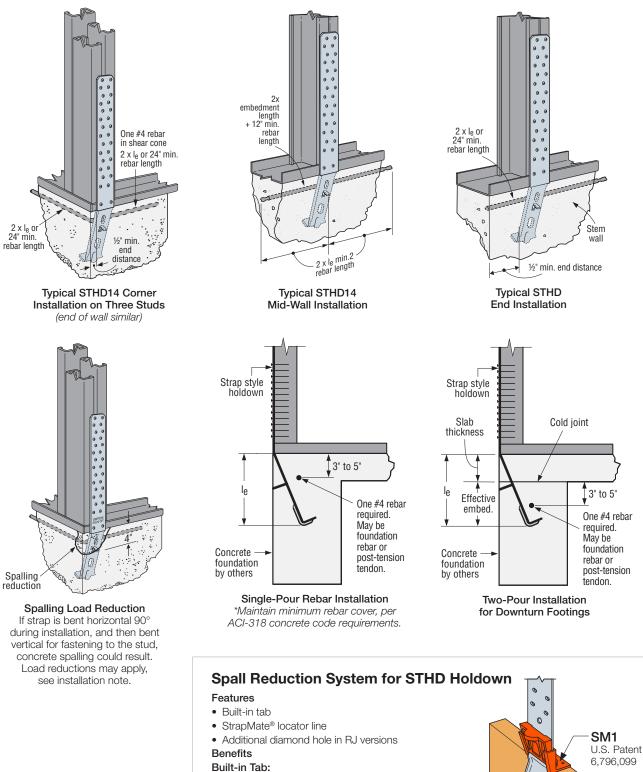
dwellings in Seismic Design Category (SDC) C may use "Wind and SDC A&B" allowable loads.

4. Minimum center-to-center spacing is three times the required embedment $(S_{min}=3 x I_{e})$ for STHD's acting in tension simultaneously. Mid wall install is based on 1.5xle end distance.

5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

Connectors for Cold-Formed Steel Construction

STHD/LSTHD Strap Tie Holdowns



- Built-In Tab:
- Reduces spalling and costly retrofits.
- No additional labor to install.
- Holds STHD away from form board.

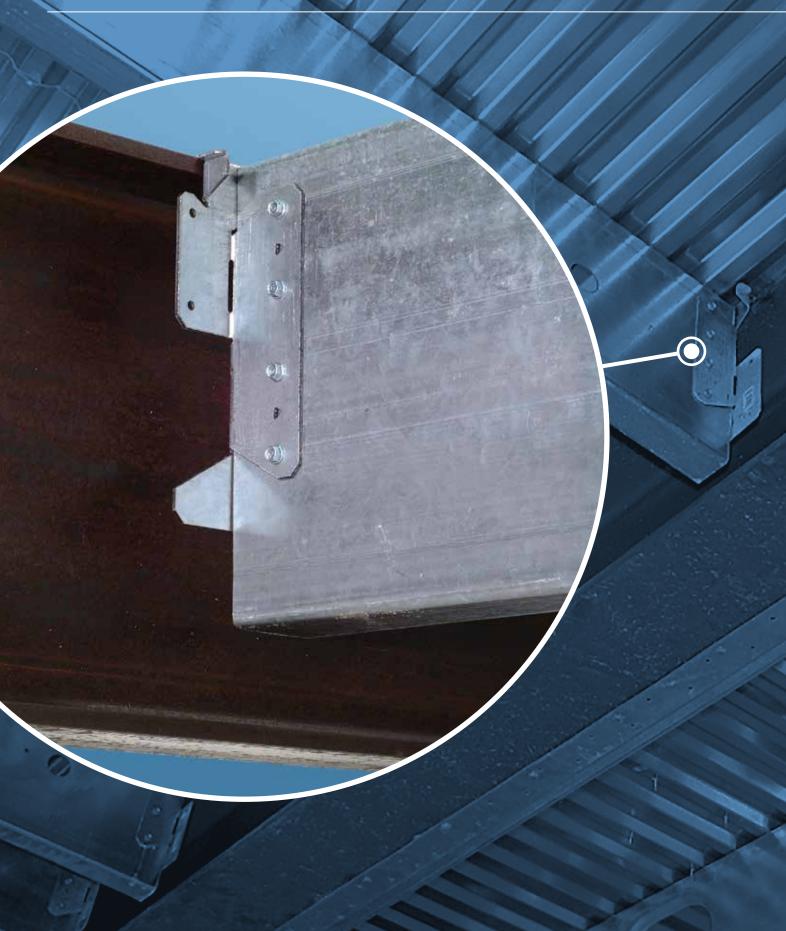
StrapMate Locator Line:

• Easy inspection to ensure proper location.

• Allows adjustment without removing STHD. Additional Diamond Hole:

• One more fastener to help prevent the STHD RJ models from bowing out at the rim joist section.

Joist Framing Connectors



SJC Steel-Joist Connectors

SJC Connectors: Steel-to-Steel

					Fasteners ⁶		Allov	vable F ₄ Load	(lb.) ³	
Model No.	Connector Material Thickness	L (in.)	Framing Member Depth⁵	Pattern ²	Carried	Carrying		Member kness	Maximum Connector	Code Ref.
	mil (ga.)	(,	(in.)	Falleni	Member	Member	54 mil (16 ga.)	68 mil (14 ga.)	Load ⁴	
				Min.	(4) #10	(4) #10	980	980		
SJC8.25	68 (14)	81⁄4	10	Max.	(9) #10	(7) #10	1,005	1,490	2,930	
				Inner	(5) #10	(4) #10	1,345	2,005		
				Min.	(4) #10	(4) #10	1,005	1,710	2,930	
MSJC8.25	97 (12)	81⁄4	10	Max.	(9) #10	(7) #10	1,135	1,765		
				Inner	(5) #10	(4) #10	1,535	2,220		IP2
				Min.	(6) #10	(4) #10	1,170	1,625		IFZ
SJC10.25	68 (14)	10¼	12	Max.	(11) #10	(7) #10	1,265	1,625	3,935	
				Inner	(7) #10	(5) #10	1,620	2,170		
				Min.	(6) #10	(4) #10	1,200	2,045		
MSJC10.25	97 (12)	10¼	12	Max.	(11) #10	(7) #10	1,265	2,045	3,935	
				Inner	(7) #10	(5) #10	1,730	2,635		

1. See p. 82 for product information.

2. Min. fastener quantity and load values - fill all round holes; Max. fastener quantity and load values - fill all

round and triangular holes; Inner fastener quantity and load values — see illustrations for fastener placement.

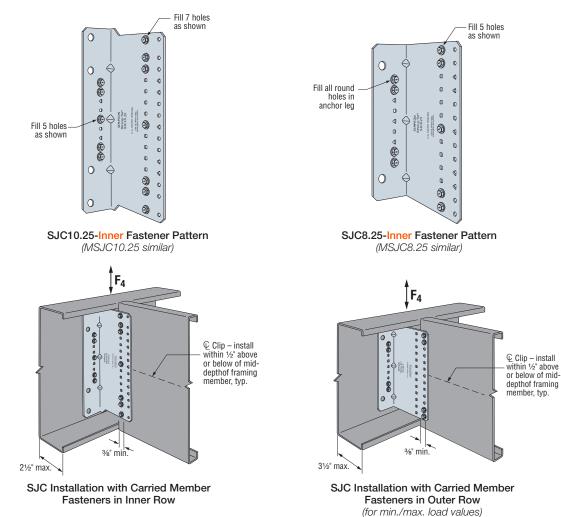
3. Allowable loads are based on bracing of the members located within 12" of the connection.

4. Maximum allowable load for connector that may not be exceeded when designing custom installations.

Designer is responsible for member and fastener design.

5. For 6" and 8" joists, SSC connectors are recommended.

6. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



S/JCT and S/HJCT Steel-Joist Connectors



This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

The S/JCT and S/HJCT are unique, skewable steel-joist framing connectors that combine strength, versatility and low installed cost. The connectors can be used with CFS headers, wood headers, steel I-beams (with welds or PAF fasteners) and masonry walls. Installed cost is minimized since these products are shear rather than bearing connectors, eliminating the need for web stiffeners. The connectors also feature

horizontal tabs that facilitate top flange alignment and joist support during screw installation.

Material: S/JCT - 68 mil (14 ga.); S/HJCT - 97 mil (12 ga.)

Finish: Galvanized

Features:

- · Uni-directional: Joist can be attached from left or right
- One size fits joists 8" through 14" deep
- Optional holes for additional load capacity
- · Simplicity of design
- Quick and easy installation
- Field skewable up to 45° left or right

Installation:

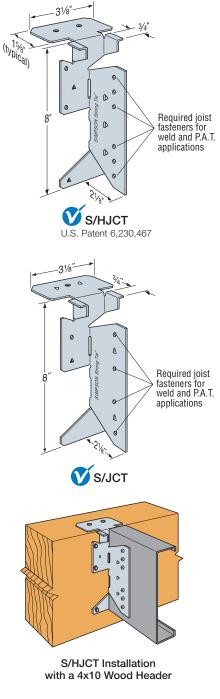
- Attach hanger with specified fasteners. Use round holes for minimum load, use round and triangle holes for maximum load.
- May be used for weld-on applications. The minimum required weld to the top flange is 1/8" x 21/2" fillet weld to each side of top flange. Consult the code for special considerations when welding galvanized steel.
- May be installed using PDPAT-62KP (0.157" x 5%") powder-actuated fasteners. Steel headers with thicknesses between $\frac{1}{4}$ and $\frac{3}{4}$ having a minimum $F_V = 36$ ksi. A Red (level 5) or Purple (level 6) powder load may be required to achieve specified penetration (p). See illustration on p. 253.

Codes: See p. 11 for Code Reference Key Chart

Ordering Information: The S/JCT is sold in cartons of 50. The S/HJCT is sold in kits as the S/HJCT-KT and contains five (5) connectors and (95) #14 screws.

		Fasteners ⁷		Allowab	le Load ¹	Code
Model No.	Тор	Face	Joist	Uplift	Down	Ref.
Att	ached to CFS Heade	er: 54 mil (16 ga.) ³ —	– Straight	Hanger		
S/JCT (min.)	(1) #10	(2) #10	(4) #10	940	1,195	1
S/JCT (max.)	(1) #10	(4) #10	(6) #10	1,435	2,105	1
S/HJCT (min.)	(2) #10	(4) #14	(6) #14	1,510	2,920	1
S/HJCT (max.)	(2) #10	(8) #14	(9) #14	1,670	3,855	
Att	ached to CFS Heade	er: 54 mil (16 ga.) ³ –	- Skewed	Hanger		I IP1
S/JCT (min.)	(1) #10	(2) #10	(4) #10	940	1,135	L2
S/JCT (max.)	(1) #10	(4) #10	(6) #10	940	1,185	FL
S/HJCT (min.)	(2) #10	(4) #10	(6) #14	1,510	2,305	1
Att	tached to Steel Hea	der⁴ — Straight and	l Skewed	Hanger		
S/JCT (min.)		<i>au</i>	(4) #10	145	940]
S/HJCT (min.)	1/8" x 21/2" to each side		(4) #14	195	1,450	
S/HJCT (min.) Skew		or top hange	(4) #14	195	1,235	1
S/JCT (min.)	(2) 0.15	7" x %"	(4) #10	145	750	
S/HJCT (min.)	powder-actua	ated fastener ⁸	(4) #14	195	1,185	
	Attached to Masonr	y — Straight and S	kewed Ha	nger		170
S/HJCT (min.)	(0) 1/." x 01/." Titon	(4) 1/" x 01/" Titon	(6) #14	710	1,785	
S/HJCT (min.) Skew	(<i>Z</i>) 74 X Z 74 TILET	(4) 1⁄4" x 21⁄4" Titen	(6) #14	710	1,410	

Madal Na		Fasteners ⁷		Allowabl	e Load ^{1,2}	Code
Model No.	Тор	Face	Joist	Uplift (160)	Down (100)	Ref.
ł	Attached to 4x DF/S	P Wood Header — S	Straight Ha	anger		
S/JCT (min.)	(1) 10d	(2) 10d	(4) #10	555	945	
S/JCT (max.)	(1) 10d	(4) 10d	(6) #10	945	1,465	
S/HJCT (min.)	(2) 10d	(4) 1⁄4"x3" SDS	(6) #14	1,210	2,625	IP1
S/HJCT (max.)	(2) 10d	(8) 1⁄4"x3" SDS	(9) #14	1,475	2,980	L2
I	Attached to 4x DF/S	P Wood Header — S	Skewed Ha	anger		FL
S/JCT (min.)	(1) 10d	(2) 10d	(4) #10	390	845	
S/JCT (max.)	(1) 10d	(4) 10d	(6) #10	775	1,300	
S/HJCT (min.)	(2) 10d	(4) ¼" x 3" SDS	(6) #14	1,210	1,935	



SIMPSON

Strong-

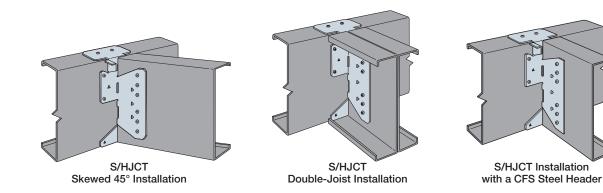
- 1. Allowable loads are based on on a minimum of single 54 mil (16 ga.) CFS joist member. CFS joist shall be laterally braced per Designer specification.
- 2. Allowable loads for wood header are based on 4x DF/SP minimum, for SPF/HF wood species use an adjustment factor of 0.72.
- 3. CFS header must be braced to prevent web buckling per Designer specification and header must have full bearing of 1%" flange-depth.
- 4. Backing in the steel beam cavity is not required behind the hanger for load listed.
- 5. Screws shall be installed using joist hanger holes screwing through the hanger into the joist.
- 6. CFS joists with up to a 0.50" gap (short cut), use an adjustment factor of 0.87 and joists with a 0.50" to 0.90" gap (short cut), use an adjustment factor of 0.75.
- 7. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.
- 8. See p. 253 for more information.

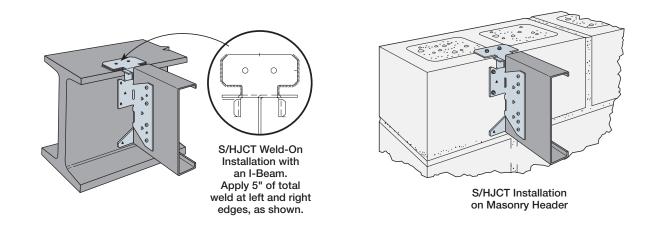
Joist Framing Connectors

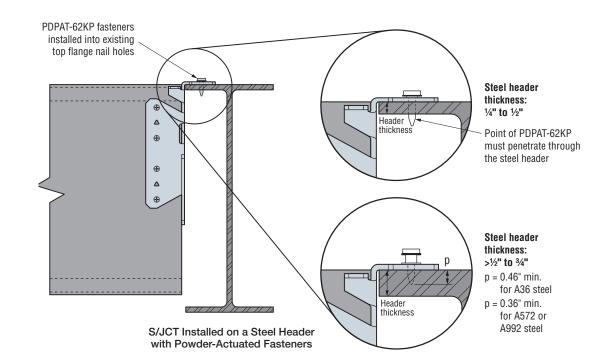
Connectors for Cold-Formed Steel Construction

S/JCT and S/HJCT Steel-Joist Connectors









S/LBV, S/B and S/BA Hangers

The S/BA is a cost-effective alternative to heavier, special-order hangers. It is value engineered and tested to achieve higher allowable loads and increased performance. It may be fastened with screws or powder-actuated fasteners or welded to the header, providing more design options and greater versatility.

S/LBV and S/B top-flange hangers are manufactured with precision forming and quality control, providing dimensional accuracy and helping to ensure proper bearing area and connection.

Material: S/LBV, S/BA - 68 mil (14 ga.); S/B - 97 mil (12 ga.)

Finish: Galvanized

Installation:

- S/LBV, S/B, and S/BA may be used for weld-on applications; a minimum of 1/8" x 2" fillet weld on each top flange is required. Distribute the weld equally on both top flanges. Consult the code for special consideration when welding galvanized steel. Uplift loads do not apply.
- S/BA may be installed using PDPAT-62KP (0.157" x 5%") powder-actuated fasteners. Steel headers with thicknesses between $\frac{1}{4}$ " and $\frac{3}{4}$ " having minimum F_V – 36 ksi. A Red (level 5) or Purple (level 6) powder load may be required to achieve specified penetration (p).

Options: Skew only:

• S/LBV and S/B series (max. width = 51/2") can be skewed to a maximum of 45 degrees

3

S/B Series

Model No.

S/B1.56X

S/B1.68X

S/B1.81X

S/B2.06X

S/B2.37X

S/B2.56X

S/B2.68X

S/B3.12X

S/B3.56X

S/B3 62X

S/B4.06X

S/B4.12X

S/B4.28X

S/B4.75X

S/B5.25X

S/B5.5X S/B5.56X

S/B6.06X

S/B7.12X

S/B7.25X

S/B7.5X

Fasteners⁶

Cold-Formed Steel Header — Straight Hanger

Cold-Formed Steel Header — Skewed Hanger

Face

(2) #10

(4) #14

(2) #10

(4) #14

uated Fastener or Welded to Steel Header — Straight Hanger

Тор

(4) #10

(8) #10

(6) #10

(4) #10

(8) #10

(4) 1/8" x 2"

fillet weld to each

side of top flange

(6) 0.157" x 5/8" PAT

W

(in.)

1%6

1 11/16 1 ¹³/16

21/16

23⁄8

2%16

211/16

31/8

3%16

3%

41/16

41⁄8

41⁄4

4¾

51/4 51⁄2

5%6

61/16

71⁄8

71⁄4

71/2

Joist

(3) #10

(3) #14

(1) #10

(3) #10

(3) #14

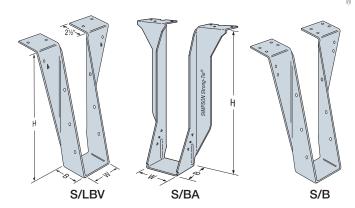
(3) #10

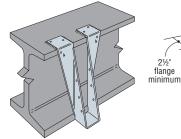
(3) #14

(1) #10

(1) #10

Codes: See p. 11 for Code Reference Key Chart





S/LBV, S/B, and S/BA Are Acceptable for Weld-on Applications (S/LBV shown)

Code

Ref.

IP1

FL, Ľ2

IP1,

FL, L2

IP1.

FL, L2

Allowable ASD Load⁵

Down

3,150

5,970

3,475

2.220

4,195

2.965

5,755

2,920

26854

Uplift

1,010

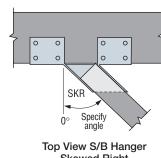
1,855

1.010

1,855

S/LBV Installation with a CFS Steel Header

- 1. Designer shall ensure that the joist member adequately transfers load to hanger.
- 2. Cold-formed steel / steel-beam header must be braced to prevent buckling per Designer specification.
- 3. Powder-actuated fasteners may be installed in up to 3/8" steel headers having a minimum F_v = 36,000 psi.
- 4. Load is based on the Simpson Strong-Tie® PDPAT-62KP powder-actuated fastener and a minimum Red (level 5) powder load.
- 5. Tabulated loads are based on testing with full bearing of 21/2" flange-depth minimum with 68 mil (14 ga.) CFS for S/LBV and S/BA hanger and 97 mil (12 ga.) CFS for S/B hanger.
- 6. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



S/BA

S/LBV Series

Model No.

S/LBV1.56X

S/LBV1.68X

S/I BV1 81X

S/LBV2.06X

S/LBV2.37X

S/LBV2.56X

S/LBV2.68X

S/LBV3.12X

S/LBV3.56X

S/LBV3.62X

S/LBV4.06X

S/LBV4.12X

S/LBV4.28X

S/LBV4.75X

S/LBV5.5X

Model	Di	mensions (in	I.)
Model	W	Н	В
S/LBV		6 to 20	21⁄4
S/B	See table	6 to 30	3
S/BA	lubic	See table	3
S/LBV	See	6 to 20	21⁄4
S/B	table	6 to 30	3
		Powde	r-Actı
S/LBV		6 to 20	21⁄4
S/B	See	6 to 30	3
S/BA	table	See table	3
0 /0 4	1	See lable	0

S/BA Series Model No.	S/B Series Model No.	S/LBV Series Model No.	W (in.)	H (in.)
S/BA2.12/8		—		8
S/BA2.12/10		—	21/8	10
S/BA2.12/12		—	2 78	12
S/BA2.12/14	—	—		14
—	S/B2.56/8	S/LBV2.56/8		8
	S/B2.56/10	S/LBV2.56/10	2%16	10
	S/B2.56/12	S/LBV2.56/12		12
_	S/B3.12/8	S/LBV3.12/8		8
_	S/B3.12/10	S/LBV3.12/10	31⁄8	10
—	S/B3.12/12	S/LBV2.56/12		12
S/BA4.18/8				8
S/BA4.18/10			43/16	10
S/BA4.18/12		_	4 %16	12
S/BA4.18/14				14

S/DHUTF Drywall Hangers

The S/DHUTF top-mount hanger is designed to carry joist loads to a CFS stud wall through two layers of 5%" gypsum board (drywall). This hanger installs after the drywall is in place and comes in sizes that accommodate most typical joists used in multi-family and commercial construction.

Material: 112 mil (12 ga.)

Finish: Galvanized (G90)

Installation:

- Use all specified fasteners; see General Notes
- · Hanger to be framed in-line with vertical wall stud
- · Drywall is installed first
- Wall top track must be restrained to counteract load eccentricity from hanger

Codes: See p. 11 for Code Reference Key Chart

Model	Dimensions (in.)				
No.	w	н			
S/DHU1.68/8TF		8			
S/DHU1.68/10TF	1 11/16	10			
S/DHU1.68/12TF		12			
S/DHU2.1/8TF		8			
S/DHU2.1/10TF	21⁄8	10			
S/DHU2.1/12TF		12			
S/DHU2.56/8TF		8			
S/DHU2.56/10TF	2%16	10			
S/DHU2.56/12TF		12			

S/DHUTF **Typical Installation**

Model		Fasteners ⁶		Allowable	Code	
Model	Тор	Face	Joist	Uplift	Down	Ref.
S/DHUTF	(6) #10	(8) #14 x 2"	(3) #10	1,230	1,700	170

1. Designer shall ensure that the joist member adequately transfers load to the hanger. 2. Tabulated loads assume (2x) 5%" Type X drywall attached per IBC.

3. Wall studs designed per Designer specifications. At a minimum, the assembly must consist of 600T350-68, Gr. 50 ksi top track and 600S162-43, Gr. 33 ksi wall studs spaced at a maximum of 24" o.c.

- 4. Tabulated loads are based on testing with full bearing of 315/16" hanger top flange. The minimum joist gauge is 54 mil (16 ga.).
- 5. S/DHUTF hanger can be installed ¾" max. from the center of the vertical stud per the in-line framing specifications of the AISI General Provisions without load adjustment.
- 6. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

³¹⁄2" min. (see footnote #3)

12" (min.) long

stud piece with

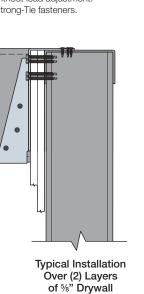
per Designer

specifications.

screws or welds to full height stud.

Stud and connection





SIMPSON

Strong-I

Н

S/DHUTF U.S. Patent 9,394,680

2

3^{15/16}

Note: This splice

detail is for uplift

If the top track is used for drag,

additional detailing

will be required by the Designer.

and download only.

W/WP Hangers

This series of purlin hangers offer the greatest design flexibility and versatility.

Material: Stirrup - 97 mil (12 ga.)

Finish: Simpson Strong-Tie® gray paint. Some models available hot-dip galvanized; specify HDG; see Corrosion Information on pp. 18–21.

Installation: Hangers may be welded to steel headers with 1%" for W and 3/16" for WP by 1 1½"-fillet welds located at each end of the top flange

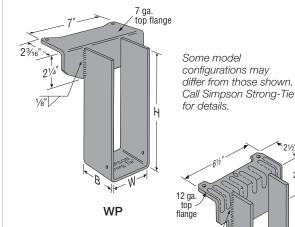
Options: • W and H dimensions are modifiable

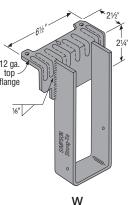
Sloped and/or Skewed Seat

- W/WP series may be skewed to a maximum of 84° and/or sloped to a maximum of 45°.
- For slope only, skew only, or slope and skew combinations, the allowable load is 100% of the table load.
- Specify the slope up or down in degrees from the horizontal plane and/or the skew right or left in degrees from the perpendicular vertical plane. Specify whether low side, high side or center of joist will be flush with the top of the header.

Codes: See p	. 11 for Code	Reference Key Chart
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Model	Dimensions (in.)			Faste	eners⁴	Allowable Down Load	Code	1
No.	W	Н	В	Header	Joist	(lb.)	Ref.	2
W	1½-4	4° - 30	21⁄2-5	Weld	(1) #10	2,335	170	3
WP	1%16 - 71/2	4°-30	21⁄2-5	Weld	(1) #10	3,650	170	





- 1. For hanger heights exceeding the joist height, the allowable load is 0.50 of the table value.
- The Designer shall ensure that the joist member adequately transfers load to the hanger.
- 3. Not all combinations of W, H, and B dimensions are available. Contact Simpson Strong-Tie.
- 4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

HUC Hangers

Material: 68 mil (14 ga.)

Finish: Galvanized

Installation:

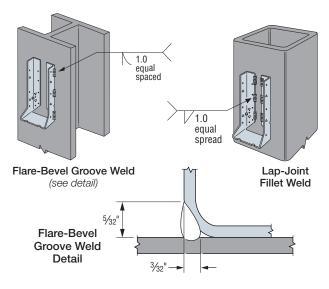
- HUC series hangers may be welded to supporting structural steel members
- Use 1" weld segments equally spaced top and bottom, with half the segments on each side of hanger
- Welds may be either lap joint (on outside edge of flanges) or flare-bevel groove (on flange bend line)

Codes: See p. 11 for Code Reference Key Chart

		Conne	ection Type	Maximum	
	Model Series	Joist	Structural Steel	Allowable Down Load	Code Ref.
		Fastener ^{5,6}	Weld	(lb.)	
	шис	#10	(4) 1" segments	3,280	170
	HUC	#10	(6) 1" segments	4,855	170

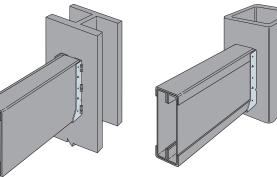
1. Loads assume E-70S-6 (60 ksi) filler rod.

- 2. Welds must conform to the current A.W.S. D1.3 structural welding code for sheet steel and must be performed by a certified welder.
- 3. Designer shall ensure that the joist member adequately transfers load to hanger.
- Hanger-to-joist connection shall be made using screws on the side of the hanger where it meets the web of the joist.
- 5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.
- 6. The number of screws is per Designer specifications.



Installation for CFS Built-Up Beam

The Designer is responsible for design of beam member.



Connectors for Cold-Formed Steel Construction

L, LS and S/LS Utility Clips and Skewable Angles

L, LS, and S/LS angles are load rated, providing the correct thickness and number of fasteners for the specifier compared with field fabricated clip angles. These angles also have well-defined fastener locations, and testing ensures that the tabulated load values account for connection eccentricities. The connectors are general utility reinforcing angles with multiple uses. S/LS and LS connectors are skewable and can be used to attach members intersecting at angles.

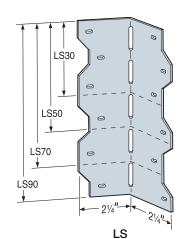
Material: L - 54 mil (16 ga.); S/LS and LS - 43 mil (18 ga.)

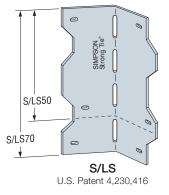
Finish: Galvanized (G90)

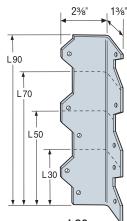
Installation:

- Use all specified fasteners
- S/LS and LS Field-skewable; bend one time only
- CFS framing must be constrained against rotation when using a single S/LS or LS per connection

Codes: See p. 11 for Code Reference Key Chart







L90	

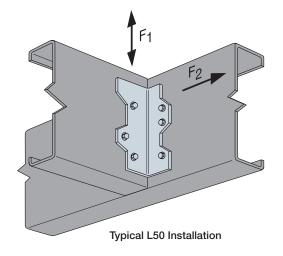
					Allowable	Load (lb.)			
Model No.	Length (in.)	Fasteners ²	33 mil	(20 ga.)	43 mil	(18 ga.)	54 mil	(16 ga.)	Code Ref.
	()		F1	F ₂	F1	F ₂	F1	F ₂	
L30	3	(4) #10	200	60	315	85	610	—	
L50	5	(6) #10	475	—	675	90	750	110	
L70	7	(8) #10	705	—	760	110	1,100	110	
L90	9	(10) #10	795	—	945	110	1,740	110	170
LS30	3%	(6) #10	200	—	370	—	500	—	170
S/LS50	41⁄8	(4) #10	200	—	370	_	500	—	
S/LS70	6%	(6) #10	465	—	575	—	715		
LS90	71⁄8	(12) #10	465	—	895	—	915	_	

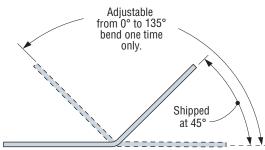
These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

this page may also be available with this option. Check with Simpson Strong-Tie for details.

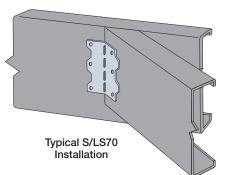
1. Loads are for one part only.

2. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.





S/LS Top View



ICFVL Ledger Connector System

The ICFVL ledger connector system is engineered to solve the challenges of mounting CFS ledgers to insulated concrete form (ICF) walls. The ICFVL is designed to provide both vertical and lateral, in-plane performance. There are many benefits over traditional anchor bolting, including better on-center spacing in most cases, faster installation and no protrusions. The embedded legs of the ICFVL are embossed for additional stiffness and the hole allows for concrete to flow through and around the connector. The exposed flange on the face of the ICF provides a structural surface for mounting a CFS ledger.

Material: ICFVL - 68 mil (14 ga.)

Finish: Galvanized (G90)

Installation:

- ICFVL in ICF
- Snap a chalk line for the bottom of the ledger
- Mark required on-center spacing
- Use ICFVL to mark kerfs locations
- Cut kerfs as marked
- Insert ICFVL flush to the face of the ICF
- Pour concrete

CFS Ledger Attachment

- Position the ledger level to the chalk line and against the ICFVL
- Attach with four #14 x 3/4", #3 drill point screws (not provided)
- All screws should be located at least 1/2" from the edge of the ICFVL
- Space screws evenly

Codes: See p. 11 for Code Reference Key Chart

Model No.	Fasteners	Allowable A 54 mil 68 mil	Code Ref.	
		Download	Lateral F ₁	
ICFVL	(4) #14 x ¾"3	1,660	1,525	170

1. Fasteners for CFS ledger are not provided.

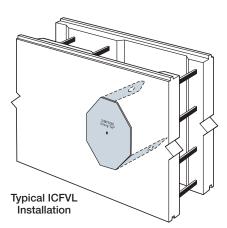
- 2. Loads apply to ICF foam thicknesses of 23/4" or less.
- Contact Simpson Strong-Tie for allowable loads on thicker walls.
- 3. Alternately, 1/4" x 3/4" fasteners may be used.
- 4. Concrete f_{C}^{\prime} = 2,500 psi minimum.
- 5. When combining download and lateral loads, the Designer shall use the following interaction equation: Design Download/Allowable Download + Design Lateral Load/ Allowable Lateral Load \leq 1.

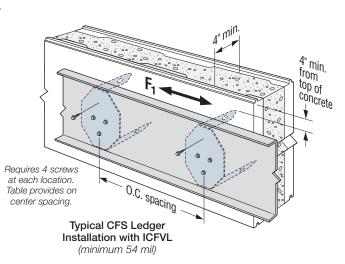
These tables address vertical load applications only

Ledger		ICFVL Spacing to Replace Anchor Bolts on a CFS Ledger (in.) ^{1,2}								
Material	Connector Type	1/2"-Diameter Anchors at				5%"-Diameter Anchors at				
Thickness mil (ga.)		12" o.c.	24" o.c.	36" o.c.	48" o.c.	12" o.c.	24" o.c.	36" o.c.	48" o.c.	
68 (14)	ICFVL	11	22	33	44	9	18	27	36	
54 (16)	ICFVL	15	30	45	48	12	24	36	48	

1. The Designer may specify different spacing based on the load requirements.

2. See flier F-ICFVL for additional connection details.





SIMPSON Strong Tie

6

Reduce the chance of mis-installations using the wrong size screws; specify Simpson Strong-Tie® #14 Self-Drilling E Metal screw (Model No. E1B1414) with the ICFVL Ledger Connector System. See p. 142 for details.

ICFVL

Patent pending

15/8"

8



Available in 100 ct. and 2,500 ct. cartons.

Warning: Industry studies show that hardened fasteners can experience performance problems in wet environments. Accordingly, use this product in dry environments only.

TB and LTB Bridging

TB and LTB bridging connectors are a cost-effective solution for bracing between floor joists when compared with field fabricated blocking and clip angles.

Material: LTB - 27 mil (22 ga.); TB - 33 mil (20 ga.)

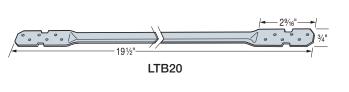
Finish: Galvanized (G90)

Installation:

• Use (2) #10 screws at each end

Codes: See p. 11 for Code Reference Key Chart

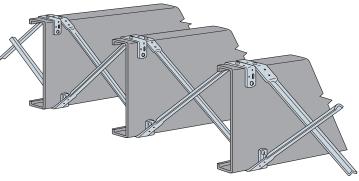
Web Height	Spacing	Т	В	LTB	Code
(in.)	(in.)	Model No.	L (in.)	Model No.	Ref.
6		TB20	20	LTB20	
8		TB20	20	LTB20	
10	12	TB20	20	—	
12		TB27	27	—	
14		TB27	27	—	
6		TB27	27	—	
8		TB27	27	—	180
10	16	TB27	27	—	
12		TB27	27	—	
14		TB27	27	—	
10		TB36	36	—	
12	24	TB36	36	—	
14		TB36	36	_	



SIMPSON

Strong





Typical TB Installation

CS Coiled Strap

CS coiled utility straps are an ideal solution when it is desired to brace floor joist flanges with flat strap. These products are packaged in lightweight cartons (about 40 lb.) and can be cut to length on the jobsite.

Material: See table

Finish: Galvanized (G90)

Installation:

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

- Use all specified fasteners
- Refer to the applicable code for minimum edge and end distance

Codes: See p. 11 for Code Reference Key Chart

	Total Connector			Faste	ners ⁴ (At Blo	cking)	Allowable	
Model No.	Length (fft) Material Thickness	Width (in.)	Framing) Thickness I	mil (ga.)	Tension Load	Code Ref.	
	(ft.)	mil (ga.)		33 (20 ga.)	43 (18 ga.)	54 (16 ga.)	(lb.)	
CS16	150	54 (16)	1¼	(9) #10	(6) #10	(4) #10	1,550	
CS18	200	43 (18)	1¼	(7) #10	(5) #10	(3) #10	1,235	IP1,
CS20	250	33 (20)	1¼	(6) #10	(4) #10	(3) #10	945	L2, FL
CS22	300	27 (22)	1¼	(5) #10	(3) #10	(3) #10	775	

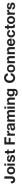
1. In order to achieve the tabulated loads in the strap, attach each strap to the blocking with the tabulated number of screws.

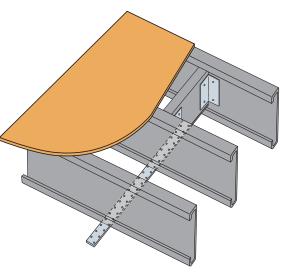
2. Strap length at blocking to achieve tabulated load = number of tabulated screws + 1".

3. Calculate the strap value for a reduced number of screws to the blocking as follows:

Allowable Load = $\frac{\text{No. of Screws Used}}{\text{No. of Screws in Table}}$ - x Table Load

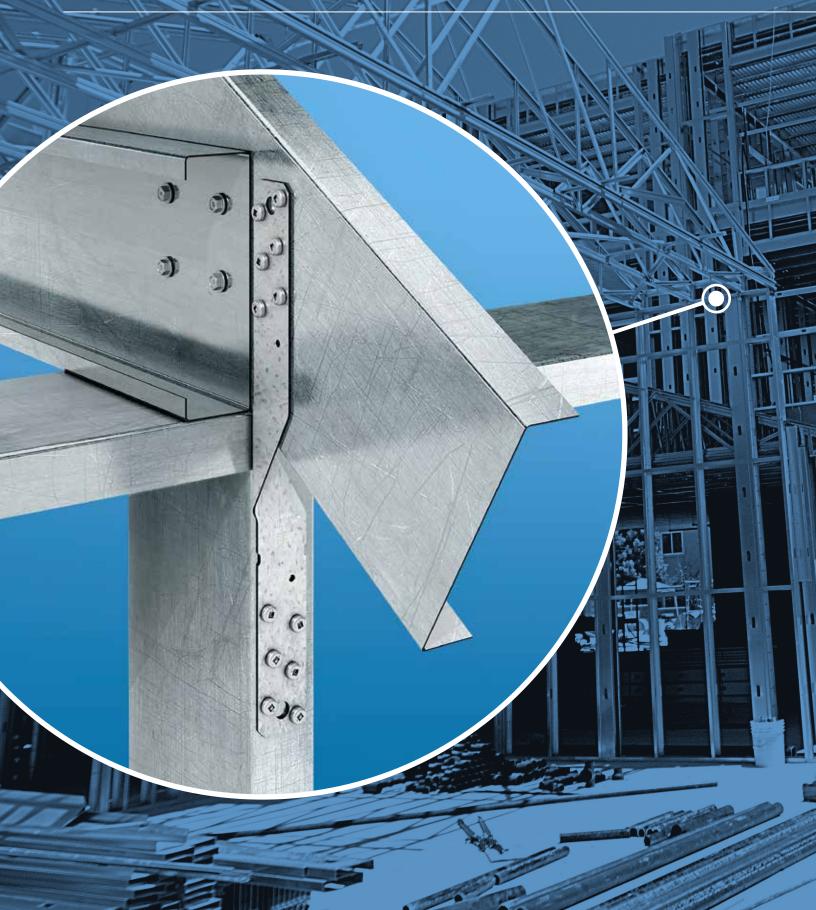
4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.





Typical CS Installation for Block and Strap Joist Bridging

Roof, Truss and Rafter Connectors, Ties and Straps



SSP/DSP/TSP Stud Plate Ties

The SSP and DSP single- and double-stud plate ties connect single and double studs to top and bottom track. Each can be used for either top- or bottom-track applications.

Material: SSP/DSP - 43 mil (18 ga.); TSP - 54 mil (16 ga.)

Finish: Galvanized. Some products available in ZMAX[®]; see Corrosion Information, pp. 18–21.

- Installation: Use all specified fasteners; see General Notes
- DSP/SSP top track installation-fill all round and triangle holes

Codes: See p. 11 for Code Reference Key Chart

	Model		Fa	steners ⁴		Allowable (II	Code	
	No.	Studs	Top 1	rack	rack Bottom Track		43 mil	Ref.
		CFS	Wood	CFS	CFS	(20 ga.)	(18 ga.)	
			—	—	(2) #10	355	625	
	SSP	(4) #10	—	(2) #10	—	340	600	
	- 30F	(4) #10	(2) #10 ³	(1) #10	—	4,051	7,151	
			(2) 10d	(1) #10	—	4,801	8,401	
		(0) #10			(4) #10	430	695	
	DSP		—	(4) #10	—	475	775	IP1,
	DOP	(8) #10	(4) #10 ³	(2) #10	—	5,851	9,551	L2, FL
			(4) 10d	(2) #10	—	7,301	12,001	
		(6) #10	—	—	(3) #10	345	645	
	тер	(0) #10	—	(3) #10	—	370	700	
	ISP	(9) #10	(3) #10 ³	(3) #10	—	3,601	6,851	
			(3) 10d	(3) #10		4,801	9,051	

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

- 1. For wood plates, noted values only apply to DF/SP members where wood top plates are used. For SPF values, multiply by 0.86.
- 2. For wood plates, when cross-grain tension cannot be avoided,
- mechanical reinforcement to resist such forces should be considered.
- 3. Screws installed into wood plates with a minimum #10 x $3^{"}$ self-drilling screw.
- 4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

SP Stud Plate Ties

The SP stud plate tie is a plate-to-stud connection providing uplift resistance.

Material: See table.

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

Finish: Galvanized. Available with ZMAX[®] coating; see Corrosion Information, pp. 18–21.

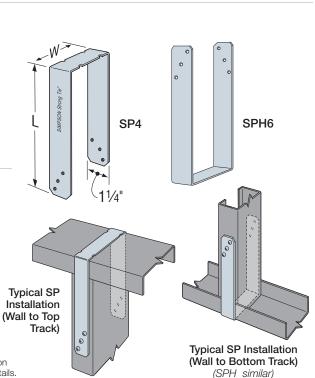
Installation: • Use all specified fasteners

Codes: See p. 11 for Code Reference Key Chart

Madal	Connector	Dimensi	ons (in.)	Ohud	Allowable	0-1-
Model No.	Material Thickness mil (ga.)	w	L	Stud Fasteners ¹	Uplift Load 33 mil (20 ga.) (lb.)	Code Ref.
SP4	33 (20)	3%16	71⁄4	(6) #10	825	IP1,
SP6	33 (20)	5%16	73⁄4	(6) #10	825	L2, FL
SP8	43 (18)	75⁄16	85⁄16	(6) #10	930	
SPH4	43 (18)	3%16	8¾	(12) #10	1,490	
SPH4R	43 (18)	4 1⁄16	81⁄4	(12) #10	1,490	170
SPH6	43 (18)	5%16	91⁄4	(12) #10	1,490	
SPH6R	43 (18)	6 1⁄16	8¾	(12) #10	1,490	
SPH8	43 (18)	75⁄16	83⁄8	(12) #10	1,490	

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

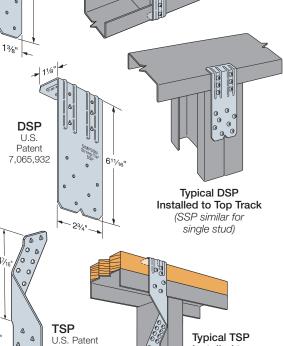


Installed to

Top Plate

Top Track with

Typical SSP Installed to Bottom Track (DSP similar for double stud)



- 1^{1/8}

SIMPSU trong-th

77/8"

15%

D618,085

6¹1/₁₆

SSP

U.S.

Patent

7,065,932

S/H and H Seismic and Hurricane Ties

SIMPSON Strong-

Designed to provide seismic and wind ties for trusses or joists, this versatile line may be used for general tie purposes, strongback attachments, and as all-purpose ties where one member crosses another.

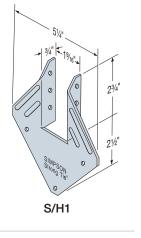
Material: See table

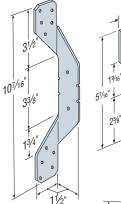
Finish: Galvanized. Available with ZMAX® coating; see Corrosion Information, pp. 18-21.

Installation: • Use all specified fasteners

- The S/H1 can be installed with flanges facing outwards (reverse of illustration 1) when installed inside a wall for truss applications
- Hurricane ties do not replace solid blocking
- S/H2.5, S/H3 and H6 ties are only shipped in equal quantities of rights and lefts

Codes: See p. 11 for Code Reference Key Chart



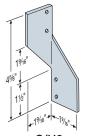


H₂A

83/8

H6

193/16



S/H3

115/8"

H10S

S/H2.5

H7Z

Model	Connector Material		Fasteners ⁵			Allowable Load (lb.) 33 mil (20 ga.)			
No.	Thickness	To Rafters	То Тор	To Stud	Uplift	Lateral		Ref.	
	mil (ga.)	/Truss	Track	10 0100	opint	F1	F ₂		
S/H1	43 (18)	(3) #10	(2) #10	(1) #10	305	100	115		
H2A	43 (18)	(5) #10	(1) #10	(5) #10	450	90	100		
S/H2.5	43 (18)	(4) #10	—	(4) #10	390	90	125		
S/H3	43 (18)	(2) #10	(2) #10	—	375	90	125	170	
H6	54 (16)	—	(8) #10	(8) #10	950		_		
H7Z	54 (16)	(4) #10	(2) #10	(8) #10	985	_	_		
H10S ⁴	43 (18)	(8) #10		(8) #10	930 ³	—	_		

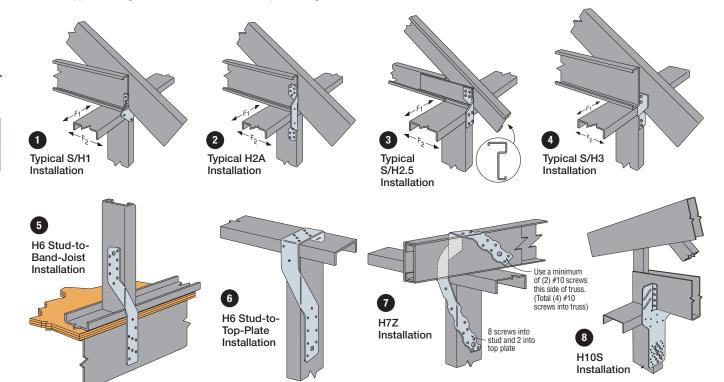
These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. Loads are based on attachment of cold-formed steel members having a minimum thickness of 33 mil (20 ga.).

2. Hurricane ties are shown installed on the outside of wall for clarity. Installation inside of wall is acceptable. For Continuous Load Path, connections in the same area must be on same side of wall.

3. For H10S connectors with CFS members having a minimum thickness of 43 mil (18 ga.), the allowable load is 1,260 lb. 4. H10S connectors can be installed 3/4" (max.) from the center of the vertical stud per the in-line framing specifications

of the AISI General Provisions for reduced uplift of 890 lb., provided that the screw edges are met. 5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



H Hybrid Connectors Seismic and Hurricane Ties for Wood Truss or Joist-to-CFS Wall

Designed to provide seismic and wind ties for wood trusses or joists-to-CFS walls, this versatile line may be used for general purposes, strongback attachments, and as all-purpose ties where one member crosses another.

HS24 attaches the bottom chord of a truss or rafter at pitches from 0:12 to 4:12 to steel top plates.

Material: See table

Finish: Galvanized. Selected products available in stainless steel or ZMAX[®] coating. See Corrosion Information, pp. 18–21.

Installation: • Use all specified fasteners; see General Notes

- H1 can be installed with flanges facing inward (reverse of illustration 1)
- Hurricane ties do not replace solid blocking
- H3 and H6 ties are only shipped in equal quantities of rights and lefts

Codes: See p. 11 for Code Reference Key Chart

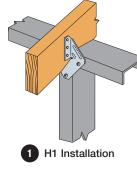
	Connector		Fasteners ⁵		Allowable	Uplift Load	
Model	Material	To Rafters/	То		33 mil (20 ga	Code	
No.	Thickness mil (ga.)	Truss	Top Track	To Stud	DF/SP	SPF/HF	Ref.
H1	43 (18)	(6) 8d x 1½"	(3) #10	(1) #10	600	500	
H2A	43 (18)	(5) 8d x 1½"	(1) #10	(5) #10	550	460	
H3	43 (18)	(4) 8d x 1½"	(4) #10	—	365	305	
H6	54 (16)	—	(8) 8d	(8) #10	950	820	170
H7Z	54 (16)	(4) 8d x 11⁄2"	(2) #10	(8) #10	985	845	
HS24	43 (18)	(8) 8d x 1½"	(4) #10	(4) #10	625	520	
H10S ⁵	43 (18)	(8) 8d x 1½"	—	(8) #10	930	780	

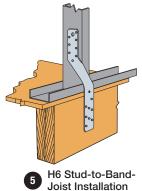
These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

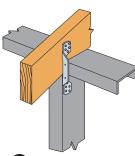
- 1. Allowable loads on wood have been increased 60% for wind or earthquake loading with no further increase allowed; reduce where other load duration factors govern.
- Hurricane Ties are shown installed on the outside of wall for clarity. Installation inside of wall is acceptable. For Continuous Load Path, connections must be on same side of wall.
- 3. When cross-grain bending or cross-grain tension cannot be avoided, mechanical
- be considered. 4. H10S connectors can be installed ¾" (max.) from the center of the vertical stud per the in-line framing specifications of the AISI

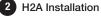
reinforcement to resist such force should

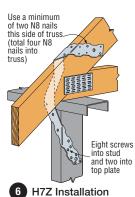
- in-line framing specifications of the AISI General Provisions for reduced uplift of 890 lb., provided that the screw edges are met. 5. See pp. 138 through 171 for more information
- 5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

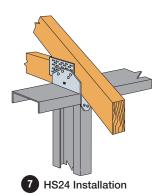




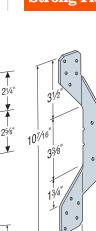


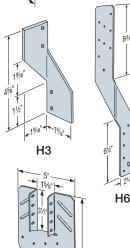






3 H3 Installation





115%

:0

0

[∼]³'⊶[H10S HS24

U.S. Patent

5.603.580

1%16'

0

51/4"

H1





H7Z

H6 Stud-to-Top-

Plate Installation

4

S/H1A Seismic and Hurricane Ties

S/H1A is designed to fit within several proprietary truss chords to provide uplift resistance.

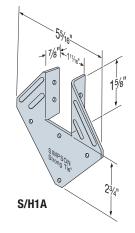
Material: 43 mil (18 ga.)

Finish: Galvanized

Installation:

- Use all specified fasteners.
- S/H1A can be installed with flanges facing outwards, reverse of illustration, when installed inside a wall for truss applications.
- S/H1A does not replace solid blocking.
- S/H1A may be used with proprietary truss sections. Contact material supplier for specific installation details.

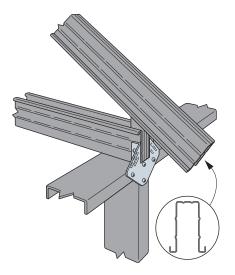
Codes: See p. 11 for Code Reference Key Chart



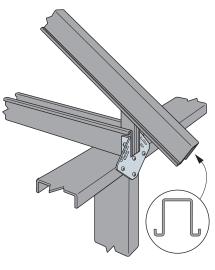
Model		Fasteners ²		Truss	All	b.)			
Model No.	Truss	Тор	Stud	Thickness	ss Plate/Wall Stud Thickness mil (ga.)				
	ITUSS	Track	Stuu	mil (ga.)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)		
	(4) #10	(3) #10	(1) #10	27 (22)	470	470	470		
0/1114	(4) #10	(3) #10	(1) #10	33 (20)	510	550	690	IP1,	
S/H1A	(4) #10	(3) #10	(1) #10	43 (18)	510	550	690	FL, L2	
	(4) #10	(3) #10	(1) #10	54 (16)	520	675	850		

1. Tabulated loads based on truss members with yield strength, F_y, of 50 ksi and tensile strength, F_u, of 65 ksi. Reduce tabulated load proportionally for lower truss member steel strength. For example: 43 mil (18 ga.) truss member with a yield strength, F_y, of 33 ksi and a tensile strength, F_u, of 45 ksi is connected to 43 mil top track and wall stud. The adjusted allowable load is then 550 lb. x minimum [33/ 50 or 45/ 60] = 363 lb.

2. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Typical S/H1A Installation



Typical S/H1A Installation

MTSM/HTSM Twist Straps

The MTSM and HTSM offer high-strength truss-tomasonry connections.

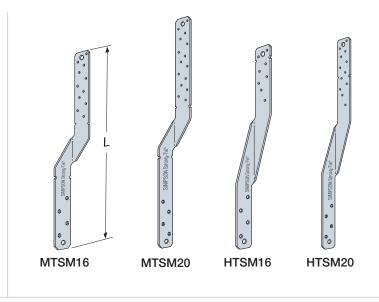
Material: See table.

Finish: Galvanized. Some products available in stainless steel and ZMAX[®]; see Corrosion Information, pp. 18–21.

Installation:

- Use all specified fasteners; see General Notes.
- May be attached to either side of a grouted block wall. A minimum of one #5 horizontal rebar shall be installed in the top course of this wall.

Codes: See p. 11 for Code Reference Key Chart



	Connector Material Thickness mil (ga.)	nector				Allowable Load 33 mil			
Model		L	Rafter/Stud/Joist Thickness						Code
No.		(in.)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	CMU⁵	Concrete ^{3,5}	(20 ga.) (lb.)	Ref.
MTSM16	E4 (16)	16	(5) #10	(4) #10	(3) #10	(4) 1⁄4" x 21⁄4" Titen 2	(4) ¼" x 1¼" Titen 2	860	
MTSM20	54 (16)	20	(5) #10	(4) #10	(3) #10	(4) 1⁄4" x 21⁄4" Titen 2	(4) ¼" x 1¼" Titen 2	860	170
HTSM16	68 (14)	16	(7) #10	(5) #10	(3) #10	(4) ¼" x 2¼" Titen 2	(4) ¼" x 1¼" Titen 2	1,175	170
HTSM20	68 (14)	20	(7) #10	(5) #10	(3) #10	(4) 1⁄4" x 21⁄4" Titen 2	(4) ¼" x 1¼" Titen 2	1,175	

1. All straps have additional fastener holes.

2. Twist straps do not have to be wrapped over the truss to achieve the load.

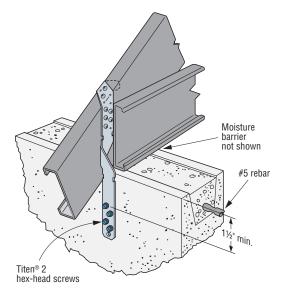
3. Minimum edge distance in concrete block for Titen® 2 screws is 11/2".

4. Straps can be installed on the inside face of the wall.

5. Min. $f'_m = 1,500$ psi and $f'_c = 2,500$ psi.

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6. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Typical MTSM20 Installation

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META/HETA/HHETA Embedded Truss Anchors

The embedded truss anchor series provides an engineered method to properly attach roof trusses to concrete and masonry walls. Information regarding the use of two anchors on single- and multi-ply trusses is included below.

Material: HHETA - 14 gauge; HETA - 16 gauge; META - 18 gauge

Finish: Galvanized. Some products available in ZMAX $^{\odot}$ coating; see Corrosion Information on pp. 18–21

Installation:

- Use all specified fasteners; see General Notes.
- The META, HETA and HHETA are embedded 4" into a 6" minimum concrete beam or 8" nominal grouted block wall.
- For mislocated truss anchors which are greater than 1%" but less than 1 1/2" from the face of the truss, a shim must be provided. Shim design by Truss Engineer. When gap is greater than 1 1/2", install retrofit anchors.
- In double embedded anchor installations, do not install fasteners where the straps overlap when wrapped over the truss heel.

Codes: See p. 11 for Code Reference Key Chart

Single Embedded Anchor Installation

- 0 -								
		Fastener ⁹	Allowa	ble Uplift Lo	ad (lb.)	Latera	l Load ⁸	
Model No.	H (in.)	Rafter/Stud/Truss Thickness	Ra	fter/Stud/Tru Thickness	uss	F1	F ₂	Code Ref.
		33 mil, 43 mil and 54 mil (20 ga., 18 ga. and 16 ga.)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	54 mil (16 ga.)		
META12	8	(7) #10	1,240	1,450	1,450			
META16	12	(9) #10	1,450	1,450	1,450			
META18	14	(9) #10	1,450	1,450	1,450			
META20	16	(9) #10	1,450	1,450	1,450	340	725	
META22	18	(9) #10	1,450	1,450	1,450			
META24	20	(9) #10	1,450	1,450	1,450			
META40	36	(9) #10	1,450	1,450	1,450			
HETA12	8	(7) #10	1,240	1,780	1,780			
HETA16	12	(9) #10	1,595	1,810	1,810			170
HETA20	16	(9) #10	1,595	1,810	1,810	340	725	
HETA24	20	(9) #10	1,595	1,810	1,810			
HETA40	36	(9) #10	1,595	1,810	1,810			
HHETA12	8	(7) #10	1,240	1,820	1,820			
HHETA16	12	(10) #10	1,770	2,235	2,235			
HHETA20	16	(10) #10	1,770	2,235	2,235	3405	815	
HHETA24	20	(10) #10	1,770	2,235	2,235			
HHETA40	36	(10) #10	1,770	2,235	2,235			

These products are available with additional corrosion protection. Additional products on

this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. Allowable loads may not be increased for wind or seismic load.

- 2. Minimum $f'_c = 2,500$ psi. Minimum $f'_m = 1,500$ psi.
- 3. For simultaneous loads in more than one direction, the connector must be evaluated as described in Note d, p. 14 under Instructions to the Designer.

4. It is acceptable to use a reduced number of fasteners provided that there is a reduction in uplift load capacity. Lateral loads do not apply when fewer fasteners are used.

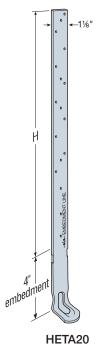
5. The HHETA allowable F_1 load can be increased to 435 lb. if the strap is wrapped over the truss and a minimum of 12 fasteners are installed.

6. Minimum spacing for multiple anchor installation is two times the embedment depth for full load. See Double Embedded Anchor Installation table on p. 267 for loads on closer spaced anchors.

7. Minimum edge distance is 11/2" for concrete and 2" masonry.

8. Lateral loads are limited to 54 mil (16 ga.) CFS members.

9. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Strong

(META/HHETA similar)

Roof, Truss and Rafter Connectors, Ties and Straps

META/HETA/HHETA Embedded Truss Anchors

Double Embedded Anchor Installation

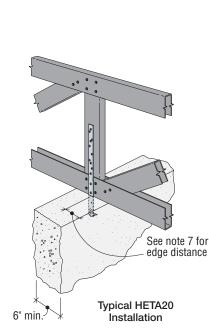
			Fasteners ⁸	Allov	vable Uplift	Load	Latera	l Load ⁷
Model	Qty.	Application	Rafter/Stud/Truss Thickness		(lb.)		F1	F2
No.			33 mil, 43 mil and 54 mil (20 ga., 18 ga. and 16 ga.)	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	54 mil (16 ga.)	
META	2	CMU	(10) #10	1,770	1,985	1,985	340	725
META	2	Concrete	(10) #10	1,770	1,985	1,985	340	725
HETA	2	CMU	(10) #10	1,770	2,035	2,035	340	725
I DE IA		Concrete	(10) #10	1,770	2,035	2,035	340	725
HHETA	2	CMU	(10) #10	1,770	2,035	2,035	340	815
TITICIA	2	Concrete	(10) #10	1,770	2,235	2,235	540	010
			3" Width Minimum of R	after/Stud/	Truss			
META	2	CMU	(14) #10	1,900	1,900	1,900	1,210	1.160
IVIETA	2	Concrete	(14) #10	2,480	2,565	2,565	1,210	1,100
HETA	2	CMU	(12) #10	2,480	2,500	2,500	1,225	1,520
HEIA	2	Concrete	(12) #10	2,480	2,700	2,700	1,220	1,520
		CMU	(12) #10	2,480	2,500	2,500		
HHETA	2	Concrete	(12) #10	2,480	3,050	3,050	1,225	1,520
		Concrete	(14) #10	2,480	3,350	3,350		

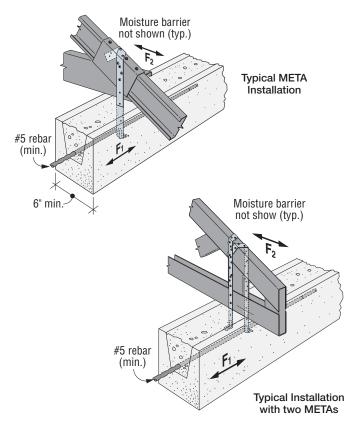
1. Allowable loads may not be increased for wind or seismic load.

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Strong-T

- 2. Minimum f^{\prime}_{C} = 2,500 psi. Minimum f^{\prime}_{m} = 1,500 psi.
- For simultaneous loads in more than one direction, the connector must be evaluated as described in Note d, p. 14 under Instructions to the Designer.
- Minimum spacing for multiple anchor installation is two times the embedment depth for full load. See Double Embedded Anchor Installation table for loads on closer spaced anchors.
- Install with spoons facing outward and straps spaced no more than 1/8" wider than the truss width.
- 6. F1 lateral loads listed may cause an additional 1/16" deflection beyond the standard 1/16" limit there the straps are installed not wrapped over the heel as shown.
- as shown. 7. Lateral loads are limited to 54 mil (16 ga.) CFS members.
- CFS members. 8. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.





LTS/MTS/HTS Twist Straps

Twist straps provide a tension connection between two members. They resist uplift at the heel of a truss economically. LTS/ MTS have a 2"-bend section and HTS has a 3¾"-bend section that eliminates interference at the transition points between the two members.

Material: See table

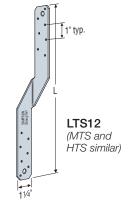
Finish: Galvanized. Some products available in stainless steel and ZMAX®; see Corrosion Information, pp. 18–21

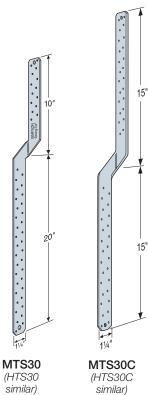
Installation:

- Use all specified fasteners; see General Notes.
- LTS, MTS and HTS are available with the bend reversed. Specify "-REV" after the model number, such as MTS16-REV.

Codes: See p. 11 for Code Reference Key Chart

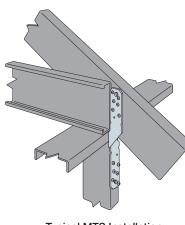
	Connector			Fasteners ⁶ (Total)		Allowable Tension Load (lb.)	
Model No.	Material	L	Rafter/	Stud/Joist Thi	ckness	33 mil (20 ga.)	Code Ref.
NU.	mil (ga.)	(in.)	33 mil	43 mil	54 mil	43 mil (18 ga.)	nei.
			(20 ga.)	(18 ga.)	(16 ga.)	54 mil (16 ga.)	
LTS12		12	(10) #10	(6) #10	(6) #10	775	
LTS16	40 (10)	16	(10) #10	(6) #10	(6) #10	775	
LTS18	43 (18)	18	(10) #10	(6) #10	(6) #10	775	
LTS20		20	(10) #10	(6) #10	(6) #10	775	
MTS12		12	(12) #10	(8) #10	(6) #10	995	
MTS16		16	(12) #10	(8) #10	(6) #10	995	
MTS18		18	(12) #10	(8) #10	(6) #10	995	
MTS20	54 (16)	20	(12) #10	(8) #10	(6) #10	995	IP1,
MTS30		30	(12) #10	(8) #10	(6) #10	995	FL, L2
MTS24C		24	(12) #10	(8) #10	(6) #10	995	
MTS30C		30	(12) #10	(8) #10	(6) #10	995	
HTS16		16	(16) #10	(12) #10	(6) #10	1,415	
HTS20	68 (14)	20	(18) #10	(12) #10	(6) #10	1,450	
HTS24		24	(18) #10	(12) #10	(6) #10	1,450	
HTS30		30	(18) #10	(12) #10	(6) #10	1,450	
HTS30C		30	(18) #10	(12) #10	(6) #10	1,450	





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Typical MTS Installation Truss to Steel Studs

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. Not all fastener holes need to be filled, as additional fastener holes are provided. Install fasteners symmetrically.

- 2. Install half of the fasteners on each end of strap to achieve full loads.
- 3. All straps except the MTS30 and HTS30 have the twist in the center of the strap.

4. Twist straps do not have to be wrapped over the truss to achieve the load.

5. May be installed on the inside face of the stud.

6. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

Roof, Truss and Rafter Connectors, Ties and Straps

VGT and SVGT2.5 Variable-Pitch Girder Tiedown

0

VGT U.S. Patent 7,707,785

41/4

SIMPSON Strong-Tie

The variable-pitch girder tiedown, S/VGT2.5, is a high-capacity tiedown for single- or multi-ply CFS girder trusses. It attaches with self-drilling screws from the side of the truss. The VGT uses Simpson Strong-Tie[®] Strong-Drive[®] SDS Heavy-Duty Connector screws for wood truss applications. They both feature a predeflected crescent washer that allows them to accommodate top-chord pitches up to 8/12.

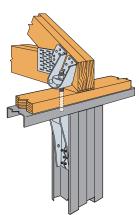
Material: 171 mil (7 ga.)

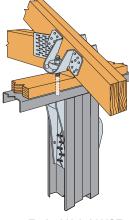
Finish: Galvanized

Installation:

- Use all specified fasteners
- Screw holes are configured to allow for double installation on multi-member girders
- Install washer component (provided) so that top of washer is horizontal and parallel with top of wall

Codes: See p. 11 for Code Reference Key Chart

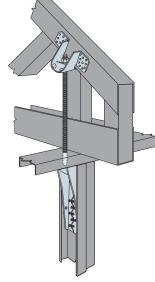




Typical Hybrid VGTR Single Installation with S/HDU4

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Typical Hybrid VGT Double Installation with S/HDU6



23/4" ---

0

0

0

31/2

Typical S/VGT2.5 Single Installation with HDU6

Allowable Uplift Load² No. of **Fasteners**⁵ (lb.) Model No. Truss Quantity Code Plies Anchor Dia. **Girder Truss** 3/12 8/12 Ref. **Cold-Formed Steel Connection** 54 mil (16 ga.) 1 1 (1) 5/8 (16) #14 3,050 2,620 S/VGT2.5 (min.)4 2 2 (32) #14 6,100 5,240 (2) 5/8 170 3.860 (1) 5/8 (20) #14 3,130 1 1 S/VGT2.5 (max.) 2 2 (2) 5/8 (40) #14 7,720 6,260 Allowable Uplift Load³ Code up to 8/12 (lb.) **Hybrid Connection** Ref. DF/SP (160) SPF/HF (160) 2 (1) % (16) 1/4" x 3" SDS 4,940 3,555 1 VGT (32) 1/4" x 3" SDS 7,185 2 2 (2) 5/8 5,175 2 3 (2) 5% (32) 1⁄4" x 3" SDS 8,890 6,400 FL 1 2 (1) 5/8 (16) 1/4" x 3" SDS 2,230 1,605 VGTR/L 2 2 (2) 5% (32) 1⁄4" x 3" SDS 5,545 3,990

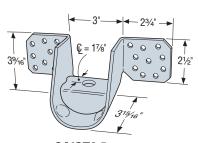
1. Designer shall insure attached members are adequately designed to resist applied loads.

2. Straight-line interpolation can be used to determine allowable loads for pitches between 3/12 and 8/12.

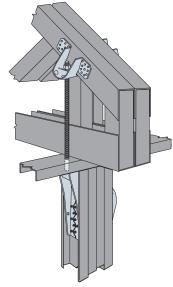
3. Allowable loads on wood have been increased 60% for wind or earthquake loading with no further increase allowed; reduce where other load duration factors govern.

4. For (min.) tabulated values, not all screw holes need to be filled. Install screws symmetrically.

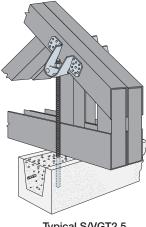
5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



S/VGT2.5 U.S. Patent 7,707,785



Typical S/VGT2.5 Double Installation with S/HDU6



Typical S/VGT2.5 Installation in CMU

LTA2, S/HGAM10 and H10S Seismic and Hurricane Ties



The LTA2 develops high uplift at a minimum heel height. Great in areas where a strap over the heel is not required. The side tab acts as a locator in the masonry block and the four embedded hooks provide for a positive bond in the concrete grout.

The H10S and the S/HGAM10 attaches to truss joist and provides good uplift resistance.

Material: See table

Finish: Galvanized

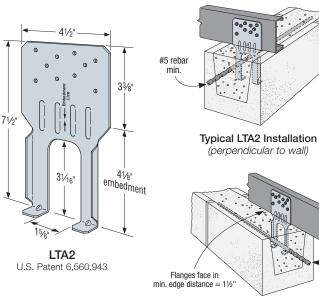
Installation:

- Use all specified fasteners; see General Notes.
- S/HGAM10 can be installed into grouted concrete block
- Titen[®] 2 screws are provided
- Hurricane ties do not replace solid blocking
- Attach to grouted concrete block with a minimum one #5 rebar horizontal in the top lintel block

Codes: See p. 11 for Code Reference Key Chart

Ordering Information:

• The HGAM10KT is a kit of (10) connectors with (40) ¼"x 2¾" Titen 2 screws.



Typical LTA2 Installation (parallel to wall)

#5 rebar

	Connector		Fasteners		Allowable Up		
Model No.	Material Thickness mil (ga.)	To Rafter/ Truss⁵	To CMU	To Concrete	33 mil (20 ga.)	43 mil (18 ga.)	Code Ref.
LTA2 perpendicular-to- wall installation	43 (18)	(10) #10	Embed	Embed	1,295	1,425	170
LTA2 parallel-to-wall installation	43 (18)	(10) #10	Embed	Embed	1,295	1,390	170
S/HGAM10KT	68 (14)	(4) #14	(4) 1⁄4" x 23⁄4" Titen 2	(4) 1⁄4" x 13⁄4" Titen 2	810	850	IP1,
H10S	43 (18)	(8) #10	(2) %" x 4" Titen HD	(2) %" x 4" Titen HD	915	1,245	L2, FL

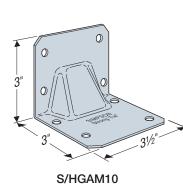
1. Min. f^{\prime}_{m} = 1,500 psi and f^{\prime}_{c} = 2,500 psi.

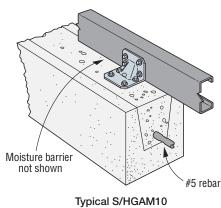
2. Minimum edge distance is 11/2" for Titen® 2 screws.

3. The products shall be installed such that the Titen 2 screws and Titen HD[®] anchors

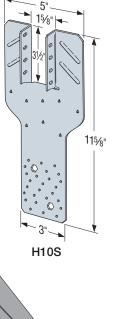
are not exposed to the weather.

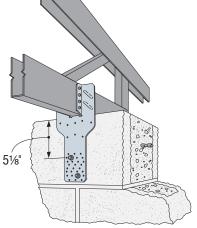
- 4. See p. 204 for Titen 2 screw information and p. 189 for Titen HD information.
- 5. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.





Installation





H10S Installation

TJC Jack Truss and Rafter Connector



TJC is a versatile connector for skewed members. Adjustable from 0° to 67.5° (shipped with 67.5° bend). Screw hole locations allow for easy installation.

Material: 54 mil (16 ga.)

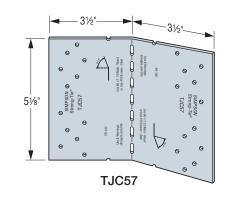
Finish: Galvanized

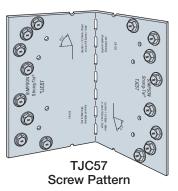
Installation:

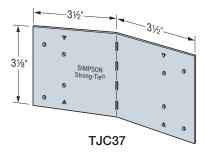
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- Use all specified fasteners; see General Notes
- Position the skewed member on the inside of the bend line with the end of the member flush with the bend line
- Bend the TJC to the desired position (one bend cycle only)

Codes: See p. 11 for Code Reference Key Chart



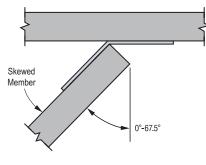




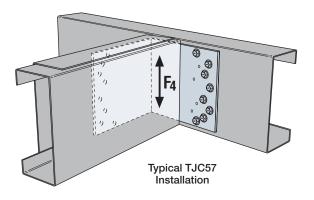
	Faste	eners ²	Member	Allov	vable Load F	4 (lb.)	Code Ref.
Model No.	Carrying Member	Carried Member	Thickness mil (ga.)	0°	1°-60°	61°–67.5°	
TJC37 (min.)	(4) #10	(4) #10	43 (18)	660	565	475	IP1,
TJC37 (max.)	(6) #10	(6) #10	43 (18)	680	630	530	L2, FL
TJC57 (min.)	(8) #10	(8) #10	43 (18)	1,295	1,215	1,235	170
TJC57 (max.)	(8) #10	(8) #10	54 (16)	1,790	1,790	1,790	170

1. Reference the illustration for the required screw pattern of the TJC57.

2. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Top View Installation



LS and S/LS Skewable Angles



LS and S/LS skewable angles are a cost effective method for connecting roof rafters to hip rafters.

Material: 43 mil (18 ga.)

Finish: Galvanized (G90)

Installation:

- Use all specified fasteners
- Field-skewable; bend one time only

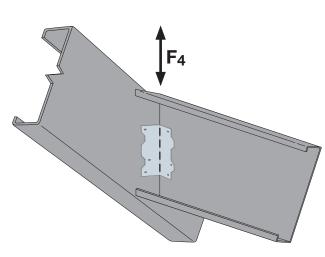
Codes: See p. 11 for Code Reference Key Chart

			Allo	(lb.)		
Model No.	Length (in.)	Fasteners ²	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	Code Ref.
			F4	F4	F4	
LS30	37⁄8	(8) #10	200	370	500	
S/LS50	41⁄8	(4) #10	200	370	500	170
S/LS70	6%	(6) #10	465	575	715	170
LS90	71⁄8	(12) #10	465	895	915	

1. Loads are for one part only.

2. See pp. 138 through 171 for more information on

Simpson Strong-Tie fasteners.



Typical Installation Between Roof Rafter and Hip Rafter

S/HTC Heavy Truss Clips

 $\ensuremath{\mathsf{S/HTC}}$ provides a slotted connection from the truss or joist to the top track when isolation of two members is required.

Material: 43 mil (18 ga.)

Finish: Galvanized

Installation:

- Use all specified fasteners
- Screws in vertical slots shall not be driven completely flush against the connector when vertical movement is desired

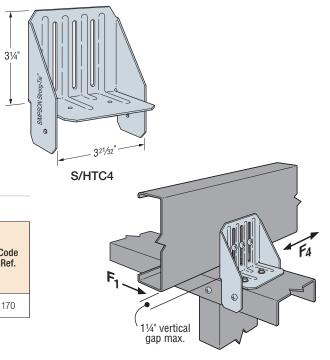
Codes: See p. 11 for Code Reference Key Chart

	Faste	ners ³	Allow				
Model No.	Тор	Truss	Without Gap ¹		ithout Gap ¹ With 1 ¼" Ga		Code Ref.
	Track	11055	F1	F4	F1	F4	
S/HTC4	(4) #8	(3) #8	320	460	85	175	170

1. Truss or rafter must be bearing on top plate to achieve the allowable loads under "Without Gap."

2. Installed with maximum 1¼" space between rafter or truss and top plate under "With 1¼" Gap." Where loads are not required, space is not limited to 1¼".

3. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Typical S/HTC4 Installation

STC/STCT/DTC Roof Truss Clips

SIMPSON Strong-Tie

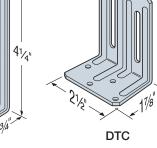
For alignment control between a roof truss and nonbearing walls; the 1½" slot permits vertical truss chord movement when loads are applied.

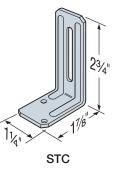
Material: 43 mil (18 ga.)

Finish: Galvanized

Installation:

- Use all specified fasteners; see General Notes
- Use STC or DTC depending on required loads
- STC / STCT / DTC may be used with proprietary material sections. Contact material supplier for specific installation details
- Use STCT where truss or rafter is separated from the top plate of the nonbearing wall
- Install slot screws in the middle of the slot
- Codes: See p. 11 for Code Reference Key Chart





23/4'

STCT

	Fasteners ³								
Model No.	Base	Slot	Without Gap		1⁄4" Max. Gap		1⁄4"< Gap ≤ 1⁄2"		Code Ref.
	Dase	5101	F1	F4	F1	F4	F1	F4	
STC	(2) #8	(1) #8	185	35	135	35	75	35	IP1, L2, FL
STCT	(2) #8	(1) #8	_	_	_	_	_	_	180
DTC	(4) #8	(2) #8	200	160	200	160	145	160	IP1, L2, FL

1. Truss or rafter must be bearing on top plate to achieve the allowable loads

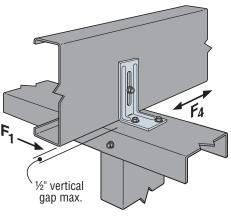
under "Without Gap."

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2. Clips are required on both sides of the truss to achieve F_1 loads in both directions

(stagger parts to avoid screw interferences).

3. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



To allow for vertical truss movement, screws into the truss or rafter should not be driven completely flush against the connector. Roof, Truss and Rafter Connectors, Ties and Straps

AHEP Adjustable Hip-End Purlin

The Simpson Strong-Tie AHEP is a structural purlin that also serves as an installation aid during the truss-erection process. The AHEP attaches to the step-down hip trusses at the leading edge, eliminating the need for drop top chords and C-stud fillers. The AHEP installs linearly, aligned with the end jacks, to maintain sheathing spacing from eave to hip or peak. Roof sheathing/decking attaches directly to the purlin. Adjustable in length, the AHEP is designed to accommodate a pitch range of 3/12 to 9/12.

Material: 33 mil (20 ga.)

Finish: Galvanized

Installation:

• Use all specified fasteners; see General Notes

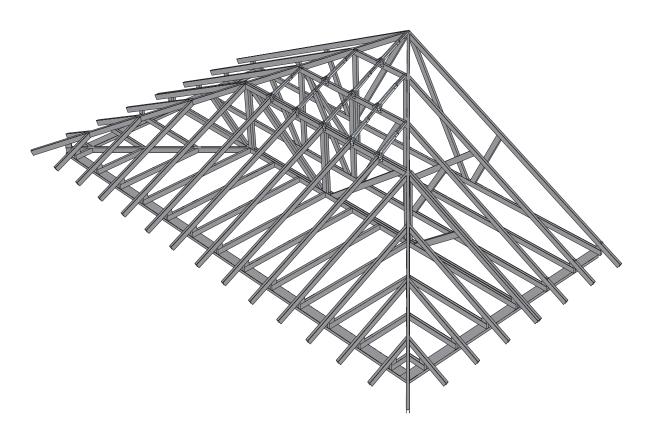
Codes: See p. 11 for Code Reference Key Chart

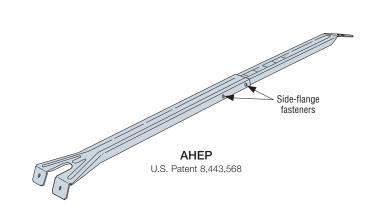
Model	Faste	ners ³	Sheathing	Allowable Down Load 33 mil (20 ga.) (lb.)				Code Ref. 3%e" 175
No.	Side	Truss	Option	3/12	Pitch	9/12	Pitch	
	Flanges	Ends		L/180	3⁄16"	L/180	3⁄16"	
AHEP	(4) #10	(4) #10	None	285	360	160	175	170
ALLE	(4) #10	(4) #10	1/2" wood sheathing	285	360	205	225	170

1. Designer shall ensure attached members are adequately designed to resist applied loads.

2. Straight-line interpolation can be used to determine allowable loads for pitches between 3/12 and 9/12.

3. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.









TBD22 Truss Brace Diagonal

The TBD22 diagonal truss brace offers a time-saving substitute for hat channel or C-stud diagonal bracing that helps meet the prescriptive recommendations of CFSBCSI. The TBD22 travels in a box like a flat strap (160 feet per carton), and is formed into an A-shape as it is pulled from the carton to provide rigidity and prevent sagging between trusses during installation. As it is fastened to the trusses the brace flattens, allowing sheathing to be installed over it, saving the time typically needed to remove bracing prior to applying the sheathing or decking.

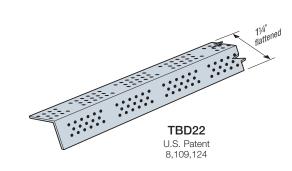
Material: 27 mil (22 ga.)

Finish: Galvanized

Installation: • Use all specified fasteners; see General Notes.

- · Strap does not have holes for fasteners. Screws shall be installed in the dimpled areas and placed to maintain a minimum of 1/4" strap edge distance and a minimum of 1/2" center-to-center distance.
- TBD22 strap span diagonally at approximately 45°.
- Strap shall not be slack, but tight and ready to engage in tension.
- To resist construction forces, diagonal X-bracing is required at each end and every ten truss spaces (20' max.). Refer to SBCA CFS-BCSI for additional information.
- At the end of the TBD22 braces, trusses shall be laterally restrained to resist out-of-plane forces.
- · Bracing locations shown in the drawing are recommendations for temporary bracing only. Installation of TBD22 for permanent bracing shall be by the Building Designer or Engineer of Record.

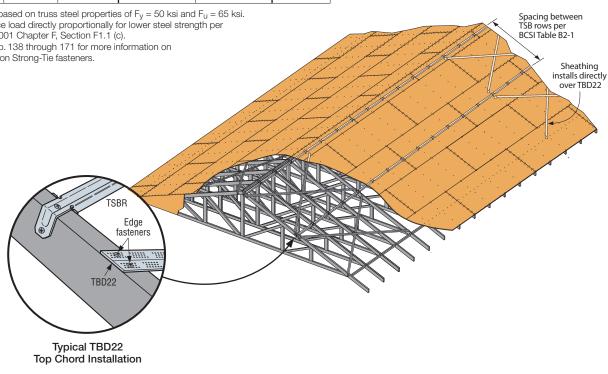
Codes: See p. 11 for Code Reference Key Chart



	Fasteners ²		Allowable Ten	sion Load (lb.)	
Model No.	Strap	Intermediate	Truss Memb	er Thickness	Code Ref.
	Ends	Trusses	27 mil (22 ga.)	33 mil (20 ga.)	
TBD22	(2) #10	(2) #10	380	510	IP1, L2, FL

1. Load based on truss steel properties of $F_V = 50$ ksi and $F_u = 65$ ksi. Reduce load directly proportionally for lower steel strength per AISI 2001 Chapter F, Section F1.1 (c).

2. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.



Lateral Connectors, Ties and Straps

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TSBR Truss Spacer Restraint

The TSBR captures the on-center spacing of CFS truss chords and webs and laterally restrains the truss members, allowing quicker, easier and safer installations. The tubular shape provides strength in both compression and tension.

Material: 27 mil (22 ga.)

Finish: Galvanized

Installation:

Use all specified fasteners; see General Notes

Codes: See p. 11 for Code Reference Key Chart

	Dimensions (in.			n.)	Faste	ners ³	Allowable	Load (lb.)	
Model No.	w	L ²	Н	В	Bend End	Straight End	Compression 33 mil (20 ga.)	Tension 33 mil (20 ga.)	Code Ref.
TSBR2-24	1½	251/2	1	13⁄4	(1) #10	(1) #10	455	215	170
13Dh2-24	1 72	20 /2	I	194	(2) #10	(2) #10	455	575	170

1. Designer shall ensure attached members are adequately designed to resist applied loads.

2. Length, L, equals the effective out-to-out dimension of the braced trusses.

3. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

S/DSC Drag Strut Connector

The S/DSC is used as a drag strut transferring loads from roof framing to the wall plates below.

Material: 229 mil (3 ga.)

Finish: Simpson Strong-Tie® gray paint

Installation:

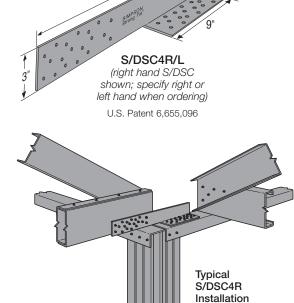
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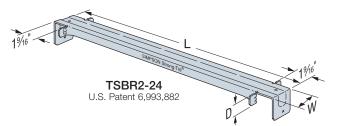
• Use all specified fasteners; see General Notes

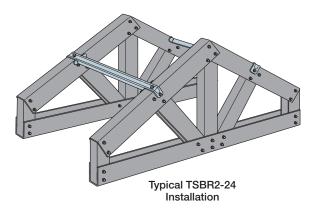
Codes: See p. 11 for Code Reference Key Chart

			Allowable		
Model No.	L (in.)	Fasteners ¹	43 mil (18 ga.)		Code Ref.
			Compression	Tension	
S/DSC4R/L	22	(40) #10	3,220	4,025	170

1. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.







SIMPSON Strong-Ti

HRS/ST/FHA/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI Strap Ties

SIMPSON Strong-Tie

Straps are load rated and provide the correct thickness and number of fasteners the specifier is looking for compared with field fabricated straps.

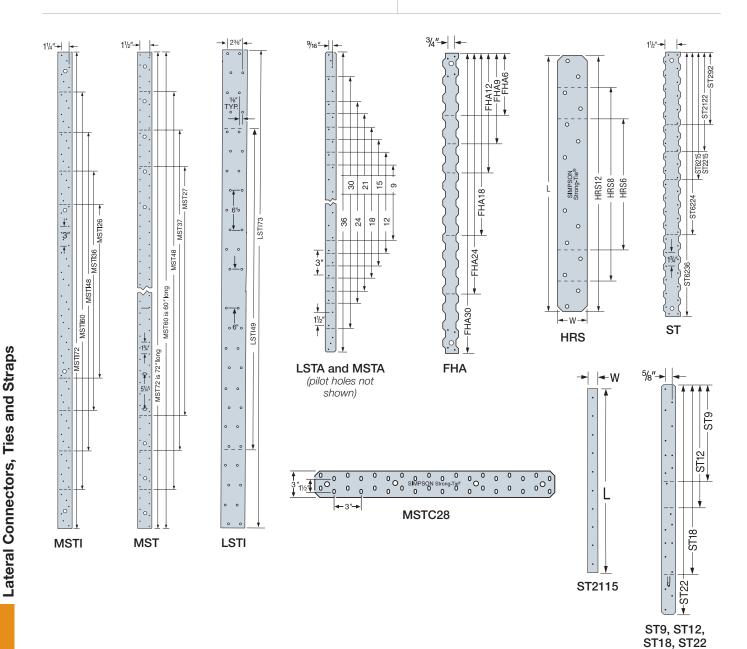
Install strap ties where top or bottom plates are cut, at wall intersections, and as ridge ties. Reduce the allowable load based on the size and quantity of fasteners used.

Refer to applicable code for minimum edge and end distances.

 $\label{eq:Finish: PS - hot-dip galvanized (HDG); all others - galvanized. Some products are available in stainless steel or ZMAX^{\circledast}; see Corrosion Information, pp. 18–21.$

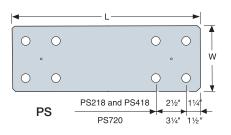
Installation: Use all specified fasteners; see General Notes

Codes: See p. 11 for Code Reference Key Chart



Model	Connector Material	Dimensi	ons (in.)	Во	lts	Code	4
No.	Thickness mil (ga.)	W	L	Qty.	Dia. (in.)	Ref.	1
PS218		2	18	4	5⁄8		
PS418	171 (7)	4	18	4	5⁄8	180	
PS720		6¾	20	8	1⁄2		

I. PS strap design loads must be determined by the Designer for each illustration. Hole diameter in the part may be oversized to accommodate the HDG. Designer must determine if the oversize creates an unacceptable installation.



HRS/ST/FHA/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI Strap Ties

Codes: See p. 11 for Code Reference Key Chart

5	M	P	S	O	N

Strong-Tie

		Connector	Dimer	nsions		Fasteners⁴ (Total)		Allowab	le ASD Tension I	_oad (lb.)	
	Model No.	Material Thick.	(in.) W L		Rafte	er/Stud/Joist Thick	iness	33 mil	43 mil	54 mil	Code Ref.
		mil (ga.)	W	L	33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	(20 ga.)	(18 ga.)	(16 ga.)	
	LSTA9		11⁄4	9	(8) #10	(8) #10	(8) #10	705	1,120	1,190	
	LSTA12		11⁄4	12	(10) #10	(10) #10	(8) #10	885	1,190	1,190	
	LSTA15		11⁄4	15	(12) #10	(12) #10	(10) #10	1,060	1,190	1,190	
	LSTA18		11⁄4	18	(14) #10	(12) #10	(10) #10	1,190	1,190	1,190	
	LSTA21	33 (20)	11⁄4	21	(14) #10	(12) #10	(10) #10	1,190	1,190	1,190	
	LSTA24	33 (20)	11⁄4	24	(14) #10	(12) #10	(10) #10	1,190	1,190	1,190	J
	ST292		21⁄16	95⁄16	(12) #10	(10) #10	(10) #10	1,060	1,240	1,240	
	ST2122		21⁄16	12 ¹³ ⁄16	(16) #10	(12) #10	(10) #10	1,415	1,502	1,502	
	ST2115		3⁄4	165/16	(8) #10	(6) #10	(4) #10	630	630	630	
	ST2215		21⁄16	165/16	(20) #10	(14) #10	(10) #10	1,765	1,825	1,825]
	LSTA30		1 1⁄4	30	(18) #10	(12) #10	(10) #10	1,555	1,555	1,555	
	LSTA36		1 1⁄4	36	(18) #10	(16) #10	(14) #10	1,555	1,555	1,555	1
	LSTI49		3¾	49	(32) #10	(32) #10	(20) #10	2,830	4,050	4,050	1
	LSTI73		3¾	73	(46) #10	(32) #10	(20) #10	4,050	4,050	4,050	1
	MSTA9	40 (40)	1 1⁄4	9	(8) #10	(8) #10	(8) #10	705	1,050	1,555	1
	MSTA12	43 (18)	1 1⁄4	12	(10) #10	(10) #10	(8) #10	885	1,315	1,555	
	MSTA15		1 1⁄4	15	(12) #10	(12) #10	(10) #10	1,060	1,555	1,555	1
	MSTA18		11⁄4	18	(14) #10	(12) #10	(10) #10	1,235	1,555	1,555	
	MSTA21		11⁄4	21	(16) #10	(12) #10	(10) #10	1,415	1,555	1,555	
51	MSTA24		11⁄4	24	(18) #10	(12) #10	(10) #10	1,555	1,555	1,555	
5	MSTA30		11⁄4	30	(22) #10	(16) #10	(12) #10	1,945	1,950	1,950	
5	MSTA36		11⁄4	36	(24) #10	(18) #10	(16) #10	1,950	1,950	1,950	
	ST6215		21/16	165/16	(20) #10	(16) #10	(10) #10	1,765	2,025	2,025	IP1,
	ST6224	-	21/16	235/16	(28) #10	(20) #10	(12) #10	2,455	2,455	2,455	L2, FL
	ST9		11⁄4	9	(8) #10	(8) #10	(8) #10	705	1,050	1,350	1
	ST12	54 (16)	11⁄4	115%	(10) #10	(10) #10	(8) #10	885	1,315	1,350	1
	ST18	01(10)	11/4	173⁄4	(14) #10	(12) #10	(12) #10	1,235	1,350	1,350	
	ST22		11/4	215%	(20) #10	(20) #10	(20) #10	1,350	1,350	1,350	-
-	MSTC28		3	281/4	(36) #10	(36) #10	(30) #10	3,180	4,600	4,600	1
-	MSTC40		3	401/4	(52) #10	(46) #10	(46) #10	4,595	4,600	4,600	1
	MSTC52		3	521/4	(54) #10	(42) #10	(42) #10	4,600	4,600	4,600	1
	MSTC66		3	653/4	(66) #10	(46) #10	(30) #10	5,795	5,795	5,795	
	MSTC78	68 (14)	3	773/4	(66) #10	(46) #10	(30) #10	5,795	5,795	5,795	
	ST6236	00(14)	21/16	3313/16	(40) #10	(30) #10	(18) #10	3,535	3,760	3,760	
-	HRS6		13/8	6	(6) #10	(6) #10	(6) #10	530	790	1,600	1
	HRS8		13/8	8	(10) #10	(10) #10	(10) #10	885	1,315	2,670	1
-	HRS12		13%	12	(14) #10	(14) #10	(12) #10	1,235	1,840	2,710	1
┢	FHA6		1 7/16	6%	(14) #10	(14) #10	(8) #10	705	1,040	2,045	-
-	FHA12		1 7/16	11%	(8) #10	(8) #10	(8) #10	705	1,050	2,045	-
-	FHA18		1 7/16	173/4	(8) #10	(8) #10	(8) #10	705	1,050	2,045	-
-	FHA24		1 7/16	237/8	(8) #10	(8) #10	(8) #10	705	1,050	2,045	-
-	FHA30	. 97 (12)	1 7/16	30	(8) #10	(8) #10		705	1,050	2,045	-
-		97 (12)					(8) #10				-
┝	MSTI26 MSTI36		21/16	26 36	(26) #10	(26) #10	(22) #10	2,300	3,420	5,025 5,025	-
┝			21/16		(36) #10	(36) #10	(22) #10	3,180	4,735		-
┝	MSTI48		21/16	48	(48) #10	(40) #10	(22) #10	4,240	5,025	5,025	-
┝	MSTI60		21/16	60	(58) #10	(40) #10	(22) #10	5,025	5,025	5,025	
┝	MST27		21/16	27	(30) #10	(30) #10	(22) #10	2,650	3,945	5,025	-
┝	MST37		21/16	37	(42) #10	(40) #10	(34) #10	3,710	5,025	5,025	170
	MST48		21/16	48	(54) #10	(54) #10	(46) #10	4,770	5,155	5,155	170
	MST60	118 (10)	21/16	60	(68) #10	(68) #10	(62) #10	5,820	6,420	6,650	-
L	MST72		21⁄16	72	(80) #10	(72) #10	(64) #10	6,650	6,650	6,650	

These products are available with additional corrosion protection. Additional products on

this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. Use half of the fasteners in each member being connected to achieve the listed loads.

2. Loads are based on lesser of steel capacity or fastener calculation.

3. Not all fastener holes need to be filled, as additional fastener holes are provided. Install fasteners symmetrically.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

Lateral Connectors, Ties and Straps

CS/CMST Coiled Straps

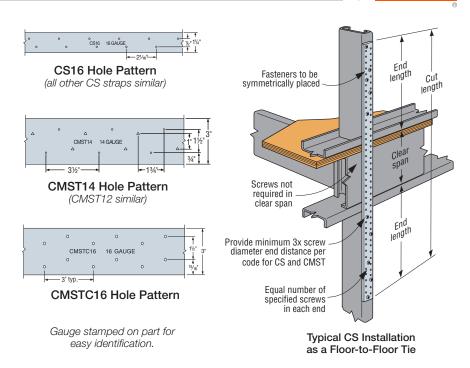
CMSTC provides countersunk fastener slots that provide a lower screw head profile. CS, CMST and CMSTC are continuous utility straps which can be cut to length on the job site. Packaged in lightweight cartons (about 40 lb.).

Finish: Galvanized. Some products available in ZMAX® coating; see Corrosion Information, pp. 18-21.

Installation:

- Use all specified fasteners; see General Notes.
- · Refer to the applicable code for minimum edge and end distances.
- The table shows the maximum allowable loads and the screws required to obtain them. See footnote #1. Fewer screws may be used; reduce the allowable load by the code lateral load for each fastener subtracted from each end.

Codes: See p. 11 for Code Reference Key Chart



x Table Load

		Connector		F	asteners ⁸ (Total)		Allowable Tension Load (lb.)	
Model No.	Total Length	Material Thickness	Width (in.)	Rafter	/Stud/Joist Thick	ness	33 mil (20 ga.)	Code Ref.
		mil (ga.)		33 mil (20 ga.)	43 mil (18 ga.)	54 mil (16 ga.)	43 mil (18 ga.) 54 mil (16 ga.)	
CMST12 ²	40'-3"	97 (12)	3	(104) #10	(70) #10	(40) #10	9,080	
CMST14 ²	52'-6"	68 (14)	3	(72) #10	(50) #10	(28) #10	6,365	
CMSTC16 ³	54'	54 (16)	3	(54) #10	(36) #10	(30) #10	4,600	
CS14	100'	68 (14)	11⁄4	(28) #10	(18) #10	(12) #10	2,305	
CS16	150'	54 (16)	11⁄4	(18) #10	(12) #10	(8) #10	1,550	IP1, L2, FL
CS18S	100'	40 (10)	11⁄4	(14) #10	(10) #10	(6) #10	1,235	
CS18	200'	43 (18)	11⁄4	(14) #10	(10) #10	(6) #10	1,235	
CS20	250'	33 (20)	11⁄4	(12) #10	(8) #10	(6) #10	945	
CS22	300'	27 (22)	11⁄4	(10) #10	(6) #10	(6) #10	775	

These products are available with additional corrosion protection. Additional products on

this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. Use half of the fasteners in each member being connected to achieve the listed loads.

2. For CMST straps: End Length (inches) = ½ total fasteners x 7/8" + 1" when all holes filled. Double length if only round holes filled.

3. For CMSTC16 straps: End Length (inches) = ½ total fasteners x ¾" + 1" when all holes filled. Double length if only round holes filled.

4. For CS straps: End Length (inches) = 1/2 total fasteners + 1".

5. Total Cut Length = End Length + Clear Span + End Length.

No. of Screws Used 6. Calculate the connector value for a reduced number of screws as follows: Allowable Load = No. of Screws in Table

24 Screws (Used) Example: CMSTC16 on 54 mil with 24 screws: x 4,600 lb. = 3,680 lb. 30 Screws (Table)

7. Loads are based on lesser of steel strap capacity and AISI S100 fastener calculation.

8. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

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280

Lateral Connectors, Ties and Straps

SIMPSO

Connectors for Cold-Formed Steel Construction

LTP5 Framing Anchor

The LTP5 framing anchor spans subfloor at the top of the blocking or rim joist. The embossments enhance performance and allow for design flexibility.

Material: 33 mil (20 ga.)

1. Allowable loads are for one anchor.

Finish: Galvanized

Installation:

• Use all specified fasteners; see General Notes

Codes: See p. 11 for Code Reference Key Chart

Model	Type of	Direction	Faste	eners ⁴	Allowable Load	Code	
No.	Connection	of Load To To Shea		To Sheathing and Track	43 mil (18 ga.) (lb.)	Ref.	
	1		(7) #10	(7) #10	1,045		
LTP5	2	G	(7) #10	(7) #10	1,110	IP1, L2, FL	
	3		(7) 8d x 1 ½"	(7) #10	730	,	

2. Allowable loads are based on steel (stud and sheet) of 43 mil (18 ga.) minimum.

4. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

3. Allowable load for Type 3 connection assumes $C_D = 1.60$.

41/2 ø SIMPSON Strong-Tie® Ø 0 51/8 Þ ø ø LTP5

Steel sheet 1/2" plywood 2 3 Steel sheet Wood rim CFS rim joist G G. 1/2" plywood Steel sheet Steel sheet

> Note: When attaching an LTP5 framing anchor over sheathing, the screws must penetrate and engage the steel framing. A minimum of three threads shall penetrate past the steel.

Lateral Connectors, Ties and Straps

joist

Lateral Systems



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CAUNUS CAUNUS

Strong Frame® Moment Frames



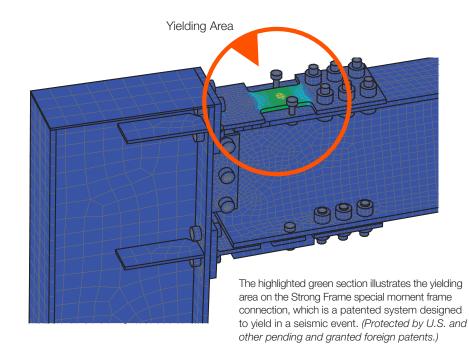
For years, moment frames have been a common method of providing high lateral-force resistance when limited wall space and large openings control the structural design. Traditionally, the disadvantage with moment frames has been that they are time-intensive to design and labor-intensive to install. Simpson Strong-Tie has taken these factors into consideration and has created a cost-effective alternative to traditional frames — the Strong Frame. For CFS applications, the Strong Frame is available without the pre-installed wood nailers.

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The "Special" Behind the Special Moment Frame

The new Strong Frame[®] Special Moment Frame provides high lateral-force resistance to seismic events. Our innovative Yield-Link[®] structural fuse is designed so the connection response remains ductile under load, providing more predictable performance. Little, if any, deformation is expected from the members.



Strong Frame® Moment Frames

Features:

- 100% Bolted Connections Install frames faster with no field welding required. No need to have a welder on site, or a welding inspector.
- Most Frames Fit in a Standard 2x6 or 2x8 Wall No thicker-wall additional framing or furring required.
- Greater Quality Control

Frames are manufactured in a quality-controlled environment and all specialty field-bolted connections are inspected in the factory.

- · Convenient to Store, Ship and Handle Disassembled frames are more compact, minimizing deliveries and simplifying handling on the jobsite. Assembled frames available.
- Preassembled Anchor-Bolt Assemblies Anchor bolts are preassembled on a shear-lug plate that mounts on the form. This helps ensures correct anchor placement and creates more efficient anchor performance.
- Streamlined Anchorage Design No more tedious anchorage calculations — select an anchorage solution for your footing geometry from the Strong Frame Selector Software.
- Multi-Story, Multi-Bay, and Retrofit Solutions Are Available

Contact Simpson Strong-Tie for more information.

Code Listed

SMF - ICC-ES ESR-2802; AISC-ANSI 358-16, Chapter 12



SIMPSON

Strong



Strong Frame[®] Design Guide

All of the information you need on our latest lateral force-resisting solution is contained in the Strong Frame Design Guide (F-L-SF). Frame and anchorage design information, installation instructions, prescriptive wall bracing requirements and installation details are all included in this useful tool. Visit strongtie.com to download or request a copy or call (800) 999-5099.



Strong Frame[®] Selector Software

The Simpson Strong-Tie® Strong Frame® Selector software is designed to help the Designer select an appropriate frame for their given geometry and loading. Only minimum inputs are required for the software to select an appropriate frame for the available space. Based on input geometry, the Strong Frame Selector software will narrow down from the more than 500 available stock frames to a handful of possible solutions. It can also help with custom frame designs. Download your free copy at strongtie.com/strongframe.

Lateral Systems

Connectors for Cold-Formed Steel Construction

Steel Strong-Wall® Shearwall



Working with specifiers, builders and contractors has given Simpson Strong-Tie insight into the needs of the various players in the design and construction process. This insight has enabled Simpson Strong-Tie to design a composite shearwall that features some of the highest allowable loads in the industry while offering the easiest and fastest installation: The Steel Strong-Wall shearwall.

Code Listed

New ICC-ES ESR-1679 code report evaluated to the 2015 IRC/IBC

Less Labor = Increased Production

Fewer anchor bolts and fasteners coupled with easy access to the top and bottom of the wall result in more efficient installation

Easier for All Trades

An easy-to-use anchor-bolt template for concrete contractors, available pre-attached CFS studs and predrilled holes where electricians need them for wiring

Support and Service

Simpson Strong-Tie provides the best engineering technical support and experienced field representation available

The Steel Strong-Wall product line has grown to address more applications:

- Standard installations on concrete
- Garage portal system
- Anchorage solutions
- · Raised floor solutions
- Two-story stacked shearwalls
- Cold-formed steel applications



Steel Strong-Wall® Cold-Formed Steel on Concrete Foundations

SIMPSON Strong-Tie

The Steel Strong-Wall provides high-capacity, Naming Scheme: narrow-wall solutions for cold-formed steel framing. S/SSW24x8X The wall installs easily in cold-formed steel framing, and pre-attached steel studs allow easy attachment Steel Strong-Wall Height Modification Option (in.) of interior and exterior finishes. Width for Cold-Formed Steel (in.) Material: Vertical Panel - 118 mil (10 ga.) Nominal Height (ft.) Finish: Vertical Panel - Galvanized Top and Base Plates - Simpson Strong-Tie® gray paint (cold galvanizing available, contact Simpson Strong-Tie) W Notes: For top-of-wall attachment, use 1/4" or #14 self-drilling screws (not provided) extended through For top of wall the connection with three exposed threads minimum. attachment, use Fill all screw holes. 1/4" or #14 selfdrilling screws Codes: ICC-ES ESR-1679; City of L.A. RR 25625; (not provided). ſ \int (See p. 293.) State of Florida FL5113 12111 Pre-attached Steel Strong-Wall® for 33 mil (20 ga.) steel studs to Cold-Formed Steel Product Data attach interior 0 0 and exterior Dimensions Anchor Number finishes Model Bolts of Screws (in.) in Top No. W Н Т Qty. Dia. of Wall Do Not S/SSW12x8 12 96% 31⁄2 2 3⁄4" 4 cut wall or Η S/SSW15x8 15 96% 3½ 2 1" 6 enlarge Prepunched holes with existing holes S/SSW18x8 18 96% 31⁄2 2 1" 9 0 0 grommets for wiring S/SSW21x8 21 96% 31/2 2 1" 12 ſ 0 . ٠ Place Steel S/SSW24x8 24 96% 31/2 2 1" 14 Strong-Wall panel over the anchor S/SSW12x9 12 108% 3½ 2 3⁄4" 4 bolts and secure 0 ſ with heavy hex nuts 1" S/SSW15x9 15 108% 31⁄2 2 6 (provided). Snug S/SSW18x9 108% 31/2 2 1" 9 18 tight fit required. . do not use an S/SSW21x9 21 108% 31⁄2 2 1" 12 impact wrench. Foundation 11/4" wrench/ S/SSW24x9 24 108% 31/2 2 1" 14 design socket required (size and for ¾" nut S/SSW15x10 15 120% 31⁄2 2 1" 6 0 reinforcement) 0 1%" wrench/ S/SSW18x10 1" 9 . . 18 120% 31/2 2 by Designer socket required for 1" nut S/SSW21x10 120% 31/2 2 12 21 1" S/SSW24x10 120% 2 1" 14 24 31/2 fi 1. S/SSW models may be ordered in custom heights. To order, add "X" to model and specify height ő. (example: S/SSW12x8X, H = 95"). 0000 , 0 0 ..;.00 . . . S/SSW24x10 S/SSW12x8 U.S. Patent 8,281,551 U.S. Patent 8,281,551 Canadian Patent 2,489,845 Canadian Patent 2,489,845 Wall Profiles \cap Ω S/SSW12 S/SSW15 S/SSW18

S/SSW21

S/SSW24

Steel Strong-Wall® Cold-Formed Steel on Concrete Foundations

2015 International Building Code®

			Seismic ²			Wind		
S/SSW Model	Max. H (in.)	Allowable Axial Load (lb.)	Allowable ASD Shear Load V (lb.)	Drift at Allowable Shear (in.)	Anchor Tension at Allowable Shear ⁶ (Ib.)	Allowable ASD Shear Load V (lb.)	Drift at Allowable Shear (in.)	Anchor Tension at Allowable Shear ⁶ (Ib.)
S/SSW12x8		1,000	645	0.42	7,710	820	0.54	10,360
	96%	4,000	645	0.42	7,710	775	0.51	9,640
		7,500	610	0.40	7,220	610	0.40	7,220
S/SSW15x8		1,000	1,280	0.42	12,390	1,415	0.47	14,090
	96%	4,000	1,250	0.41	12,025	1,250	0.41	12,025
		7,500	1,070	0.35	9,955	1,070	0.35	9,955
		1,000	2,140	0.41	16,895	2,785	0.54	24,565
S/SSW18x8	96%	4,000	2,140	0.41	16,895	2,680	0.52	23,130
		7,500	2,140	0.41	16,895	2,460	0.48	20,400
		1,000	3,265	0.41	21,905	3,870	0.48	27,930
S/SSW21x8	96%	4,000	3,265	0.41	21,905	3,765	0.47	26,790
		7,500	3,265	0.41	21,905	3,460	0.43	23,715
		1,000	4,540	0.39	26,335	4,985	0.43	30,045
S/SSW24x8	96%	4,000	4,540	0.39	26,335	4,890	0.42	29,220
		7,500	4,540	0.39	26,335	4,555	0.39	26,455
		1,000	545	0.48	7,255	695	0.61	9,735
S/SSW12x9	108%	4,000	545	0.48	7,255	605	0.53	8,210
		7,500	445	0.39	5,755	445	0.39	5,755
		1,000	1,090	0.48	11,725	1,180	0.52	12,955
S/SSW15x9	108%	4,000	1,025	0.45	10,875	1,025	0.45	10,875
		7,500	850	0.37	8,720	850	0.37	8,720
		1,000	1,835	0.47	16,105	2,365	0.61	22,835
S/SSW18x9	108%	4,000	1,835	0.47	16,105	2,365	0.61	22,835
		7,500	1,835	0.47	16,105	2,150	0.55	19,890
	1085%	1,000	2,800	0.46	20,855	3,275	0.54	25,900
S/SSW21x9		4,000	2,800	0.46	20,855	3,025	0.50	23,140
		7,500	2,735	0.45	20,220	2,735	0.45	20,220
	108%	1,000	4,005	0.46	26,025	4,220	0.48	27,970
S/SSW24x9		4,000	3,950	0.45	25,540	3,950	0.45	25,540
		7,500	3,630	0.41	22,855	3,630	0.41	22855
	120%	1,000	945	0.53	11,185	990	0.56	11,845
S/SSW15x10		4,000	835	0.47	9,645	835	0.47	9,645
		7,500	665	0.37	7,425	665	0.37	7,425
	120%	1,000	1,605	0.53	15,515	2,045	0.67	21490
S/SSW18x10		4,000	1,605	0.53	15,515	1,960	0.64	20,225
		7,500	1,605	0.53	15,515	1,715	0.56	16,890
	120%	1,000	2,440	0.52	19,970	2,650	0.56	22,275
S/SSW21x10		4,000	2,405	0.51	19,600	2,405	0.51	19,600
		7,500	2,120	0.45	16,730	2,120	0.45	16,730
		1,000	3,425	0.50	24,275	3,425	0.50	24,275
S/SSW24x10	120%	4,000	3,160	0.46	21,875	3,160	0.46	21,875
		7,500	2,855	0.42	19,275	2,855	0.42	19,275
	1		· · ·	1	· · ·	· · ·		

NOTE:

For models with an "X" suffix, specify height when ordering (example: S/SSW12x8X, h = 95").

SIMPSON

Strong-T

- 1. Allowable shear loads and anchor tension forces are applicable to installation on concrete with minimum $f_c = 2,500$ psi using the ASD basic (Section 1605.3.1) or the alternative basic (Section 1605.3.2) load combinations. Load values include evaluation of bearing stresses.
- For seismic designs based on the 2015 IBC using R = 6.5. For other codes, use the seismic coefficients corresponding to light-frame bearing walls with wood structural panels or sheet steel panels.
- 3. Top-of-wall screws for the S/SSW shall be approved 1/4" or #14 self-drilling screws (see p. 142) with a minimum nominal shear strength (Pss) of 2,000 lb. Top of panel shall be connected to a minimum 43 mil (18 ga.) thick steel member typical. S/SSW18 and wider panels up to 97 inches tall require connection to a minimum 54 mil (16 ga.) thick steel member. When connected to a minimum 43 mil (18 ga.) thick steel member, the allowable load shall be limited to 2,720 lb. for S/SSW18, 3,625 lb. for S/SSW21, and 4,230 lb. for S/SSW24.
- Allowable shear, drift and anchor tension values may be interpolated for intermediate height or axial loads. See example on p. 288.
- High-strength anchor bolts are required for anchor tension forces exceeding the allowable load for standard-strength bolts tabulated on pp. 296–297.
 High-strength anchor bolts are required for S/SSW12 when seismic shear X panel height exceeds 61.6k-in. See pp. 296–303. for SSWAB anchor bolt information and anchorage solutions.
- 6. Tabulated anchor tension loads assume no resisting axial load. For anchor tension loads at design shear values and including the effect of axial load, refer to the Strong-Wall SelectorTM software or use the equations on p. 289. Drifts at lower design shear may be linearly reduced.
- 7. See p. 288 for allowable outof-plane loads and axial loads.

Lateral Systems

Steel Strong-Wall® Cold-Formed Steel on Concrete Foundations

SIMPSON Strong-Tie

Allowable Out-of-Plane Loads (PSF)^{1,3}

Model	Axial Load	Nominal Height of Panel (ft.)				
Width	(lb.) ^{2,4}	8	9	10		
	1,000	195	140	100		
12" wide	4,000	145	100	70		
	7,500	85	50	25		
15" wide	1,000	160	125	100		
	4,000	130	95	70		
	7,500	90	65	45		
18" wide	7,500	300	210	155		
21" wide	7,500	255	180	130		
24" wide	7,500	265	190	135		

1. Loads shown are at ASD level in pounds per square foot (psf) of wall with no further increase allowed and are applicable to either the ASD Basic or Alternative Basic load combinations.

2. Axial load denotes maximum gravity load permitted on entire panel acting in combination with the out-of-plane load.

3. Load considers a deflection limit of h/240.

4. Allowable out-of-plane loads for the 12- and 15-inch walls may be linearly interpolated between the axial loads shown.

Allowable Axial Loads on Concrete

Model	Compression Capacity (Ib.) with No Lateral Load ^{1,2,3}						
Width	Nominal Height of Panel (ft.)						
	7	8	9	10			
12" wide	20,200	16,300	13,700	11,100			
15" wide	25,300	21,800	19,200	16,600			
18" wide	42,500	36,000	31,400	27,000			
21" wide	43,700	35,800	30,300	25,100			
24" wide	51,600	42,900	36,900	31,100			

1. Allowable compression is lesser of wall-buckling capacity or 2,500 psi uniform concrete bearing.

 Allowable compression of wall assumes concentric loading with no lateral loads present. See allowable in-plane or out-of-plane shear load tables for combined lateral and axial loading conditions.

3. Loads are applicable to either the ASD Basic or Alternative Basic load combinations.

S/SSW Shear Load Interpolation Example

Given:

Seismic, 2,500 psi Concrete Shear Load = 2,000 lb. Axial = 4,000 lb. S/SSW Wall Height Required: 8'-6" = 102"

Interpolate (See table on p. 287):

$$\begin{split} & \text{S/SSW18x8} \quad V_1 = 2,140 \text{ lb.}, \ h_1 = 96\%'' \\ & \text{S/SSW18x9} \quad V_2 = 1,835 \text{ lb.}, \ h_2 = 108\%'' \\ & \text{Equation:} \quad V_{\text{allow}} = \left(\frac{V_1 - V_2}{h_1 - h_2}\right) (h_{\text{required}} - h_1) + V_1 \\ & \text{V}_{\text{allow}} \quad \left(\frac{2,140 \text{ lb.} - 1,835 \text{ lb.}}{96\%'' - 108\%''}\right) (102'' - 96\%'') + 2,140 \text{ lb.} = 2,003 \text{ lb.} \ \text{@} \ 102'' \\ \end{split}$$

 $V_{allow}\;$ = 2,003 lb. > 2,000 lb. OK

Use S/SSW18x9X H = 102"

Steel Strong-Wall® Uplift Equations

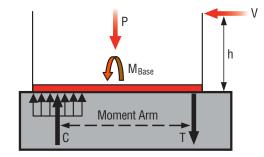
(Based on limiting concrete bearing on a 31/2"-wide base plate at the edge of the concrete)

These equations may be used to calculate uplift forces at the base of the 1st-story wall to aid Designers in developing anchorage solutions other than those shown on pp. 296–300.

12 in. wall	$T = \left[11.2f_c' - \sqrt{126f_c'^2 - 2.38f_c'(3.44P + Vh)}\right] - P$
15 in. wall	$T = \left[14.4f_c' - \sqrt{208f_c'^2 - 2.38f_c'(4.63P + Vh)}\right] - P$
18 in. wall	$T = \left[18.0f_c' - \sqrt{324f_c'^2 - 2.38f_c'(6.13P + Vh)}\right] - P$
21 in. wall	$T = \left[21.6f_c' - \sqrt{465f_c'^2 - 2.38f_c'(7.63P + Vh)}\right] - P$
24 in. wall	$T = \left[25.1f'_c - \sqrt{632f'_c^2 - 2.38f'_c(9.13P + Vh)}\right] - P$

Notes:

- 1. Equations may be used to calculate uplift forces at the base of first-story walls on concrete foundations.
- Equations are based on the design methodology contained in AISC Steel Design Guide 1 – Base Plate and Anchor Rod Design, second edition using a rectangular compression stress block.



Forces at Base of Wall

- T = Resulting anchorage tension (uplift) force (kips)
- V = Design shear (kips)
- P = Total vertical load (kips)
- h = Wall height (inches)
- f'c = Concrete compressive strength (ksi)

For two-story stacked applications, substitute M_{base} for V_h:

$$Vh = M_{base} \left(\frac{12}{1,000}\right) kip-in$$

Where M_{base} = Design moment at base of wall (ft.-lb.)

EXAMPLE 1 – Single-Story S/SSW

Given:

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

- S/SSW18x9X wall on 2.5 ksi concrete
- Seismic
- Design Shear (V) = 1.5 kips < 1.835 kips (Vallowable)
- P (Vertical Load) = 1.0 kip
- h = Wall height = 109"

 $T = \left[18.0 \text{ f'c} - \sqrt{324 \text{ f'c}^2 - 2.38 \text{ f'c} (6.13P + Vh)} \right] - P$

 $T = \left[18.0 (2.5) - \sqrt{324 (2.5)^2 - 2.38 (2.5) (6.13 \times 1 + 1.5 \times 109)}\right] - 1.0 = \underline{12.1 \text{ kips}}$

EXAMPLE 2 – Two-Story Stacked S/SSW Condition Given:

- See Two-Story Design Example on p. 295
- S/SSW18x9X-STK wall on 2.5 ksi concrete
- Wind
- $M_{base} = 17,550$ ft.-lb. (Moment at base of two-story stacked wall)

• Vh = 17,550 x
$$\left(\frac{12}{1,000}\right)$$
 kip-in. = 210.6 kip-in.

- P (Vertical Load) = 2.0 kips
- $\mathbf{T} = \left[18.0 \text{ f'c} \sqrt{324 \text{ f'c}^2 2.38 \text{ f'c} (6.13\text{ P} + \text{Vh})} \right] \mathbf{P}$
- $T = \left[18.0 (2.5) \sqrt{324 (2.5)^2 2.38 (2.5) (6.13 \times 2 + 210.6)} \right] 2 = \underline{16.6 \text{ kips}}$

Steel Strong-Wall® Cold-Formed Steel 1st-Story Floor Systems

SIMPSON Strong-

Quantity

#10 Screws

6

10

12

16

18

For a complete set of wall profile

Shear-Transfer Plate

Floor Applications

Fasteners for Raised-

Fastener

#14 Screws

4

4

6

6

7

drawings, see p. 286.

Strong-Wall

Width

12" wall

15" wall

18" wall

21" wall

24" wall

SSW Shear-Transfer Plate

 \cap

DO NOT

cut wall or enlarge

existing holes

Steel Strong-Wall panels designed for use on concrete foundations can be used with cold-formed steel floor systems by extending the anchor bolts and installing compression nuts and stud blocking below the wall.

Material and Finish: See p. 286

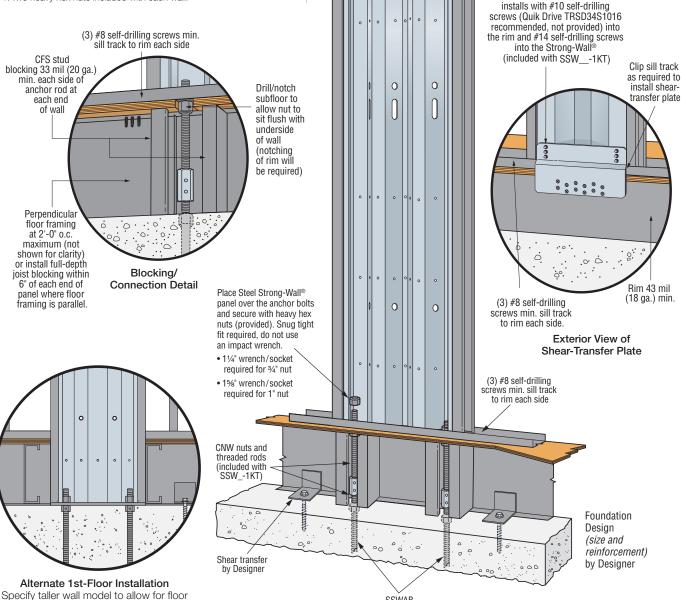
For product data and naming scheme information, see p. 286.

CFS First-Floor Wall Connection Kit

Wall Width (in.)	Model No.	Contents
12	SSW12-1KT	(1) Shear-transfer plate
15	SSW15-1KT	(with #14 self-drilling screws) (2) ³ ⁄4" or 1" x 18" threaded rods
18	SSW18-1KT	F1554 Grade 36
21	SSW21-1KT	(2) Coupler nuts (2) Heavy hex nuts
24	SSW24-1KT	Installation instructions

1. Two heavy hex nuts included with each wall.

on concrete; see pp. 286-287.



SSWAB framing and use load values for installation Cold-Formed Steel 1st-Story Floor System U.S. Patent 8,281,551; Canadian Patent 2,489,845

Steel Strong-Wall® Cold-Formed Steel 1st-Story Floor Systems

2015 International Building Code®

		Seismic ²		Wind			
S/SSW Model	Allowable ASD Shear Load V (Ib.)	Drift at Allowable Shear (in.)	Anchor Tension at Allowable Shear⁴ (lb.)	Allowable ASD Shear Load V (Ib.)	Drift at Allowable Shear (in.)	Anchor Tension at Allowable Shear ⁴ (lb.)	
S/SSW12x8	435	0.40	6,135	435	0.40	6,135	
S/SSW15x8	1,050	0.42	11,010	1,150	0.46	12,060	
S/SSW18x8	1,525	0.36	12,075	1,525	0.36	12,075	
S/SSW21x8	1,900	0.29	12,085	1,900	0.29	12,085	
S/SSW24x8	2,270	0.24	12,065	2,270	0.24	12,065	
S/SSW12x9	390	0.47	6,185	390	0.47	6,185	
S/SSW15x9	900	0.48	10,605	1,025	0.54	12,080	
S/SSW18x9	1,355	0.42	12,055	1,355	0.42	12,055	
S/SSW21x9	1,690	0.34	12,080	1,690	0.34	12,080	
S/SSW24x9	2,020	0.28	12,065	2,020	0.28	12,065	
S/SSW15x10	785	0.53	10,270	925	0.63	12,100	
S/SSW18x10	1,220	0.48	12,050	1,220	0.48	12,050	
S/SSW21x10	1,520	0.39	12,060	1,520	0.39	12,060	
S/SSW24x10	1,820	0.32	12,065	1,820	0.32	12,065	

1. Loads are applicable to 1st-Story Cold-Formed Steel Raised-Floor installations supported on concrete or masonry foundations using the ASD basic (Section 1605.3.1) or the alternative basic (Section 1605.3.2) load combinations. Load values include evaluation of anchor rod compression capacity and do not require further evaluation by the Designer.

2. For seismic designs based on the 2015 IBC using R = 6.5. For other codes, use the seismic coefficients corresponding to light-frame bearing walls with wood structural panels or sheet steel panels.

3. Minimum standard-strength anchor bolts required. See pp. 296–303 for SSWAB anchor bolt information and anchorage solutions. Tabulated anchor tension loads assume no resisting axial load. Anchor rod tension at design shear load and including the effect of axial load may be determined using the Strong-Wall Selector™ software or the following equation:

 $T = [(V \times h) / B] - P/2$, where: T = Anchor rod tension load (lb.)

V = design shear load (lb.)

- h = Strong-Wall® height per page 146 (in.)
- P = applied axial load (lb.)
- B = Anchor bolt centerline dimension (in.)
 - (61/8" for S/SSW12, 91/4" for S/SSW15, 121/4" for S/SSW18,
- 151/4" for S/SSW21, and 181/4" for S/SSW24)
- 4. Allowable shear loads assume a maximum first-floor joist depth of 12".
- 5. Allowable shear loads are based on 1,000 lb. total uniformly distributed axial load acting on the entire panel in combination with

the shear load. For allowable shear loads at 2,000 lb. uniformly distributed axial load, multiply table values by 0.92 for S/SSW12x models, and 0.96 for other S/SSW widths.

6. Top-of-wall screws for the S/SSW shall be approved ¼" or #14 self-drilling screws with a minimum nominal shear strength (Pss) of 2,000 lb. Top of panel shall be connected to a minimum 43 mil (18 ga.) thick steel member typical.

SIMPSO

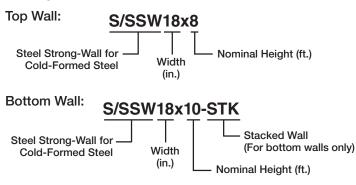
Strong

Steel Strong-Wall® Cold-Formed Steel Two-Story Stacked on Concrete Foundation

A complete stacked-wall solution for two-story applications. The Steel Strong-Wall option for two-story cold-formed steel installations combines simplified installation with superior performance.

- Some of the highest loads in the industry, and design procedures that account for cumulative overturning
- Complete concrete anchorage designs for two-story applications (foundation design by Designer)
- No bearing plates to install. Walls can be placed flush against a corner
- Same anchor bolt template as single-story installation
- Compression loads transferred by nut/rod
- Material and Finish: See p. 286

Naming Scheme:



Cold-Formed Steel Two-Story Stacked-Wall Product Data — Bottom Wall

Model No.	Din	nensions ((in.)	Ancho	Number of Screws	
Mouel No.	w	Н	т	Qty.	Dia.	in Top of Wall
S/SSW15x8-STK	15	96%	31⁄2	2	1"	6
S/SSW18x8-STK	18	96%	31⁄2	2	1"	9
S/SSW21x8-STK	21	96%	31⁄2	2	1"	12
S/SSW24x8-STK	24	96%	31⁄2	2	1"	14
S/SSW15x9-STK	15	108%	31⁄2	2	1"	6
S/SSW18x9-STK	18	108%	31⁄2	2	1"	9
S/SSW21x9-STK	21	108%	31⁄2	2	1"	12
S/SSW24x9-STK	24	108%	31⁄2	2	1"	14
S/SSW15x10-STK	15	120%	31⁄2	2	1"	6
S/SSW18x10-STK	18	120%	31⁄2	2	1"	9
S/SSW21x10-STK	21	120%	31⁄2	2	1"	12
S/SSW24x10-STK	24	120%	31⁄2	2	1"	14

Specify height when ordering "X-STK" models.
 See p. 286 for product data on top wall.

Two-Story Stacked-Wall Connection Kit

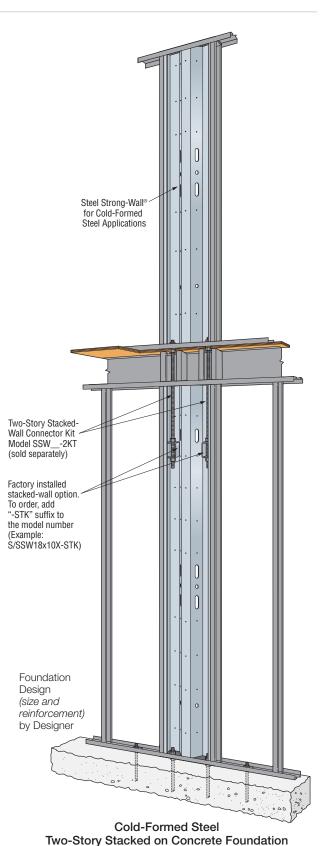
Wall Width (in.)	Model No.	Contents
15	SSW15-2KT	(1) Shear-transfer plate (with
18	SSW18-2KT	#14 self-drilling screws, included) (2) 1" x 48" threaded rods F1554 Grade 36
21	SSW21-2KT	(6) Heavy hex nuts
24	SSW24-2KT	Installation instructions

1. Two heavy hex nuts included with each wall.





For a complete set of wall profile drawings, see p. 286.



U.S. Patents 8,281,551 and 8,689,518; Canadian Patent 2,489,845

Connectors for Cold-Formed Steel Construction

Steel Strong-Wall[®] Cold-Formed Steel Two-Story Stacked on Concrete Foundation

Attach to top track with 1/4" or #14

self-drilling screws (not provided)

SIMPSON

Strong-Tie

#10 Screws

10

12

16

Shear-Transfer Plate Fasteners

#14 Screws

4

6

6

Strong-Wall

Width

15" wall

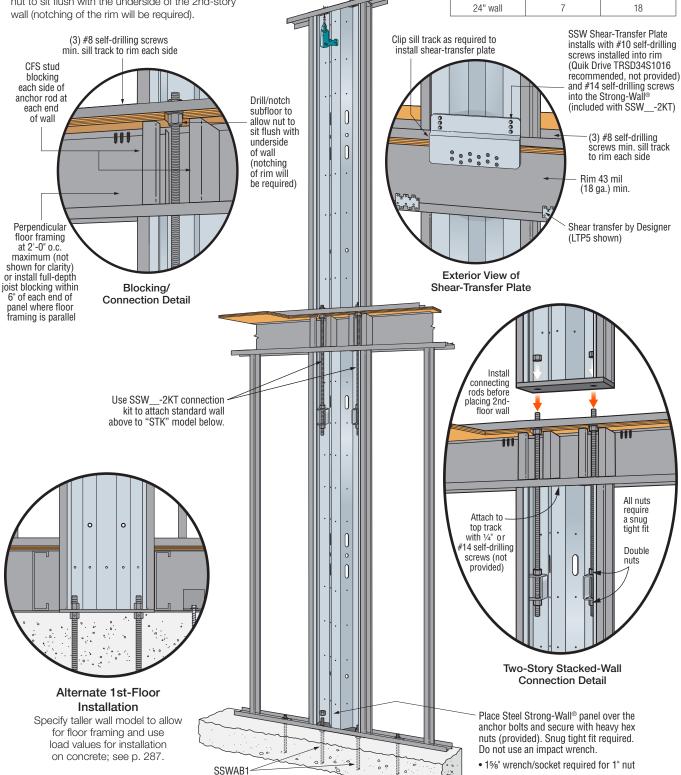
18" wall

21" wall

Fastener Quantity

Installation

- Do not cut the Steel Strong-Wall or enlarge existing holes. Doing so will compromise the performance of the wall.
- Do not use an impact wrench to tighten nuts on the anchor bolts.
- Drill or notch the subfloor to allow the compression nut to sit flush with the underside of the 2nd-story wall (notching of the rim will be required).



Cold-Formed Steel Two-Story Stacked on Concrete Foundation U.S. Patents 8,281,551 and 8,689,518; Canadian Patent 2,489,845

Steel Strong-Wall[®] Cold-Formed Steel Two-Story Stacked on Concrete Foundation

SIMPSON Strong-Tie

Second-Story Walls 6,8

	Seis	mic ²	Wi	nd
Second-Story Wall Models	Allowable ASD Shear Load V (Ib.)	Drift at Allowable Shear (in.)	Allowable ASD Shear Load V (lb.)	Drift at Allowable Shear (in.)
S/SSW15x8	550	0.29	550	0.29
S/SSW18x8	995	0.31	1,275	0.39
S/SSW21x8	1,515	0.30	1,650	0.33
S/SSW24x8	1,970	0.27	1,970	0.27
S/SSW15x9	510	0.35	510	0.35
S/SSW18x9	940	0.38	1,180	0.47
S/SSW21x9	1,435	0.37	1,465	0.38
S/SSW24x9	1,755	0.31	1,755	0.31
S/SSW15x10	475	0.41	475	0.41
S/SSW18x10	890	0.45	1,060	0.54
S/SSW21x10	1,300	0.42	1,300	0.42
S/SSW24x10	1,580	0.36	1,580	0.36

1. Allowable base moment and anchor tension forces are applicable to installation on concrete foundations with minimum $f_c = 2,500$ psi using the ASD basic (Section 1605.3.1) or the alternative basic (Section 1605.3.2) load combinations. Load values include evaluation of anchor rod compression at second story and bearing stresses at foundation.

- 2. For seismic designs based on the 2015 IBC using R = 6.5. For other codes, use the seismic coefficients corresponding to light-frame bearing walls with wood structural panels or sheet steel panels.
- 3. Two-Story Stacked-Wall installations may consist of any height combination of equal width wall models listed in these tables.
- 4. Loads are based on a 1,000 lb. maximum uniformly distributed total axial load acting on the second-story panel and a 2,000 lb. maximum uniformly distributed total axial load acting on the first-story panel in combination with the tabulated shear load and base moment.
- The designer must verify that the cumulative overturning moment at the base of the first-story Steel Strong-Wall does not exceed the allowable base moment capacity. See design example on p. 295 for procedure.
- 6. The allowable second-story shear loads assume a maximum floor joist depth of 14".

- Allowable shear, drift and base moment values may be interpolated for intermediate heights.
- 8. Minimum ASTM F 1554 Grade 36 threaded rods are required at the second-story wall anchorage.
- High-strength anchor bolts are required at the first-story wall for anchor tension forces exceeding the allowable load for standard strength bolts tabulated on pp. 296–297. See pp. 296–303 for SSWAB anchor bolt information and anchorage solutions.
- 10. Tabulated anchor tension loads assume no resisting axial load. For anchor tension loads at design shear values and including the effect of axial load, refer to the Strong-Wall Selector software or use the equations on p. 289. Drifts at lower design shear or base moment may be linearly reduced.
- 11. Top-of-wall screws for the S/SSW shall be approved ¼" or #14 self-drilling screws with a minimum nominal shear strength (P_{ss}) of 2,000 lb. Top of panel shall be connected to a minimum 43 mil (18 ga.) thick steel member typical. First-story S/SSW 24x8-STK requires connection to a minimum 54 mil (16 ga.) thick steel member where the total applied shear load exceeds 4,230 lb.

First-Story Walls on Concrete Foundations 5,9,11

		Seismic ²			Wind	
First-Story Wall Models	Allowable ASD Base Moment (ftlb.)	Drift at Allowable Base Moment (in.)	Anchor Tension at Allowable Base Moment ¹⁰ (lb.)	Allowable ASD Base Moment (ftlb.)	Drift at Allowable Base Moment (in.)	Anchor Tension at Allowable Base Moment ¹⁰ (lb.)
S/SSW15x8-STK	10,130	0.41	12,065	10,130	0.41	12,065
S/SSW18x8-STK	17,300	0.41	16,895	22,230	0.53	24,075
S/SSW21x8-STK	26,390	0.41	21,905	31,000	0.48	27,545
S/SSW24x8-STK	36,700	0.39	26,335	40,040	0.43	29,770
S/SSW15x9-STK	9,705	0.47	11,440	9,945	0.48	11,790
S/SSW18x9-STK	16,670	0.47	16,110	21,480	0.61	22,835
S/SSW21x9-STK	25,435	0.46	20,855	28,990	0.53	24,950
S/SSW24x9-STK	36,210	0.45	25,860	37,515	0.47	27,145
S/SSW15x10-STK	9,160	0.51	10,665	9,460	0.53	11,090
S/SSW18x10-STK	16,185	0.53	15,515	20,335	0.66	21,060
S/SSW21x10-STK	24,485	0.52	19,845	25,895	0.55	21,355
S/SSW24x10-STK	33,645	0.49	23,460	33,645	0.49	23,460

Steel Strong-Wall® Cold-Formed Steel Two-Story Stacked on Concrete Foundation

Strong⁻

Steel Strong-Wall Two-Story Design Example

Example: Cold-Formed Steel Two-Story Wall Design

Given:

Wind, $f^{\prime}_{\text{C}}=2,500~\text{psi}$

V_{2nd-story wall} = 650 lb.

V_{1st-story wall} = 650 lb.

- $V_{total} = 650 \text{ lb.} + 650 \text{ lb.} = 1,300 \text{ lb.}$
- M_{allow} = Allowable ASD Base Moment (ft.-lb.) (See Cold-Formed Steel Two-Story Stacked tables)
- V_{allow} = Allowable ASD Shear Load V (lb.) (See Cold-Formed Steel Two-Story Stacked tables)

Step 1 - Select First-Story Wall (See tables on p. 294)

M_{base} = (650 lb. x 18 ft.) + (650 lb. x 9 ft.) = 17,550 ft.-lb.

Using First-Story Wall table, select a 9' wall with $M_{allow} \geq M_{base}$

Select S/SSW18x9-STK

 $M_{allow} = 21,480 \text{ ft.-lb.} > 17,550 \text{ ft.-lb.} OK$

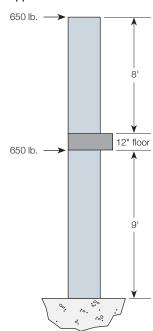
Step 2 - Check Second-Story Wall

Using the Second-Story Wall Table on p. 294, check the capacity of an 8' wall with the same width as the First-Story Wall selected in Step 1:

Select S/SSW18x8

 $V_{allow}\ =$ 1,275 lb. > 650 lb. \mbox{OK}

Use S/SSW18x8 over S/SSW18x9-STK



Applied Loads

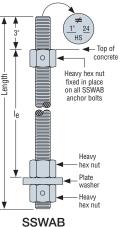
Top of concrete

SSWAB Anchor Bolts

SSWAB anchor bolts in ¾" and 1" diameters offer flexibility to meet specific project demands. Inspection is easy; the head is stamped with a "No Equal" symbol for identification, bolt length, bolt diameter, and optional "HS" for High Strength if specified.

Material: ASTM F1554 Grade 36; High-strength (HS) ASTM A449

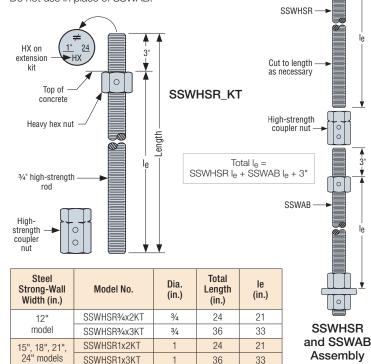
An additional nut for template installation is provided with each SSWAB. It may also be used for SSW installation.



Steel Strong-Wall Width (in.)	trong-Wall Model Width No.		Total Length (in.)	le (in.)
	SSWAB¾x24	3⁄4	24	19
10"	SSWAB¾x24HS	3⁄4	24	19
12" model	SSWAB¾x30	3⁄4	30	25
model	SSWAB¾x30HS	3⁄4	30	25
	SSWAB¾x36HS	3⁄4	36	31
	SSWAB1x24	1	24	19
	SSWAB1x24HS	1	24	19
15", 18", 21", 24" models	SSWAB1x30	1	30	25
24 1100013	SSWAB1x30HS	1	30	25
	SSWAB1x36HS	1	36	31

SSWHSR Extension Kit

SSWHSR allows for anchorage in tall stemwall applications where full embedment of an SSWAB into the footing is required. The head is stamped for identification like an SSWAB. Kit includes ASTM A449 high-strength rod with heavy hex nut fixed in place and high-strength coupler nut. Do not use in place of SSWAB.



Steel Strong-Wall Anchorage Solutions - 2,500 psi Concrete^{1,2,6}

Design	Concrete	Anchor	SSWAB 3/	4" Anchor E	Bolt	SSWAB 1	" Anchor B	olt
Criteria Condition		Strength ³	ASD Allowable Tension (lb.)	W (in.)	d _e (in.)	ASD Allowable Tension (lb.)	W (in.)	d _e (in.)
		Standard	8,800	22	8	16,100	33	11
	Cracked	Stanuaru	9,600	24	8	17,100	35	12
	GIACKEU	High strongth	18,500	36	12	33,000	51	17
Seismic ⁴		High strength	19,900	38	13	35,300	54	18
Seisittic		Standard	8,800	19	7	15,700	28	10
	Uncracked	Standard	9,600	21	7	17,100	30	10
	Uncrackeu	High strength	18,300	31	11	32,300	44	15
		Thigh strength	19,900	33	11	35,300	47	16
			5,100	14	6	6,200	16	6
		Standard	7,400	18	6	11,400	24	8
			9,600	22	8	17,100	32	11
	Cracked		11,400	24	8	21,100	36	12
		High strength	13,600	27	9	27,300	42	14
		High strength	15,900	30	10	31,800	46	16
Wind ⁵			19,900	35	12	35,300	50	17
WILL			5,000	12	6	6,400	14	6
		Standard	7,800	16	6	12,500	22	8
			9,600	19	7	17,100	28	10
	Uncracked		12,500	22	8	21,900	32	11
		High strength	14,300	24	8	26,400	36	12
		riigii siteliyiii	17,000	27	9	31,500	40	14
			19,900	30	10	35,300	43	15

1. See pp. 299–300 for foundation illustrations showing W and d_e dimensions.

- Anchorage designs conform to ACI 318-14 with no supplement edge reinforcement and cracked or uncracked concrete as noted.
 Anchor strength indicates required grade of SSWAB anchor bolt. Standard or high-strength (HS).
- Seismic indicates Seismic Design Category C through F. Detached one- and two-family dwellings in SDC C may use wind anchorage solutions. Seismic anchorage designs conform to ACI 318-14 Section 17.2.3.4.3.
- 5. Wind includes Seismic Design Category A&B.
- Foundation dimensions are for anchorage only. Foundation design (size and reinforcement) by Designer. The registered design professional may specify alternate embedment, footing size or anchor bolt.

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Steel Strong-Wall® Anchorage Solutions

Steel Strong-Wall Anchorage Solutions - 3,500 psi Concrete^{1,2,6}

	<u>_</u>	I							
Design	Concrete	Anchor	SSWAB 3	4" Anchor I	Bolt	SSWAB	1" Anchor E	Bolt	
Design Criteria	Condition	Strength ³	ASD Allowable Tension (lb.)	W (in.)	d _e (in.)	ASD Allowable Tension (lb.)	W (in.)	d _e (in.)	
		Standard	9,000	20	7	15,700	29	10	
	Cracked	Standard	9,600	21	7	17,100	31	11	
	Grackeu	High strength	18,200	32	11	32,900	46	16	
Seismic ⁴			19,900	34	12	35,300	48	16	
Seisinic		Standard	8,800	17	6	15,700	25	9	
	Uncracked	Stanuaru	9,600	19	7	17,100	27	9	
	UNCIALKEU	High strength	18,600	28	10	32,600	40	14	
		High strength	19,900	30	10	35,300	42	14	
		Standard	6,000	14	6	7,300	16	6	
			7,300	16	6	13,500	24	8	
			9,600	20	7	17,100	29	10	
	Cracked		11,800	22	8	22,700	34	12	
		High strength	13,500	24	8	27,400	38	13	
		riigii suengui	17,000	28	10	32,300	42	14	
Wind⁵			19,900	32	11	35,300	45	15	
WITIG			6,000	12	6	7,500	14	6	
		Standard	7,500	14	6	12,800	20	7	
			9,600	17	6	17,100	25	9	
	Uncracked		12,800	20	7	21,300	28	10	
		High strength	14,800	22	8	26,000	32	11	
		nigh ou ongui	16,900	24	8	31,300	36	12	
			19,900	27	9	35,300	39	13	

- 1. See pp. 299-300 for foundation illustrations showing W and de dimensions.
- 2. Anchorage designs conform to ACI 318-14 with no supplement edge reinforcement and cracked or uncracked concrete as noted.
- 3. Anchor strength indicates required grade of SSWAB anchor bolt. Standard or high-strength (HS).

4. Seismic indicates Seismic Design Category C through F. Detached one- and two-family dwellings in SDC C may use wind anchorage solutions. Seismic anchorage designs conform to ACI 318-14 Section 17.2.3.4.3.

- 5. Wind includes Seismic Design Category A&B.
- 6. Foundation dimensions are for anchorage only. Foundation design (size and reinforcement) by Designer. The registered design professional may specify alternate embedment, footing size or anchor bolt.

Steel Strong-Wall Anchorage Solutions — 4,500 psi Concrete^{1,2,6}

	_		SSWAB 3	4" Anchor I	Bolt	SSWAB	1" Anchor E	Bolt
Design Criteria	Concrete Condition	Anchor Strength ³	ASD Allowable Tension (lb.)	W (in.)	de (in.)	ASD Allowable Tension (lb.)	W (in.)	de (in.)
		Ctandard	8,700	18	6	16,000	27	9
	Cracked	Standard	9,600	20	7	17,100	29	10
	Grackeu	High strength	17,800	29	10	32,100	42	14
Seismic ⁴			19,900	32	11	35,300	45	15
Seismic		Standard	9,100	16	6	15,700	23	8
	Uncracked	Stanuaru	9,600	17	6	17,100	25	9
	UTICIACKEU	High strength	17,800	25	9	32,500	37	13
			19,900	27	9	35,300	39	13
		Standard	5,400	12	6	6,800	14	6
			8,300	16	6	11,600	20	7
			9,600	18	6	17,100	26	9
	Cracked		11,600	20	7	21,400	30	10
		High strength	13,400	22	8	25,800	34	12
		riigii su eiigui	17,300	26	9	31,000	38	13
Wind⁵			19,900	29	10	35,300	42	14
vvii iu-			6,800	12	6	6,800	12	6
		Standard	8,500	14	6	12,400	18	6
			9,600	16	6	17,100	23	8
	Uncracked		12,400	18	6	21,600	26	9
		High strength	14,500	20	7	26,700	30	10
		riigii strengtri	16,800	22	8	32,200	34	12
			19,900	25	9	35,300	36	12

- 1. See pp. 299-300 for foundation illustrations showing W and de dimensions.
- 2. Anchorage designs conform to ACI 318-14 with no supplement edge reinforcement and cracked or uncracked concrete as noted.
- 3. Anchor strength indicates required grade of SSWAB anchor bolt. Standard or high-strength (HS).
- 4. Seismic indicates Seismic Design Category C through F. Detached one- and two-family dwellings in SDC C may use wind anchorage solutions. Seismic anchorage designs conform to ACI 318-14 Section 17.2.3.4.3.
- 5. Wind includes Seismic Design Category A&B.
- 6. Foundation dimensions are for anchorage only. Foundation design (size and reinforcement) by Designer. The registered design professional may specify alternate embedment, footing size or anchor bolt.



Steel Strong-Wall Shear Anchorage

Foundation shear reinforcement to resist shear forces from Strong-Wall[®] panels located at the edge of concrete is shown in the table below. The S/SSW12 and S/SSW15 used in wind applications do not require shear reinforcement when the panel design shear force is less than the anchorage allowable shear load shown in the table below.

		Seis	mic ³			Wind	4		
Model No.	L _t or L _h		Minimum Curb/Stemwall	Shear Reinforcement	inforcement Width	ASD Allowable Shear Load V ⁶ (lb.)			
NU.	(in.)	Reinforcement	Width			6" Minimum C	urb/Stemwall	8" Minimum Curb/Stemwall	
			(in.)		(in.)	Uncracked	Cracked	Uncracked	Cracked
S/SSW12	9	(1) #3 tie	6	See note 6	—	1,230	880	1,440	1,030
S/SSW15	12	(2) #3 ties	6	See note 6	—	1,590	1,135	1,810	1,295
S/SSW18	14	(1) #3 hairpin	85	(1) #3 hairpin	6	Hairpin reinforcement achieves maximum allowable shear load of the Steel Strong-Wall panel.			
S/SSW21	15	(2) #3 hairpins	85	(1) #3 hairpin	6				
S/SSW24	17	(2) #3 hairpins	85	(1) #3 hairpin	6				

1. Shear anchorage designs conform to ACI 318-14 and assume minimum f'_{C} = 2,500 psi concrete.

2. Shear reinforcement is not required for panels installed on a cold-formed steel floor, interior foundation applications

(panel installed away from edge of concrete) or braced-wall panel applications.

3. Seismic indicates Seismic Design Category C through F. Detached 1 and 2 family dwellings in SDC C may use wind anchorage solutions.

Seismic shear reinforcement designs conform to ACI 318-14 Section 17.2.3.5.3.

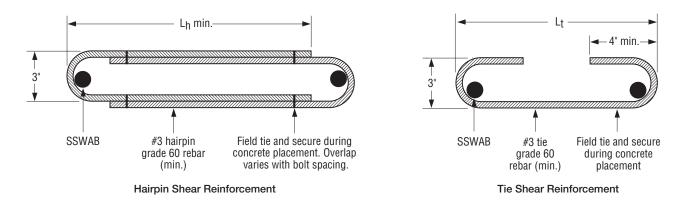
4. Wind includes Seismic Design Category A&B.

5. Where noted, minimum curb/stemwall width is 6" when standard-strength SSWAB is used.

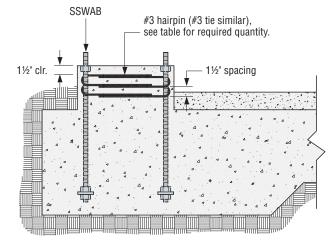
6. Use (1) #3 tie for S/SSW12 and S/SSW15 when the Steel Strong-Wall® panel design shear force exceeds the tabulated

anchorage allowable shear load.

7. The registered design professional may specify alternate shear anchorage.



Lateral Systems

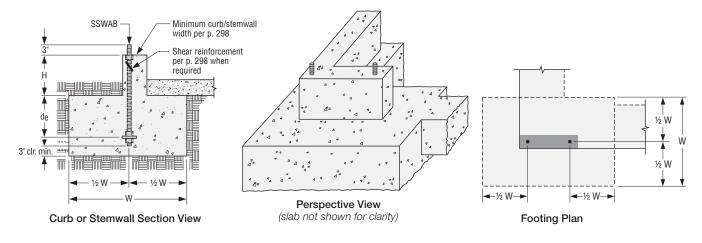


Hairpin Installation (garage curb shown, other footing types similar)

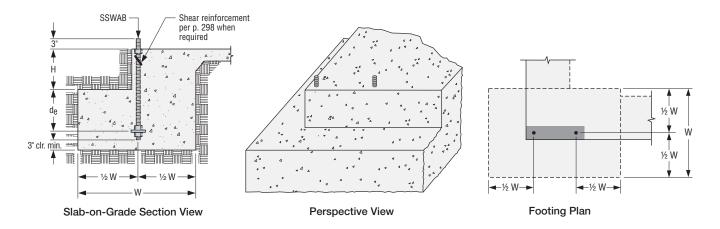
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See pp. 296–297 for tension anchorage.

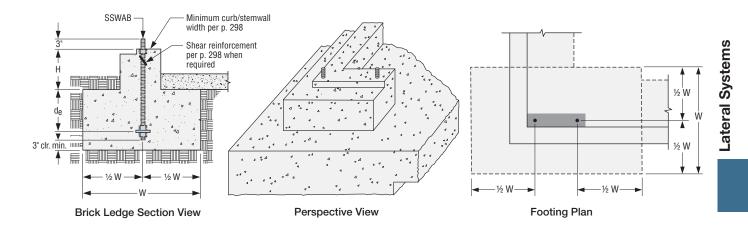
Curb or Stemwall Installation



Slab-On-Grade Installation



Brick Ledge Installation



Anchorage Solutions General Notes

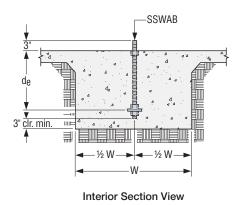
1. The Designer may specify alternate embedment, footing size or bolt grade.

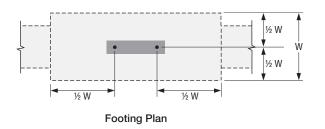
2. Footing dimensions and rebar requirements are for anchorage only.

Foundation design (size and reinforcement) by Designer.

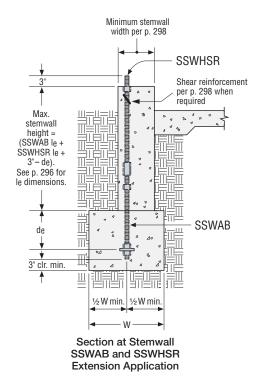


Interior Installation









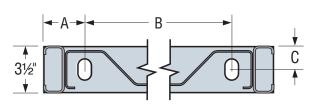
Anchorage Solutions General Notes

1. The Designer may specify alternate embedment, footing size or bolt grade.

2. Footing dimensions and rebar requirements are for anchorage only.

Steel Strong-Wall Anchor Bolt Layout

Wall Model	Distance From End of Wall to Center of SSWABs (A)	Distance From Center to Center of SSWABs (B)	Distance From Exterior Face of Wall to Center of All SSWABs (C)
S/SSW12	2%16"	67⁄8"	2"
S/SSW15	27⁄8"	91⁄4"	17⁄8"
S/SSW18	21⁄8"	121⁄4"	17⁄8"
S/SSW21	27⁄8"	151⁄4"	17%"
S/SSW24	27⁄8"	181⁄4"	1 7⁄8"



Steel Strong-Wall® Grade Beam Anchorage Solutions

Simpson Strong-Tie now provides grade beam anchorage solutions for the Steel Strong-Wall®, which have been calculated to conform to ACI 318-14. Through funding from the Structural Engineers Association of Northern California, initial testing at Scientific Construction Laboratories Inc. confirmed the need to comply with ACI 318 requirements to prevent plastic hinging at anchor locations. Follow-on testing at the Simpson Strong-Tie Tyrell Gilb Research Laboratory was then used to confirm these findings and validate performance. The testing consisted of specimens with closed tie anchor reinforcement, non-closed u-stirrups and control specimens without anchor reinforcement. Flexural and shear reinforcement were designed to resist amplified anchorage forces and compared to test beams designed for non-amplified strength level forces. The test program has proven the performance of the anchor reinforcement details developed by Simpson Strong-Tie.

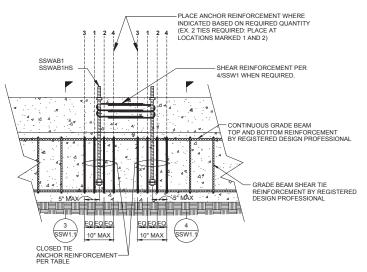
Significant Findings From Testing:

Grade beam flexural and shear capacity is critical to anchor performance and must be designed to exceed the demands created by the attached structure. In wind load applications, this demand includes the factored demand from the Steel Strong-Wall (S/SSW) shearwall. In seismic applications, testing and analysis has shown that in order to achieve the anchor performance expected by ACI 318 design methodologies, the concrete member design strength needs to resist the amplified anchor design demand from ACI 318-14 Section 17.2.3.4.3 and ACI 318-11 Appendix D Section D.3.3.4.3. To help designers achieve this, Simpson Strong-Tie recommends Designers apply the seismic design moment listed in the table below at the S/SSW location when evaluating the grade beam design strength under seismic loads. The tabulated moment correlates to the lowest of the anchor tension design limits defined in the sections listed above as they relate to each S/SSW model.

Closed tie anchor reinforcement is critical to maintain the integrity of the reinforced core where the anchor is located. Testing with u-stirrups that did not include complete closed ties showed premature splitting failure of the grade beam.



Grade Beam Test



S/SSW Grade Beam Anchorage Detail See strongtie.com for additional grade-beam anchor reinforcement details and requirements.

Steel	Anchor Bolt Model No.	Anchor Diameter (in.)	Anchor Reinforcemen	t for Wind and Seismic	LRFD Applied Seismic Design Moment for Grade Beam Design (ftlb.)	
Strong-Wall Model			Standard-Strength SSWAB	High-Strength SSWABHS	Standard-Strength SSWAB	High-Strength SSWABHS
S/SSW12	SSWAB3/4	3⁄4	(2) #4 closed ties / wall	(5) #4 closed ties / wall	16,700	23,000
S/SSW15			(4) #4 closed ties / wall	(7) #4 closed ties / wall	37,000	44,000
S/SSW18	SSWAB1	-1	(2) #4 closed ties / anchor	(4) #4 closed ties / anchor	48,700	61,000
S/SSW21		SSWADI I			60,300	77,000
S/SSW24					72,000	87,000

Steel Strong-Wall Grade Beam Anchorage Solutions

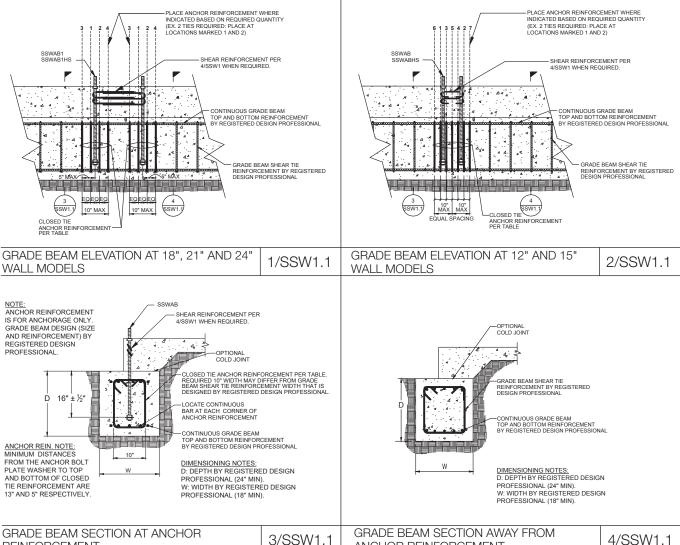
1. Anchor reinforcement conforms to ACI 318-14, Section 17.4.2.9 and ACI 318-11, Section D.5.2.9. Full-scale testing was used to validate anchor reinforcement configuration and placement.

2. Minimum concrete compressive strength, $f'_{c} = 2,500$ psi.

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- 3. Closed tie anchor reinforcement to be ASTM A615 Grade 60 (min.) #4 rebar.
- 4. Grade beam longitudinal and tie reinforcement shall be specified by the registered design professional for flexure and shear loading. Design should
- registered design professional for flexure and snear loading. Design should consider project specific design loads and allowable soil pressure.
- 5. Simpson Strong-Tie recommends using the tabulated minimum LRFD-applied seismic design moment to ensure grade-beam design flexure and shear strength is adequate to prevent plastic hinge formation under demands associated with anchorage forces corresponding to ACI 318-14, Section 17.2.3.4.3 and ACI 318-11, Section D.3.3.4.3.
- 6. Designer may use reduced moment due to applied S/SSW lateral load. Minimum moment shall be the lesser of the tabulated moment or the amplified LRFD design moment for seismic: (ASD Shear/0.7) x Ω_0 x S/SSW height for grade-beam design.
- 7. Minimum grade beam design moment for wind and seismic in Seismic Design Category A and B and detached one- and two-family dwellings in SDC C: (ASD Shear/0.6) x S/SSW height.
- 8. Closed tie may be single-piece hoop or two-piece assembly with a u-stirrup with 135° hooks and a top-cross tie cap.
- 9. Closed-tie anchor reinforcement quantity is per wall for the 12"- and 15"-wall models, and per anchor for the 18", 21" and 24" models.

Steel Strong-Wall® Anchorage Details



STEEL STRONG-WALL WIDTH (in.) ANCHOR DIAMETER (in.) ANCHOR MODEL NO STANDARD STRENGTH SSWAP SSWAB3/4 SSWAB3/4HS 12" MODEL 3/4 2- #4 CLOSED TIES PER 15" MODEL 4- #4 CLOSED TIES PER 18" MODEL SSWAB1 1 SSWAB1HS 2- #4 CLOSED TIES PER 21" MODEL 24" MODEL

NOTES

REINFORCEMENT

SSW GRADE BEAM ANCHOR REINFORCEMENT

ANCHOR REINFORCEMENT FOR WIND AND SEISMIC 3.8.5

ANCHOR REINFORCEMENT

HIGH STRENGTH

(HS) SSWAB

5- #4 CLOSED TIES PER

7- #4 CLOSED TIES PER

4- #4 CLOSED TIES PER

TES: ANCHOR REINFORCEMENT CONFORMS TO ACI 318-14 SECTION 17.4.2.9 AND ACI 318-11 SECTION D.5.2.9 AND PERFORMANCE WAS VALIDATED THROUGH FULL SCALE TESTING. MINIMUM CONCRETE COMPRESSIVE STRENGTH, fc = 2,500 psi. CLOSED TIE ANCHOR REINFORCEMENT TO BE ASTM A615 GRADE 60 (MIN) #4 REBAR. GRADE BEAM LONGITUDINAL AND TIE REINFORCEMENT SHALL BE SPECIFIED BY THE REGISTERED DESIGN PROFESSIONAL FOR FLEXURE AND SHEAR LOADING. DESIGN SHOULD CONSIDER PROJECT SPECIFIC DESIGN LOADS AND ALLOWABLE SOIL PRESSURE. SIMPSON STRONG-TIE RECOMMENDS USING THE TABULATED MINIMUM LRFD APPLIED SEISMIC DESIGN MOMENT TO ENSURE GRADE BEAM DESIGN FLEXURE AND SHEAR STRENGTH IS ADEQUATE TO PREVENT PLASTIC HINGE FORMATION UNDER DEMANDS ASSOCIATED WITH ANCHORAGE FORCES CORRESPONDING TO ACI 318-14 SECTION 17.2.3.4.3 AND ACI 318-11 SECTION D.3.4.3. DESIGNER MAY USE REDUCED MOMENT DUE TO APPLIED SSIW LATERAL LOAD. MINIMUM MOMENT SHALL BE THE LESSER OF THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM WHETOH TO DESTING PROFILED SEISMIC MOMENT SHALL BE THE LESSER OF THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM WHETOH TO CRADE PROFILED SEISMIC MOMENT SHALL BE THE LESSER OF THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM WHETOH TO DE TO APPLIED SEISMIC MOMENT SHALL BE THE LESSER OF THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM WHETOH TO BE AND PESICON DESIGN AS THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM WHETOH TO DE TO APPLIED SEISMIC MOMENT SHALL BE THE LESSER OF THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM WHETOH TO DESTING TO ACT AND PESICON DESTING TO ACT AND THE TABULATED MOMENT OR THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO Y SOM Y SOM TO THE AMPLIFIED LRFD DESIGN MOMENT FOR SEISMIC: (445) SHEAPOT 12 V DO 5. 6.

(ASD SHEAR/0.7) x Ωo x SSW HEIGHT FOR GRADE BEAM DESIGN. MINIMUM GRADE BEAM DESIGN MOMENT FOR WIND AND SEISMIC IN SEISMIC DESIGN CATEGORY A AND B AND DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C: (ASD SHEAR/0.6) × SSW HEIGHT

COSED THE MAY BE SINGLE PIECE HOOP OR TWO PIECE ASSEMBLY WITH A U-STIRRUP WITH STANDARD 135 DEGREE HOOKS AND A TOP CROSS THE CAP. SEE DETAIL 6/SSW1.1. SEE DETAILS FOR GRADE BEAM ANCHOR REINFORCEMENT PLACEMENT, INSTALLATION AND SPACING REQUIREMENTS. CLOSED THE ANCHOR REINFORCEMENT QUANTITY IS PER WALL FOR THE 12" AND 15" WALL MODELS, AND PER ANCHOR FOR THE 18", 21" AND 24" MODELS.

SSWAB ANCHOR GRADE BEAM REINFORCEMENT AND DESIGN MOMENTS

5/SSW1.1

LRFD APPLIED DESIGN SEISMIC MOMENT (ft.-lb.) 4.5.6.7

STANDARD

STRENGTH SSWAB

16,700

37.000

48,700

60,300

72.000

HIGH STRENGTH

(HS) SSWAB

23,000

44.000

61,000

77.000

87.000

SIMPSON

Strong-Tie

C-CF-2017 @ 2017 SIMPSON STRONG-TIE COMPANY INC.

Connectors for Cold-Formed Steel Construction

Steel Strong-Wall® Anchor Bolt Templates

Simpson Strong-Tie now offers anchor bolt stabilizers that may be used with all anchor template models. The bolt stabilizer enables the Steel Strong-Wall anchorage to be installed without being tied to the footing rebar cage by helping to eliminate movement of the anchor bolts during concrete placement. Two bolt stabilizers are used for each SSW anchor assembly; one at the embedded plate washer and the other above the template. Half-inch-diameter dowels (not supplied) are then driven down through the bolt stabilizers and into the ground to ensure plumb installation of the anchors and prevent movement during concrete placement. Immediately following concrete placement, the dowels are removed and reused in other locations.

Steel Strong-Wall Anchor Bolt Templates

_								
	Steel	Width	Anchor Bolt Stabilizer Model	Steel Strong-Wall Template Model				
	Strong-Wall Model	(in.)		Reversible	Panel Form	Brick Ledge	Extended Leg	
	S/SSW12	12	SSWBS12	SSWT12	SSWTPF12	SSWTBL12	SSWTEL12	
	S/SSW15	15	SSWBS15	SSWT15	SSWTPF15	SSWTBL15	SSWTEL15	
ſ	S/SSW18	18	SSWBS18	SSWT18	SSWTPF18	SSWTBL18	SSWTEL18	
	S/SSW21	21	SSWBS21	SSWT21	SSWTPF21	SSWTBL21	SSWTEL21	
ſ	S/SSW24	24	SSWBS24	SSWT24	SSWTPF24	SSWTBL24	SSWTEL24	

1. The height of the garage curb above the garage slab is critical for rough header opening at garage return walls.

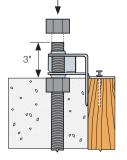
2. Templates are recommended and are required in some jurisdictions.

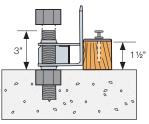
3. Foundation design by Designer.

4. Reversible, panel form and bridge ledge templates are the same for 4" or 6" thick walls.

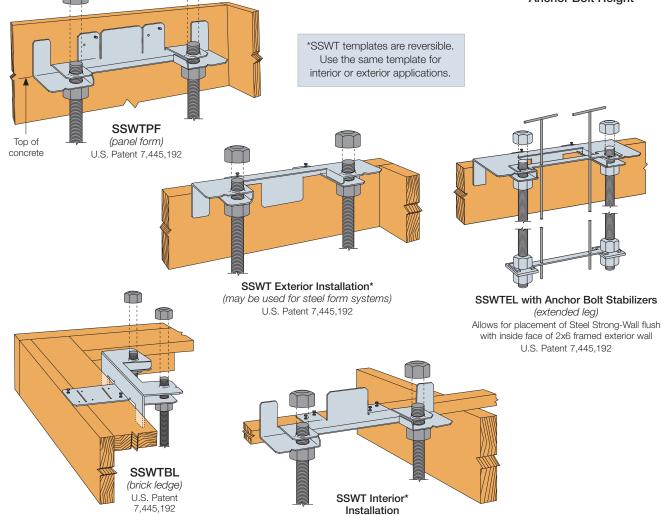


An additional nut for template installation is provided with each SSWAB. It may also be used for SSW installation.





Anchor Bolt Height



U.S. Patent 7,445,192

Miscellaneous

网络图

1.00

EIM I

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LCE Post Cap

The universal design of the LCE4 provides high capacity while eliminating the need for rights and lefts. For use with $3\frac{1}{2}$ " to 6" framing members.

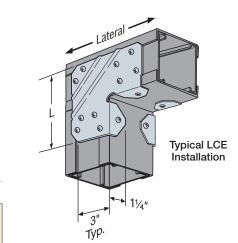
Material: LCE4 - 33 mil (20 ga.)

Finish: Galvanized, ZMAX® coating or stainless steel: see Corrosion Information, pp. 18–21

Installation:

- Use all specified fasteners; see General Notes
- Install in pairs

Codes: See p. 11 for Code Reference Key Chart



Installation for Cold-Formed Steel Built-Up Column Note: The Designer is responsible for design of column and beam member.

	Dimensions (in.)		isions	Fasteners ¹		Allowable Load (lb.)				Code
			(Total)		Uplift		Lateral			
	No.	W	L	Beam	Post	33 mil (20 ga.)	43 mil (18 ga.)	33 mill (20 ga.)	43 mil (18 ga.)	Ref.
	LCE4	_	5%	(14) #10	(10) #10	1,700	2,355	1,420	2,150	170

These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. See pp. 138 through 171 for more information on Simpson Strong-Tie fasteners.

TP/TPA Tie Plates

TPs are screw-on tie plates. TPAs are flanged for added support.

Material: 33 mil (20 ga.)

Finish: Galvanized

Installation:

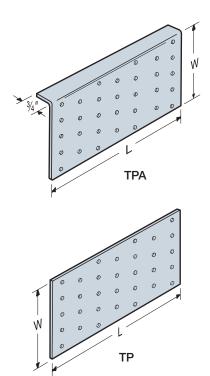
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• Holes are sized for #8 or #10 screw

Codes: See p. 11 for Code Reference Key Chart

Model	Dimensi	ons (in.)	Number of	Code Ref.	
No.	W	L	Nail Holes		
TP15	1 ¹³ ⁄16	5	13		
TPA37	31⁄2	7	32		
TPA39	31⁄2	9	41		
TP35	31⁄8	5	23		
TP37	31⁄8	7	32		
TP39	31⁄8	9	41		
TP311	31⁄8	11	50	180	
TP45	41⁄8	5	30		
TP47	41⁄8	7	42		
TP49	41⁄8	9	54		
TP411	41⁄8	11	66		
TP57	5%	7	60		
TPA57	5	7	49		

1. Connectors are not load rated.



PSPN Protecting Shield Plate

SIMPSON Strong-Tie

PSPN58 and PSPN516 protecting shield plate fastener stoppers meet IRC, IBC and the International Plumbing Code. PSPN516 meets the code plumbing protection requirements as well as having additional fasteners if the Designer chooses to use it as a track splice strap.

Material: 54 mil (16 ga.)

Finish: Galvanized, available in ZMAX® coating

Installation:

• Flatten prongs with hammer as needed

• Use #10 screws

Codes: See p. 11 for Code Reference Key Chart

PSPN516 at top plates

- International Residential Code[®] 2006/2009/2012 P2603.2.1
- International Plumbing Code 2006/2009/2012 305.8

PSPN516 at bottom plate.

International Plumbing Code — 2006/2009/2012 305.8

- PSPN58 at top plates and bottom plate.
- International Plumbing Code 2006/2009/2012 305.8

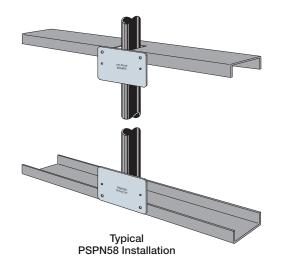
Note that the IBC section 2308.9.8 and the IRC section 602.6.1 require a 54 mil (16 ga.) strap with (6)16d nails and (8)16d nails respectively each side at a hole or notch in a wood top, sill or sole plate. The Designer or local building jurisdiction may permit an equivalent fastener strength (e.g., screws in lieu of nails) to be used for the same condition in a CFS top or bottom track.

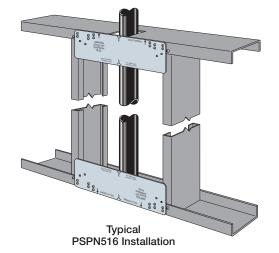
Model No.	W (in.)	L (in.)	Code Ref.	
PSPN58	5	8	100	
PSPN516	5	165⁄16	190	

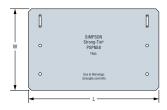
These products are available with additional corrosion protection. Additional products on this page may also be available with this option. Check with Simpson Strong-Tie for details.

1. #10 self-tapping screws may be used to attach PSPN to CFS framing with quantity determined by Designer.

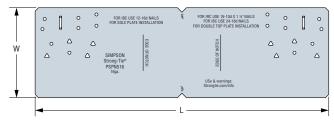
2. PSPN516 with (6) #10 self-tapping screws each side achieves an allowable shear capacity of 1,060 lb. and 1,580 lb. to 33 mil (20 ga.) track and to 43 mil (18 ga.) track, respectively.







PSPN58



PSPN516

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Connectors for Cold-Formed Steel Construction

PSCL/PSCA Panel Sheathing Clips

Simpson Strong-Tie[®] panel sheathing clips are used to brace unsupported sheathing edges and provide a $\frac{1}{2}$ gap to address shrinkage and expansion of roof sheathing.

Material: 33 mil (20 ga.)

Finish: Galvanized

Installation:

- Use the same size sheathing clip as the panel thickness
- Maximum spans may be reduced for low slopes or high uniform loads; refer to manufacturer's installation instructions

Codes: See p. 11 for Code Reference Key Chart

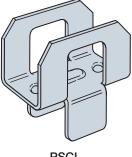
	Span	Panel Thickness	Model	Maximum Roof	No. of	Code	
	Rating	(in.)	No.	With Clip	Without Clip	Clips Per Span	Ref.
	24/0	3⁄8	PSCL%	24	20	1	
	24/16	7⁄16	PSCA7/6	24	24	1 ²	180
	24/16	7⁄16	PSCL7/16	24	24	1	
	32/16	15/32	PSCA ¹⁵ /32	32	28	1 ²	
1	32/16	15/32	PSCL ¹⁵ 32	32	28	1	
	32/16	1/2	PSCA1/2	32	28	1 ²	100
	32/16	1/2	PSCL1/2	32	28	1	
	40/20	5⁄8	PSCL%	40	32	1	
	40/20	19/32	PSCL ¹ %2	40	32	1	
	48/24	3⁄4	PSCL3/4	48	36	2	

 Span rating and Maximum Roof Sheathing Spans are for reference only, refer to 2015 IBC Table 2304.8 (3) for additional important information.

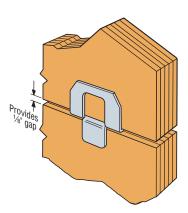
2. Maximum roof sheathing span with single PSCA is 24".

For spans > 24" use two PSCAs.

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PSCL (PSCA similar)



Typical PSCL Installation

Loads of Anchoring Options

You now have tabulated design values for anchorage. The new SCHA slide-clip connector is designed and tested to accommodate several different methods of anchoring to concrete or steel, which helps you easily account for the buckling capacity of the anchored leg and mitigate risk. The SCHA also features a wider support leg to decrease eccentricity on anchors and provide a variety of anchorage options.

With prepunched holes to accommodate ¼"-diameter Titen HD[®] concrete screw anchors or 0.157" PDPAT pins for attachment to steel, the SCHA connector is easy to specify and to install. To learn more, call (800) 999-5099 or visit **strongtie.com/scha**.

SIMPSON Strong-Tie

C-CF-2017 Effective 6/1/2017 Expires 12/31/2019 © 2017 Simpson Strong-Tie Company Inc.